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TRANSPORTATION IMPACT ANALYSIS

To

City of Newberg /
Oregon Department of
Transportation

For

Fred Meyer Stores, Inc.
Fuel Facility Expansion
Newberg, Oregon

Submitted

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Project Number

2140436.00



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I. INTRODUCTION

Mackenzie presents this Transportation Impact Analysis (TIA) to address the potential impacts of the proposed Fred Meyer fuel facility expansion in Newberg, Oregon. The site is located on the south side of Highway 99W between Springbrook Road and Brutscher Street. Figure 1 is a vicinity map indicating the project location.

The subject property is zoned Community Commercial (C-2) in which the existing fuel facility was approved as a conditional use in September 2010; the current application represents an update to the prior conditional use approval. The prior approval was informed by our March 17, 2009 TIA and subsequent documents.

Project Description

The existing fuel facility provides 14 vehicle fueling positions (VFPs) at the west side of the Fred Meyer Newberg store near its driveway accessing Springbrook Road. The proposed expansion will add four VFPs by extending the canopy over two new fuel dispensers on the south side of the existing facility. No new driveways or changes to existing driveways are proposed. Construction is planned to occur in one phase, and completion is expected in 2015. Site plans prepared by Barghausen Consulting Engineers are provided in the Appendix for reference.

Scope of Analysis

This TIA addresses impacts of the proposed fuel facility expansion on the surrounding roadways, the Fred Meyer site driveways, and the Fred Meyer internal circulation system. Analyses are presented to address both City of Newberg and Oregon Department of Transportation (ODOT) requirements. Review includes local pedestrian and bicycle facilities, crash history, sight distance, intersection operation analysis, and intersection queuing. Specific attention is given to Phase 1 of the Newberg-Dundee Bypass Project, which is currently under construction and is anticipated to be complete in 2017. Additional analyses are presented to address local concerns raised during the original application process for the current fuel facility, including on-site circulation and fuel delivery truck operations.

Traffic analyses are presented for the following two periods:

- Weekday PM Peak Hour
- Saturday Midday Peak Hour

Traffic analyses are presented for the following four scenarios:

- 2014 Existing
- 2015 Pre-Development (without expansion)
- 2015 Post-Development (with expansion)
- 2017 Post-Development (with expansion) with Newberg-Dundee Bypass Phase 1 complete

Traffic analyses are presented at the following study area intersections:

- Springbrook Road/Hayes Street
- Springbrook Road/Fred Meyer Driveway
- Highway 99W/Springbrook Road
- Highway 99W/Fred Meyer West (Right-In) Driveway
- Highway 99W/Fred Meyer East (Right-In Right-Out) Driveway
- Highway 99W/Brutscher Street
- Brutscher Street/Fred Meyer North Driveway

- Fred Meyer On-Site Intersections:
 - Main Drive Aisle/US Bank/Fuel Drive Aisle
 - Main Drive Aisle/West (Right-In) Driveway
 - Main Drive Aisle/East (Right-In Right-Out) Driveway

II. EXISTING CONDITIONS

Site Conditions

The fuel facility occupies a portion of the overall Fred Meyer store site, located at 3300 Portland Road in Newberg, Oregon; and identified as tax lot 2004 in Section 16 of Township 3 South, Range 2 West of the Willamette Meridian on Yamhill County Assessor's maps. Five other contiguous commercial developments share parking/circulation areas and roadway accesses with the Fred Meyer site; uses include a bank, fast food restaurants, an office/retail building, and a quick lubrication vehicle service shop.

The existing fuel facility provides 14 vehicle fueling positions (VFPs) at the west side of the Fred Meyer store. Customer traffic flows one way (east to west) past the dispensers, and all vehicles are directed to turn left upon exiting the canopy area to circulate counterclockwise back to the north-south drive aisle between the fuel facility and the Fred Meyer store.

The overall site accesses the public roadway system at six locations. Four of these driveway intersections are included in this TIA study area:

- Springbrook Road driveway (right-in/right-out only)
- Highway 99W West Driveway (right-in only)
- Highway 99W East Driveway (right-in/right-out only)
- Brutscher Street North Driveway (full movement)

The two other site driveways provide full movement access to Brutscher Street and Little Oak Street.

Transportation Facilities

The following table summarizes the study area roadway classifications and characteristics, as identified by Mackenzie staff and as classified by the *Newberg Transportation System Plan* (June 2005), the *1999 Oregon Highway Plan* (revised through August 22, 2013), and the ODOT Functional Classification and National Highway System Status Table (as of October 2014).

TABLE 1 – STUDY AREA ROADWAY CHARACTERISTICS						
Roadway	Classification	Posted Speed (mph)	Travel Lanes	Bike Lanes	On-Street Parking	Side-walks
Highway 99W	City: Major Arterial ODOT: Statewide Highway, National Highway System (NHS), National Network (NN, a federally designated truck route), Freight Route (FR), Reduction Review Route (RRR), Other Principal Arterial-Urban	35	5	Yes	No	Yes
Springbrook Road	Minor Arterial	25 (N of 99W) 35 (S of 99W)	2 to 3	Partial	No	Partial
Brutscher Street	Major Collector	25	3	Yes	No	Yes
Hayes Street	Major Collector	25	2	Yes	No	Yes

All Fred Meyer site driveways are stop controlled. The Highway 99W intersections at Springbrook Road and Brutscher Street are both signalized and allow U-turns along Highway 99W. The Fred Meyer on-site intersections are all-way stop controlled, except the eastbound Main Drive Aisle approach to the East Driveway (right-in/right-out) operates freely; this intersection is modeled as all-way stop controlled to generate a configuration compatible with standard analysis methodologies.

Figure 2 presents the existing lane configurations and traffic controls at study area intersections.

Pedestrian and Bicycle Facilities

Sidewalks and bicycle lanes are currently provided throughout most of the study area, including along the entire Fred Meyer site frontage. The exception is a portion of Springbrook Road south of the Fred Meyer site frontage where a separated paved path along the west side of the roadway provides the path for pedestrians and bicyclists.

Transit Facilities

There are two bus services that provide transit service in the study area.

Yamhill County Transit Area (YCTA) operates three bus routes serving Newberg. Route 44 travels between McMinnville and Tigard on Hwy 99W (46S on Saturdays), with a stop at Brutscher Street and service between 1-2 hours during weekdays. Routes 5 and 7 serve the north and east ends of Newberg, with stops every hour. Only route 7 stops at Fred Meyer. The operation schedules for the three routes are located in the Appendix.

Traffic Volumes

Turning movement count data were collected at study area intersections during the weekday PM peak period (4:00-6:00 PM) and during the Saturday midday peak period (11:00 AM-2:00 PM) on October 16th and 18th, 2014, respectively. The system peak hours were identified as 4:00-5:00 PM (weekday) and 1:00-2:00 PM (Saturday) for this analysis. Copies of the data reports are provided in the Appendix.

In accordance with Chapter 5.4.1 of the ODOT *Analysis Procedures Manual*, Version 2 (APM v2), October 2014 Edition, a seasonal adjustment factor of 1.065 is applied to through and U-turn volumes along Highway 99W. This adjustment is applied to estimate the 30th highest hour for the year, which typically occurs during July or August in the Newberg vicinity. Details of the adjustment calculations and data from Automatic Traffic Recorder (ATR) station 36-004 are provided in the Appendix.

Figures 3A, 3B, 3C, and 3D present a summary of the system peak hour volumes: the volumes as counted, the seasonal adjustments applied, and the adjusted existing intersection volumes.

Crash Analysis

Intersection Crash Analysis

When evaluating the relative safety of an intersection, consideration is given not only to the total number and types of crashes occurring, but also to the number of vehicles entering the intersection. This leads to the concept known as “crash rate,” typically expressed in terms of the number of crashes occurring per one million vehicles entering the intersection (MEV). Intersections having a crash rate less

than 1.0 crashes/MEV are considered relatively safe. At crash rates higher than 1.0 crashes/MEV, consideration may be given to correcting operational problems.

For this analysis, annual traffic entering each study area intersection is estimated by multiplying the average annual daily traffic (AADT) entering the intersection by 365. AADT is estimated by multiplying the intersection weekday PM peak hour total volume by a factor, a typical method of estimating daily traffic. The most recent ODOT ATR daily volume count (2013 data) on Highway 99W just west of Brutscher Street is approximately 11.4 times the recent (October 2014) PM peak count volumes, so a factor of 11.5 is applied to the seasonally adjusted 2014 intersection volumes as an approximation method for this area.

Crash data for the study area intersections were obtained from the ODOT Zigzag data portal for the five-year period spanning January 2009 through December 2013. The following table presents calculated crash rates at the study intersections for the data period. Crash data are presented in the Appendix.

Intersection	Total Crashes	Crash Types			Worst Injury Severity				AADT	Crash Rate
		Angle/Turn	Rear-End/Sideswipe	Other	A	B	C	PDO		
1 Highway 99W / Springbrook Road	86	14	70	2	1	3	31	51	51,175	0.92
2 Highway 99W / Fred Meyer West (Right-In) Driveway	0	0	0	0	0	0	0	0	18,538*	0
3 Highway 99W / Fred Meyer East (Right-In Right-Out) Driveway	0	0	0	0	0	0	0	0	16,192**	0
4 Highway 99W / Brutscher Street	27	4	22	1	1	3	12	11	41,262	0.36
5 Springbrook Road / Fred Meyer Driveway	0	0	0	0	0	0	0	0	13,248	0
9 Brutscher Street / Fred Meyer North Driveway	6	3	3	0	0	0	3	3	8,556	0.38
12 Springbrook Road / Hayes Street	1	0	1	0	0	0	1	0	10,281	0.05

* Includes only traffic involved in the turning movement, so only eastbound volumes at this location.

** Includes only traffic involved in the turning movements, so only eastbound and northbound volumes at this location.

All study area intersections have crash rates less than 1.0 crashes/MEV, so no further review is merited at these locations based on crash rates. A majority of crashes in the data set are coded as rear-end types, which are common at signalized intersections and difficult to mitigate. No fatalities are noted in the data set; the worst injury severity noted is “InjA – Incapacitating Injury,” which occurred twice in five years. A majority of crashes were “PDO – property damage only” with no reported injuries.

SPIS Review

ODOT maintains a Safety Priority Index System (SPIS) to identify the state highway segments exhibiting the most severe safety needs. Specific to the study area for this analysis, two Highway 99W segments are noted in the 2013 SPIS listing; a copy of the listing is provided in the Appendix. The following table provides a summary of the relevant segments.

TABLE 3 – TOP 10% SPIS SEGMENTS						
Location	Beginning MP	End MP	ADT	Crashes	SPIS Score	SPIS Rank
Brutscher Street	21.71	21.87	34,100	19	48.88	Top 10%
Leg (to N Springbrook Road)	21.95	22.14	38,500	56	66.79	Top 5%

For reference, the Brutscher Street intersection is located at MP 21.80, and the Springbrook Road intersection is located at MP 22.05. It is anticipated ODOT staff may identify crash patterns or safety concerns within the study area, particularly within the identified SPIS segments, during TIA review. If this occurs, the merits of potential mitigation measures will be addressed at that time.

III. PRE-DEVELOPMENT CONDITIONS

Future traffic conditions without the proposed fuel facility expansion are addressed as pre-development conditions. These include anticipated changes to intersection volumes and to roadway conditions.

Background Growth

General growth in traffic volumes not related to specific projects is termed background growth. Review of 2008-2013 data from the ODOT ATR 36-004 indicates traffic volumes have grown slowly—at less than 1% per year—since the last economic downturn. ODOT Future Volume Tables, however, suggest a 2.1% annual growth rate is anticipated at ATR 36-004 from 2013 to 2033. As a conservative estimate, 2.0% background growth is applied to the adjusted existing intersection volumes to estimate 2015 pre-development volumes. Figures 4A and 4B presents background growth from 2014 to 2015 for a typical weekday PM peak hour and Saturday mid-day peak hour.

In-Process Traffic

Traffic volumes generated by approved projects not yet completed at the time of analysis is termed in-process traffic. One project, likely to add trips to the study area, was identified by City of Newberg staff is Springbrook Ridge Apartments, located at the northeast corner of the Springbrook Road/E 2nd Street-Fernwood Road intersection. Study area traffic for the project was estimated based on the September 25, 2013, Traffic Impact Analysis prepared by Associated Transportation Engineering & Planning, Inc.; trip details from this TIA are provided in the Appendix.

Figures 5A and 5B present the in-process trips for the current study area intersections. Saturday trips are estimated based on the Saturday peak hour of the generator regression equation provided for ITE Land Use Code 220 – Apartments. It is assumed 10% of the apartment trips traveled to and from the Fred Meyer store. The remaining trips traveling on Springbrook Road and Brutscher Street are distributed according to current traffic patterns.

Planned Improvements

The Newberg-Dundee Bypass Project, when fully completed, will shift through traffic away from the existing Highway 99W alignment. Construction funding and dates for the full bypass are uncertain, but Phase 1 of the project, from west Dundee to Highway 219 in Newberg, is under construction and is anticipated to be complete by 2017 according to ODOT staff estimates. The Bypass project will provide the following improvements within the TIA study area:

- At the Highway 99W/Springbrook Road intersection:
 - Add a second westbound left-turn lane
 - Add a second southbound receiving lane (for 300 feet)
 - Rebuild the traffic signal
- Along Springbrook Road between Highway 219 and Highway 99W:
 - Widen to a three-lane section
 - Provide a shared pedestrian-bicyclist path

The City of Newberg *2013-2014 Capital Improvement Program* (April 2013) does not identify any transportation projects funded for completion within the TIA study area and study analysis years.

Pre-Development Volumes

Pre-development volumes for 2015, the anticipated year of the fuel facility expansion, are the sum of the adjusted existing volumes, background growth, and in-process traffic. Figures 6A and 6B present the 2015 pre-development traffic volumes.

IV. SITE DEVELOPMENT

The proposed development will expand the existing fuel facility from 14 to 18 vehicle fueling positions (VFPs). An extension of the canopy to the south to cover the additional fuel dispensers will result in a loss of five existing parking spaces; overall site parking minimums will still be met, however.

Trip Generation

Trip generation estimates for the proposed fuel facility expansion are prepared using average trip rates from Land Use Code 944 – Gasoline/Service Station in the Institute of Transportation Engineers' (ITE) *Trip Generation Manual*, 9th Edition (2012) and the ITE *Trip Generation Handbook*, 3rd Edition (2014). Due to the nature of the fuel facility, no alternate trip modes are assumed. No large trucks are anticipated to use the fuel facility other than for fuel delivery.

ITE estimates a fuel facility expansion of four VFPs will generate 674 additional weekday trips, including 55 additional trips during the weekday PM peak hour.

ITE does not provide Saturday data for Gasoline/Service Station (Code 944), so Saturday peak hour trips are estimated as 16% greater than the weekday PM peak hour trips. The percentage is based on 2002 trip data from other Pacific Northwest Fred Meyer fuel facilities that indicated Saturday peak hour trip generation was approximately 16% greater than weekday PM peak hour trip generation. A copy of the Fred Meyer fuel facility trip data may be provided upon request. According to this factor, a fuel facility expansion of 4 VFPs will generate 64 Saturday peak hour trips.

Trip Types

ITE estimates the total number of vehicle trips to and from the fuel facility. Total trips are divided into internal, pass-by, diverted linked, and primary trips below.

Fred Meyer stores offer a per-gallon discount on fuel purchases to Rewards Card Members. Trip data from Fred Meyer trip surveys indicate internal trip rates of 35% to 52% at fuel facilities sited adjacent to Fred Meyer stores. As a conservative estimate the internal trip rate is estimated as 20% of the total trips to match the value applied in our March 2009 TIA for the existing fuel facility.

The percentages for the other trip types are determined based on Table F.36 of the ITE *Handbook*, specifically from the six surveys that include separate data for primary and diverted linked trips. Consistent with ITE procedures, the rates for pass-by, diverted linked, and primary trips are applied to the external trips remaining after subtracting the internal trips.

The following summarizes the trip type splits applied to this analysis.

TABLE 4 – TRIP TYPES				
Trip Types:	Internal Trips	External Trips		
		Pass-By	Diverted Linked	Primary
% of Total Trips:	20%	41.6%	27.6%	10.8%
% of External Trips:		52%	34.5%	13.5%

ITE provides trip type values only for the weekday AM and PM peak hours, so the weekday PM peak hour trip types are applied to the typical weekday and Saturday peak hour trip estimates.

Internal Trips

In a development such as Fred Meyer, many customers take advantage of the multiple services and retailers at the site in a single trip. These are known as shared or internal trips. For purposes of this analysis, internal trips will remain on the Fred Meyer site and will not add to traffic volumes at site driveways or on public roadways. As noted above, internal trips are estimated as 20% of the total trips. The remaining 80% of the total trips are considered external trips because they add to traffic volumes at or beyond site driveways.

Pass-By Trips

Pass-by trips are those site trips already driving past the site on the adjacent roadways. These trips do not increase the total traffic volumes on the roadways, but do add to turning movement volumes at the site accesses. A pass-by trip to other uses at the site that also stops at the fuel facility is considered an internal trip. The ITE *Handbook* indicates an average pass-by percentage of 52% during the weekday PM peak hour (calculated as noted above). This rate is applied to the Saturday midday peak hour as well.

The Fred Meyer site accesses Highway 99W, Springbrook Road, Brutscher Street, and Little Oak Street. Existing volumes on Little Oak Street are much lower than those on the other roadways, and most through traffic on Brutscher Street also travels on Highway 99W, so pass-by trips are assumed to be drawn only from existing traffic volumes approaching the study area on Highway 99W and Springbrook Road.

Diverted Linked Trips

Diverted linked trips are those site trips already traveling in the site vicinity on roadways other than those immediately adjacent to the site. These vehicles change their direction or route to access the site. The ITE *Handbook* estimates an average diverted linked trip rate of 34.5% during the weekday PM peak hour. This rate is applied to the Saturday midday peak hour as well.

Parallel routes near the proposed fuel facility include Highway 219-E 1st Street, E 2nd Street-Fernwood Road, Elliott Road, Providence Drive-Werth Boulevard, and Benjamin Road, so diverted linked trips could be drawn from these roadways. However, because trips from any of these roadways would likely use Highway 99W, Springbrook Road, or Brutscher Street to access the fuel site, diverted linked trips are treated together with primary trips.

Primary Trips

Primary trips are those site trips stopping only at the fuel facility and then returning to their point of origin. These are considered new trips generated by the expansion. The ITE *Handbook* indicates an average pass-by percentage of 13.5% during the weekday PM peak hour (calculated as noted above). This rate is applied to the Saturday midday peak hour as well. When treated together with the diverted linked trips, the primary trips represent 48% of the external site trips.

Summary

The following table summarizes the trip generation estimates for the proposed Fred Meyer fuel facility expansion from 14 to 18 VFPs.

TABLE 5 – TRIP GENERATION CHARACTERISTICS – GASOLINE/SERVICE STATION (ITE CODE 944)							
Trip Type (Rate)	Daily (ADT)	Weekday PM Peak Hour			Saturday Midday Peak Hour		
		Enter	Exit	Total	Enter	Exit	Total
Total Trips	674	27	28	55	32	32	64
<i>Internal Trips (20%)</i>	135	5	6	11	7	6	13
External Trips (80%)	539	22	22	44	25	26	51
<i>Pass-By Trips (52%)</i>	280	11	12	23	13	14	27
Diverted Linked + Primary Trips (48%)	259	11	10	21	12	12	24

The fuel facility expansion will generate 259 additional diverted linked+primary trips during a typical weekday, including 21 additional diverted linked+primary trips in the weekday PM peak hour, and will generate 24 additional diverted linked+primary trips during the Saturday midday peak hour.

Trip Distribution and Assignment

Trip distribution is addressed differently according to trip type. Note due to driveway movement restrictions all external trip paths to/from the south are assumed to travel via Brutscher Street and/or Hayes Street.

Internal Trips

Internal trips remain on the Fred Meyer site. Based on review of historical aerial imagery, a majority of on-site vehicles park near the Fred Meyer store entrances, which are accessed from the Main Drive Aisle between the West (Right-In Only) Access and the East (Right-In/Right-Out) Access intersections. All the internal trips added with the proposed fuel facility expansion are assigned to travel to/from these parking areas. Figures 7A and 7B present the internal trip assignments.

Pass-By Trips

The pass-by trips are distributed based on the proportions of traffic approaching the study area in the October 2014 counts from four approaches:

- Westbound Highway 99W at Brutscher Street
- Southbound Springbrook Road at Highway 99W
- Eastbound Highway 99W at Springbrook Road
- Northbound Springbrook Road at the Fred Meyer Driveway

The weekday PM peak hour and Saturday midday peak hour distributions are both estimated as follows:

- 45% from westbound Highway 99W
- 15% from southbound Springbrook Road
- 30% from eastbound Highway 99W
- 10% from northbound Springbrook Road

Figures 8A and 8B present the pass-by trip distributions and assignments. Negative values identify movements from which trips have been rerouted.

Primary Trips

Trip distribution for primary trips is based on previous studies prepared for the Newberg Fred Meyer store and fuel facility. The distributions are estimated as follows:

- 15% to and from the east on Highway 99W
- 25% to and from the north on Springbrook Road
- 35% to and from the west on Highway 99W
- 25% to and from the south on Springbrook Road or Brutscher Street

Figures 9A and 9B present the primary trip distribution and assignments.

Total Trips

Figures 10A and 10B present the total trip impact anticipated with the proposed fuel facility expansion, or the sum of internal trips, pass-by trips, and primary trips.

Post-Development Volumes

Post-development volumes for 2015, the anticipated year of the fuel facility expansion, are the sum of the 2015 pre-development volumes and the total site trip assignment. Figures 11A and 11B present the 2015 post-development traffic volumes.

Post-develop volumes for 2017, the anticipated year of the Newberg-Dundee Bypass Project Phase 1 completion, are based on Figure 3A (2011 Existing Traffic Conditions) of the September 2011 Phase 1 Technical Report Addendum Memorandum by Kittelson & Associates, Inc., and Figure 7 (Year 2016 Phase 1 Scenario) of their July 2012 Analysis TIS. The reroute of traffic with the bypass is presented in Figure 12, based on the difference in the volumes identified for the intersection of Highway 99W with Springbrook Road.

The Kittelson memo addresses only weekday PM peak hour conditions, so Saturday conditions are not addressed for 2017 scenarios. Figure 13 presents the 2017 post-development traffic volumes, which is the addition of the reroute volume to Post-Development 2015 volumes.

V. CIRCULATION EVALUATION

Fuel Facility Circulation

At the existing fuel facility customer traffic flows one way (east to west) past the dispensers, and all vehicles are directed to turn left upon exiting the canopy area to circulate in a counterclockwise fashion back to the Fuel Drive Aisle and then to the Main Drive Aisle. Customers are discouraged from using the loading area behind the Fred Meyer store as an alternative to the Main Drive Aisle. This circulation pattern is anticipated to continue with the proposed expansion.

On-Site Intersections

Traffic circulation for the fuel facility includes access on the Main Drive Aisle just west of the Fred Meyer store and across from the US Bank site. For purposes of this analysis, the Main Drive Aisle/US Bank/Fuel Drive Aisle intersection treats the Fuel Drive Aisle as intersecting opposite the nearest US Bank exit aisle and the only US Bank entry aisle. Due to the offset of the US Bank entry aisle, the eastbound left turn lane extends into the intersection, across the Fuel Drive Aisle. This intersection is all-way stop controlled to maintain low speeds and to rotate right-of-way among the approaching vehicles. As is shown in the Intersection Analysis section, delays at this intersection are currently well within the standard and are anticipated to remain so with the fuel facility expansion. No changes to the intersection configuration are proposed with the fuel facility expansion.

When the current fuel facility was developed two pedestrian crosswalks between the US Bank and Fred Meyer store were added—one crossing the Main Drive Aisle and one crossing the Fuel Drive Aisle—as an alternative to the prior angled crosswalk through the middle of the Main Drive Aisle/US Bank/Fuel Drive Aisle intersection. These two newer crosswalks serve to reduce vehicles' pedestrian delay and to provide a more predictable pedestrian route.

When the current fuel facility was developed, the Main Drive Aisle/West (Right-In) Driveway intersection was modified to provide all-way stop control (the Main Drive Aisle previously moved freely) and to widen the southbound approach entering from Highway 99W. These improvements serve to maintain low speeds, to rotate right-of-way among the approaching vehicles, and to maintain lower delays and queues on the southbound approach than might otherwise occur. As is shown in the Intersection Analysis section, delays at this intersection are currently well within the standard and are anticipated to remain so with the fuel facility expansion. No changes to the intersection configuration are proposed with the fuel facility expansion.

The Main Drive Aisle/East (Right-In/Right-Out) Driveway intersection currently operates with stop control on the two entering approaches (southbound from Highway 99W and westbound from Brutscher Street) while the exiting (eastbound) approach moves freely. This configuration was in place prior to the current fuel facility's development and is not proposed to change with the fuel facility expansion. As is shown in the Intersection Analysis section, delays at this intersection are currently well within the standard and are anticipated to remain so with the fuel facility expansion. (Note a modification was required to generate results compatible with the analysis methodology.)

Fuel Delivery Truck

The fuel delivery truck currently circulates as shown on the two truck turning path exhibits by Barghausen Consulting Engineers (provided in Appendix A). The truck currently enters from the site driveway on Little Oak Street, arriving from Brutscher Street, and then travels through the loading area

behind the store to the fuel facility. Fuel delivery trucks exit through the same route. This route will be maintained with the expansion, as it minimizes conflicts between pedestrians and customer vehicle traffic.

Note the fuel delivery truck does not currently travel to/from south of the Fred Meyer site on Brutscher Street, Hayes Street, or any other local residential roadways. The segment of Little Oak Street between the Fred Meyer Driveway and Brutscher Street lies substantially within the contiguous Springbrook Oaks Office Building commercial parcel, not within public right-of-way. Fuel deliveries currently occur approximately once every two to three days, and no significant increase in delivery frequency is anticipated with the proposed fuel facility expansion. While parked and offloading fuel, the delivery truck may partially impede some vehicle fueling positions, as it does currently, but adequate width will remain on all sides for circulation around the truck. Overall, with fuel deliveries taking place just a few times per week, the impacts of the fuel truck will be negligible.

Access Review

The Fred Meyer site currently has six driveways: two on Highway 99W, one on Springbrook Road, two on Brutscher Street, and one on Little Oak Street. No changes are proposed to any of the driveways with the fuel facility expansion.

The West (Right-In) Driveway on Highway 99W is located approximately 465 feet east of Springbrook Road, and the East (Right-In Right Out) Driveway is located approximately 490 feet west of Brutscher Street, as measured between centerlines; the distance between the driveways is approximately 405 feet. Approach permits for both driveways are on file with ODOT. Both are limited to right turns by a median and have right turn deceleration lanes on the highway.

The Springbrook Road driveway is located approximately 400 feet south of Highway 99W, as measured between centerlines. Movements are limited to right turns by a median.

The North Driveway on Brutscher Street is located approximately 330 feet south of Highway 99W, as measured between centerlines, opposite a driveway accessing Columbia Bank. All movements are permitted.

Sight Distance Review

Our March 2009 TIA evaluated intersection sight distance at the three Fred Meyer exit driveway locations analyzed in this current TIA. In all cases, available sight distance was identified as meeting or exceeding the minimums recommended by the American Association of State Highway and Transportation Officials' (AASHTO) *A Policy on Geometric Design of Highways and Streets*. No changes to the driveway exits are noted since the time of the prior sight distance evaluation; only the posted speed on Highway 99W appears to be different: previously it was 40 mph, and now it is 35 mph, for which a lower sight distance minimum is recommended.

Evidence suggests intersection sight distance standards are met or exceeded at the Fred Meyer site driveways. This assertion may be confirmed upon request.

VI. INTERSECTION ANALYSIS

Operations Analysis

Intersection capacity calculations were prepared using methodologies presented in the Transportation Research Board's *Highway Capacity Manual* 2000 Edition (HCM 2000). Trafficware's Synchro software, Version 8, which implements HCM methodologies, was used to prepare the capacity and level-of-service calculations. Copies of the Synchro HCM reports are provided in the Appendix.

Performance Measures and Standards

The HCM generally assesses intersection capacity characteristics by three measures of effectiveness (MOEs): volume-to-capacity (v/c) ratio, level of service (LOS), and control delay per vehicle (delay).

V/c ratio is a measurement of the theoretical capacity used by a specific traffic volume or for an entire intersection. It is calculated as the traffic demand volume divided by the theoretical capacity.

LOS is a relative measure of the average control delay (in seconds per vehicle) experienced by drivers at an intersection and is described by a letter on the scale from A to F. The HCM defines LOS grades according to delay thresholds. Qualitatively speaking, LOS A represents optimum operating conditions and minimum delay while LOS F indicates long delays and, often, over-capacity conditions.

The City of Newberg uses LOS to evaluate intersection performance on city roadways. Table 5-1 of the *Newberg Transportation System Plan (TSP)*, June 2005 Edition, indicates the operational standard for signalized intersections is LOS D and the operational standard for unsignalized intersections is LOS E. In addition, any intersection operating at or above capacity (i.e., with a v/c ratio equal to or greater than 1.00) is considered to be operating unacceptably, even if the LOS standard is met.

ODOT uses v/c ratio to assess capacity on state roadways. Table 6 of Policy 1F in the *Oregon Highway Plan*, as updated through December 21, 2011, provides the v/c standards outside the Portland metropolitan region. The study area intersections lie within the Newberg urban growth boundary (UGB) and along a Statewide Highway designated as a Freight Route with a posted speed of 35 mph, so the applicable ODOT target is a v/c ratio of 0.85. When the Newberg-Dundee Bypass Project is completed in the future, the Newberg TSP anticipates the current Highway 99W alignment will be classified as a District Highway (and, presumably, not a Freight Route), so the future ODOT v/c target is assumed to be 0.95 for the study area intersections.

Traffic Signals

The Highway 99W traffic signals in the study area are owned and maintained by ODOT, and the Springbrook Road/Hayes Street intersection traffic signal is owned and maintained by the City of Newberg. Copies of the current signal plans and timings were obtained from agency staff and are provided in the Appendix. These current signal timing plans were applied to the Synchro analysis models. Optimized inputs were applied at the Springbrook Road/Hayes Street intersection in lieu of current timing plans, which were unavailable from the City at the time of this TIA issuance date.

The Springbrook Road/Highway 99W and Brutscher Street/Highway 99W intersections in the 2017 Post Development were optimized to accommodate the change in traffic volumes caused by the proposed bypass project as well as the addition of a second westbound left turn lane.

The ODOT APM Version 2, October 2014 Edition, requires traffic analyses use the current edition of the HCM (2010 in this case) unless otherwise specified in the APM. Chapter 2.5.1 notes HCM 2000 methodology may be used to produce intersection v/c ratios, which are not provided in HCM 2010 Synchro output reports. It is also noted the 2010 methodology does not model u-turns, and requires modifications to detector loops (Typical ODOT detector templates for highway through lanes do not include stop bar detection)For these reasons, the analysis has been prepared using HCM 2000 methodology in the Synchro and SimTraffic software program.

Analysis Reporting and Results

Operations analysis is presented for the following scenarios:

- 2014 Existing
- 2015 Pre-Development (without expansion)
- 2015 Post-Development (with expansion)
- 2017 Post-Development (with expansion) with Newberg-Dundee Bypass Phase 1 complete (PM only)

At signalized study area intersections the v/c ratio, LOS, and delay are reported for the overall intersection. At unsignalized intersections the v/c ratio, LOS, and delay are reported for the critical approach lane. Where standards are exceeded, discussion is provided in the following section. The following table presents the operational analysis calculation results for the weekday PM peak hour.

TABLE 6 – INTERSECTION OPERATION ANALYSIS						
Intersection		Time Period	2014 Existing	2015 Pre	2015 Post	2017 Post
1	Highway 99W / Springbrook Road	PM	0.87-D-38.6	0.89-D-39.8	0.90-D-40.1	0.89-D-47.2
		SAT	0.83-C-31.1	0.85-C-32.4	0.86-C-32.8	
3	Highway 99W / Fred Meyer East (Right-In Right-Out) Driveway	PM	0.03-A-9.5	0.03-A-9.5	0.04-A-9.5	0.04-A-9.7
		SAT	0.03-A-9.8	0.03-A-9.9	0.04-A-10.0	
4	Highway 99W / Brutscher Street	PM	0.83-C-28.9	0.85-C-29.9	0.85-C-30.6	0.84-C-34.2
		SAT	0.76-C-20.9	0.77-C-21.4	0.77-C-21.7	
5	Springbrook Road / Fred Meyer Driveway	PM	0.57-C-17.3	0.57-C-17.7	0.59-C-18.3	0.85-E-43.3
		SAT	0.45-B-13.7	0.46-B-14.0	0.48-B-14.4	
6	Main Drive Aisle / US Bank / Fuel Drive Aisle	PM	0.52-B-11.9	0.52-B-11.9	0.55-B-13.0	0.55-B-13.0
		SAT	0.49-B-10.9	0.49-B-10.9	0.53-B-11.9	
7	Main Drive Aisle / West (Right-In) Driveway	PM	0.31-A-9.4	0.31-A-9.5	0.32-A-9.7	0.32-A-9.7
		SAT	0.30-A-9.6	0.30-A-9.6	0.32-A-9.9	
8	Main Drive Aisle / East (Right-In Right-Out) Driveway	PM	0.39-A-8.5	0.39-A-8.5	0.40-A-8.6	0.40-A-8.6
		SAT	0.34-A-8.4	0.34-A-8.5	0.39-A-8.6	
9	Brutscher Street / Fred Meyer North Driveway	PM	0.54-C-20.7	0.56-C-22.2	0.57-C-22.4	0.57-C-22.4
		SAT	0.45-B-14.9	0.47-C-15.4	0.48-C-15.6	
12	Springbrook Road / Hayes Street	PM	0.40-A-8.7	0.41-A-8.7	0.42-A-8.8	0.63-A-9.7
		SAT	0.25-A-7.5	0.27-A-7.4	0.27-A-7.6	

All private on-site intersections and all study area intersections under City of Newberg jurisdiction are anticipated to operate at acceptable levels of service (LOS D or better) in all analysis scenarios. The Springbrook Road/Highway 99W intersection under ODOT jurisdiction, by contrast, is anticipated to operate beyond acceptable v/c ratios in several analysis scenarios. Overall, however, the additional fuel facility expansion trips have little impact on intersection capacity or delay, and all intersections will operate well under capacity. With the by pass Phase 1 project, the v/c will be the same or better as 2015 Pre-Development conditions at the Highway 99 signalized intersections. It is also noted, the ODOT v/c standard will be revised from 0.85 to 0.95 with the full by pass project.

Queuing Analysis

Intersection queuing was evaluated to estimate the queue demand for each signalized approach lane along the Fred Meyer frontage and for the two Fred Meyer driveway entrances from Highway 99W. In accordance with Chapter 2.5.1 of the ODOT APM v2, October 2014 Edition, queues were estimated using a simulation software, specifically Trafficware's SimTraffic Version 8, to model interactions among signalized intersections within the study area. The SimTraffic 95th percentile queues during the 2015 and 2017 analysis scenarios are compared to the available storage.

The following tables present the available queue storage and the anticipated queue demand values for the weekday PM peak hour and Saturday midday peak hour. Available storage is measured from recent

aerial photographs and is rounded to the nearest 5 feet. SimTraffic queue demand results are rounded to the nearest 25 feet. Copies of the SimTraffic Queue Reports are provided in the Appendix.

TABLE 7 – INTERSECTION QUEUING ANALYSIS – WEEKDAY PM PEAK HOUR							
Intersection		Approach	Lane Group	Available Storage	2015 Pre	2015 Post	2017 Post
1	Highway 99W / Springbrook Road	NB	Lt	275	325	400	275
			Th	300	1050	1050	175
			Rt	195	350	375	225
4	Highway 99W / Brutscher Street	EB	Th	365	450	475	525
			NB	Lt	235	375	225
		Th/Rt	235	175	125	300	
7	Main Drive Aisle / West (Right-In) Driveway	SB	Lt	80+	50	50	50
			Rt	80+	50	75	75
8	Main Drive Aisle / East (Right-In Right-Out) Driveway	SB	Lt/Rt	210	50	50	50

TABLE 8 – INTERSECTION QUEUING ANALYSIS – SATURDAY MIDDAY PEAK HOUR							
Intersection		Approach	Lane Group	Available Storage	2015 Pre	2015 Post	2017 Post
1	Highway 99W / Springbrook Road	NB	Lt	275	225	275	-
			Th	300	250	300	-
			Rt	195	150	200	-
4	Highway 99W / Brutscher Street	EB	Th	365	425	400	-
			NB	Lt	235	225	275
		Th/Rt	235	100	150	-	
7	Main Drive Aisle / West (Right-In) Driveway	SB	Lt	80+	50	50	-
			Rt	80+	75	50	-
8	Main Drive Aisle / East (Right-In Right-Out) Driveway	SB	Lt/Rt	210	50	50	-

Springbrook Road at Highway 99W

The northbound approach of Springbrook Road to Highway 99W is currently striped for four lanes; two left lanes, one through lane, and one right lane. The two left lanes have approximately 275 feet of storage each, and the right lane has 195 feet of storage. The through lane provides approximately 300 feet of storage back to the Fred Meyer access.

The analysis indicates queue lengths for all Pre/Post Development lane group movements exceed available storage for the weekday PM peak hour with the current ODOT signal timing. The Pre/Post Development through movement queues for the weekday PM peak hour extends beyond the Fred

Meyer access. Therefore vehicles in the Fred Meyer driveway may need to wait for the queue to clear before proceeding with right turns, which does not impact the public street operation.

With the addition of traffic on Springbrook Road with the Phase 1 By pass project, it is anticipated the signal timing will be adjusted to provide more time for the northbound Springbrook Road approach, such that vehicle queues will be significantly reduced. This is reflected in the table above.

Highway 99W at Brutscher Street

The eastbound approach along Highway 99W is currently striped for four lanes; one left lane, two through lanes, and one right lane. The through lanes have approximately 365 feet of available storage. The northbound approach is currently striped for two lanes; one left lane and one through/right turn lane. Both northbound lanes have approximately 235 feet of storage. A Fred Meyer RIRO access is located approximately 400 feet west of Brutscher Street.

Queuing analysis for all weekday PM peak hour and Saturday mid-day peak hour scenarios show eastbound through and northbound left turn storage may be exceeded at times. The addition of fuel expansion trips has little impact on these queues.

Main Drive Aisle at West (Right-In) Driveway

The southbound right-in driveway approach is currently striped for one left turn lane and one turn right lane. Both of these lanes have 80 feet of available storage and an additional 180 feet of storage from the edge of the driveway crosswalk to the north to the point where the left and right turn lanes form.

The analysis indicates available storage for southbound left turning vehicles is adequate for all weekday PM peak hour and Saturday mid-day peak hour scenarios.

Main Drive Aisle at East (RIRO)

The RIRO driveway's southbound approach is striped for a single left/right turn lane and has 210 feet of available storage. This lane storage is adequate for all scenario queues.

VII. SUMMARY AND RECOMMENDATIONS

This Transportation Impact Analysis (TIA) has been prepared to address the potential impacts of the proposed Fred Meyer fuel facility expansion in Newberg, Oregon. The site is zoned Community Commercial (C-2) in which the existing fuel facility was approved as a conditional use in September 2010. The current application represents an update to the prior conditional use approval.

The existing fuel facility provides 14 vehicle fueling positions (VFPs) at the west side of the Fred Meyer Newberg store near its driveway accessing Springbrook Road. The proposed expansion will add four VFPs by extending the canopy over two new fuel dispensers on the south side of the existing facility. No new driveways or changes to existing driveways are proposed. Construction is planned to occur in one phase, and completion is expected in 2015.

This TIA addresses impacts of the proposed fuel facility expansion on the surrounding roadways, the Fred Meyer site driveways, and the Fred Meyer internal circulation system. Analyses are presented to address both City of Newberg and Oregon Department of Transportation (ODOT) requirements. Impacts are addressed for the weekday PM Peak Hour and Saturday Midday Peak Hour. Conditions with the Phase 1 By pass project are included for the PM Peak Hour, anticipated for 2017.

The overall site accesses the public roadway system at six locations, four of which will typically be used by fuel facility customers. The two other site driveways provide full movement access to Brutscher Street and Little Oak Street. All Fred Meyer site driveways are stop controlled. The Fred Meyer on-site intersections are all-way stop controlled, except the eastbound Main Drive Aisle approach to the East Driveway (right-in/right-out) operates freely.

Sidewalks and bicycle lanes are currently provided throughout most of the study area, including along the entire Fred Meyer site frontage. The exception is a portion of Springbrook Road south of the Fred Meyer site frontage where a separated paved path along the west side of the roadway provides the path for pedestrians and bicyclists. Yamhill County Transit Area (YCTA) operates three bus routes serving Newberg, with Routes 44 and 7 providing service at or adjacent to Fred Meyer.

Turning movement count data were collected at study area intersections during the weekday PM peak period (4:00-6:00 PM) and during the Saturday midday peak period (11:00 AM-2:00 PM) on October 16 and 18, 2014, respectively. The system peak hours were identified as 4:00-5:00 PM (weekday) and 1:00-2:00 PM (Saturday) for this analysis. These volumes were adjusted in accordance with ODOT procedures to estimate the 30th highest hour for the year, which typically occurs during July or August in the Newberg vicinity.

Crash data for the study area intersections were obtained from ODOT for the five-year period spanning January 2009 through December 2013. All study area intersections have crash rates less than 1.0 crashes/MEV, so no further review or mitigation is merited. A majority of crashes in the data set are coded as rear-end types, which are common at signalized intersections and difficult to mitigate.

Future traffic conditions without the proposed fuel facility expansion are addressed as pre-development conditions, and include background growth at 2% per year plus in-process trips added to existing traffic volumes. Only one in-process project was noted by City staff: Springbrook Ridge Apartments, located at the northeast corner of the Springbrook Road/E 2nd Street-Fernwood Road intersection.

The Newberg-Dundee Bypass Project, when fully completed, will shift through traffic away from the existing Highway 99W alignment. Construction funding and dates for the full bypass are uncertain, but

Phase 1 of the project, from west Dundee to Highway 219 in Newberg, is under construction and is anticipated to be complete by 2017 according to ODOT staff estimates. The Phase 1 Bypass project will modify the Highway 99W/Springbrook Road intersection by adding a second westbound left-turn lane, second southbound receiving lane (for 300 feet) and rebuilding the traffic signal. No changes are proposed to other study area intersections that would affect the capacity or operation. Conditions in the PM peak hour for the 2017 condition with Phase 1 of the Bypass project are analyzed, and found to be within acceptable levels.

The proposed expansion of the fuel facility from 14 to 18 vehicle fueling positions (VFPs) is estimated to generate 674 additional weekday trips, including 55 additional trips during the weekday PM peak hour, and 64 Saturday peak hour trips. More than half of the trips visiting the fuel facility are expected to be from vehicles already at the Fred Meyer site or driving by on adjacent roadways.

With the fuel facility expansion, customer traffic flows will continue to be one way (east to west) past the dispensers, and all vehicles are directed to turn left upon exiting the canopy area to circulate in a counterclockwise fashion back to the Fuel Drive Aisle and then to the Main Drive Aisle. Customers are discouraged from using the loading area behind the Fred Meyer store as an alternative to the Main Drive Aisle. This circulation pattern is anticipated to continue with the proposed expansion.

All on-site intersections and driveways to the public streets are anticipated to continue operating at acceptable levels with the fuel expansion, and no changes to the intersection configurations are proposed.

The fuel delivery truck currently enters the site at the driveway on Little Oak Street, arriving from Brutscher Street, and then travels through the loading area behind the store to the fuel facility. Fuel delivery trucks exit through the same route. This route will be maintained with the expansion, as it minimizes conflicts between pedestrians and customer vehicle traffic. Fuel delivery trucks do not currently travel to/from south of the Fred Meyer site on Brutscher Street, Hayes Street, or any other local residential roadways.

Intersection capacity calculations were prepared using methodologies presented in the Transportation Research Board's Highway Capacity Manual, 2000 Edition (HCM 2000), and Trafficware's Synchro software, Version 8, which implements HCM methodologies. The City of Newberg uses LOS to evaluate intersection performance on city roadways, with the operational standard for signalized intersections a LOS D and the operational standard for unsignalized intersections a LOS E. ODOT uses v/c ratio to assess capacity on state roadways, with a ratio of 0.85 for this segment of Highway 99W. When the Newberg-Dundee Bypass Project is completed in the future, the Newberg TSP anticipates the current Highway 99W alignment will be classified as a District Highway, so the future ODOT v/c target is assumed to be 0.95 for the study area intersections.

The intersection of Highway 99W at Springbrook Road currently exceeds ODOT's 0.85 v/c standard, and will continue to do so with the addition of trips from the fuel facility and will the Phase 1 Bypass project. Overall intersection operation is still at a level of service D, and well under capacity, so meets the City of Newberg operational standards. With the full Bypass project, the reduction in traffic volumes and change to the ODOT standard are expected to result in operation meeting ODOT standards. For these reasons, no mitigation is recommended.

Intersection queuing was evaluated in accordance with ODOT standards utilizing Trafficware's SimTraffic Version 8, with 95th percentile queues reported. Queues at the Springbrook Road northbound approach to Highway 99W currently exceed the available storage lengths with the existing ODOT timing, but the

addition of site trips have little impact on the queue lengths. These queue lengths can be reduced with signal timing modifications, and will likely occur with the planned ODOT improvements with the Phase 1 Bypass project.

Queues on the eastbound Highway 99W approach to Brutscher Street extend beyond the existing Fred Meyer driveway. Because this is a right turn exit, vehicles will simply need to wait for queues to clear before entering the Highway. On the Brutscher Street approach to Highway 99W, vehicle queues are expected to occasionally extend to the first Fred Meyer driveway, but the addition of fuel facility trips will have little impact on the queue lengths.

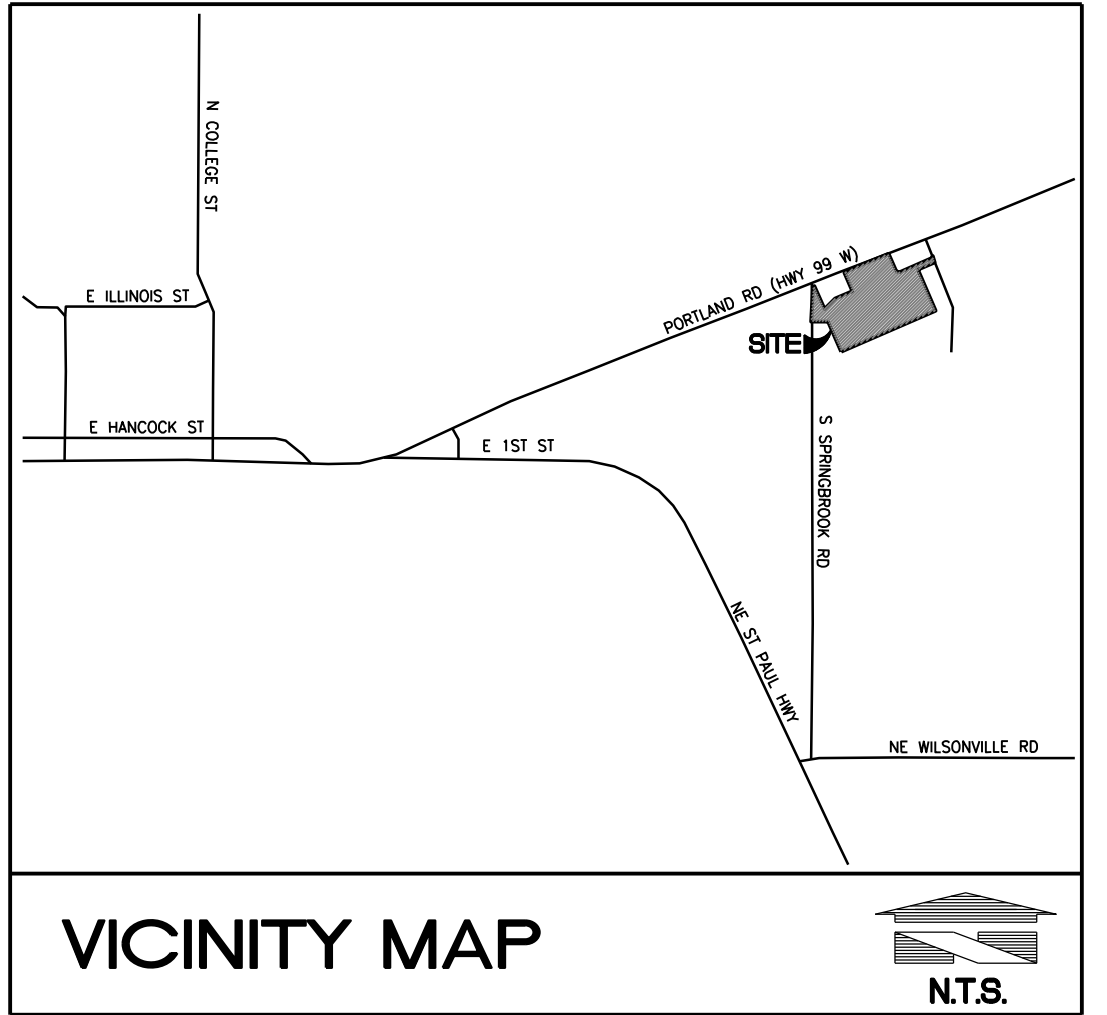
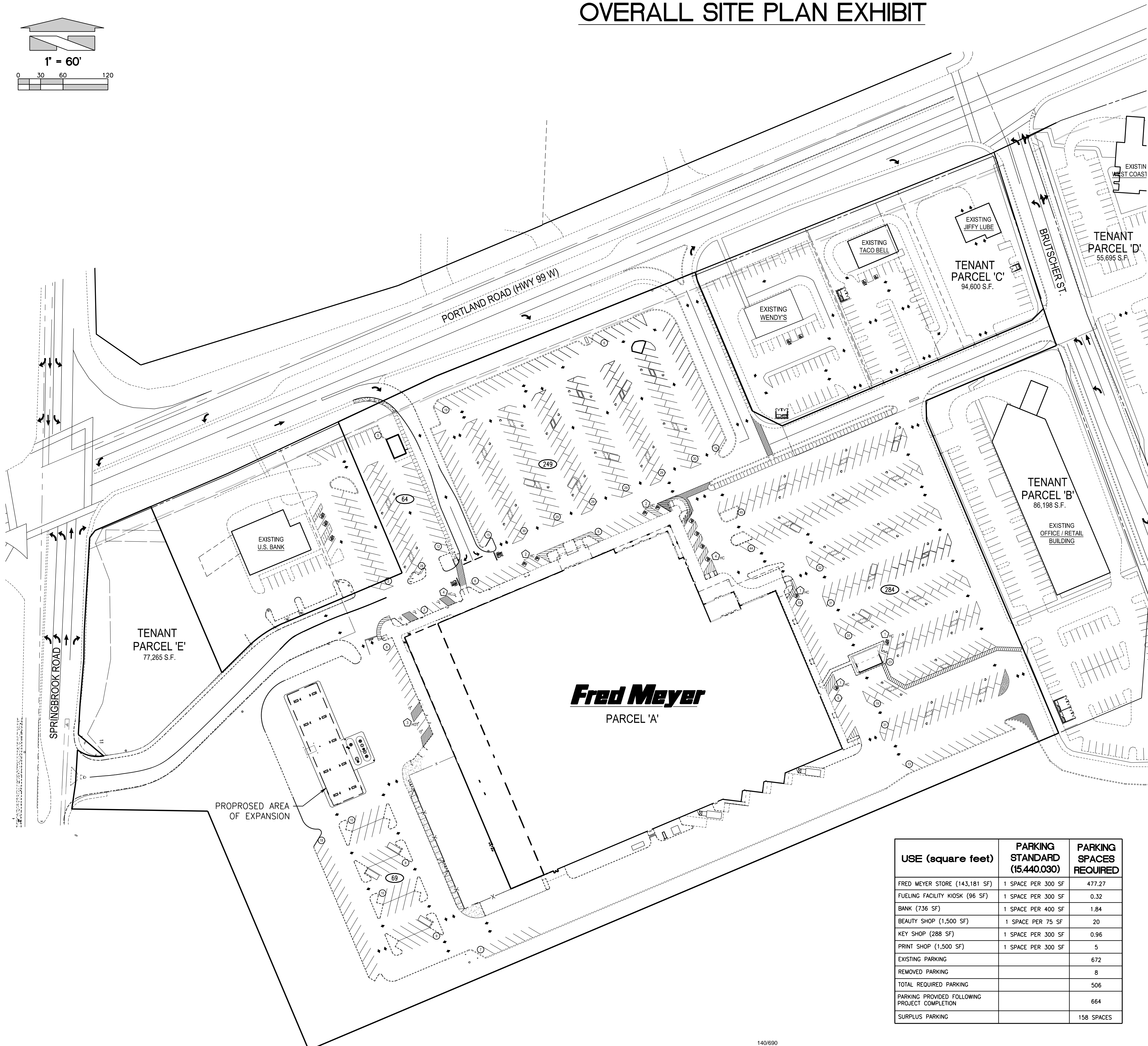
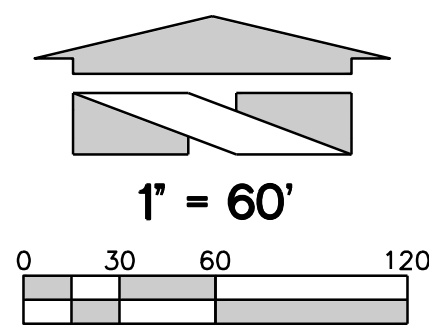
Based on our analysis, the addition of fuel facility expansion trips will have little impact on the operation and queuing at study area intersections. No mitigation measures are recommended.

VIII. APPENDIX

- A. Site Plans and Fuel Truck Routing
- B. Traffic Volume Figures
- C. Traffic Count Summaries
- D. Seasonal Adjustment Factor Data and Calculations
- E. In-Process Projects
- F. Crash Data and SPIS Listing
- G. Signal Plans and Timing Plans
- H. Synchro HCM 2000 Reports
- I. SimTraffic Queuing Reports
- J. Transit Bus Line Schedules
- K. Newberg Dundee Bypass Project Volumes

APPENDIX A
**SITE PLANS AND
FUEL TRUCK
ROUTING**

OVERALL SITE PLAN EXHIBIT



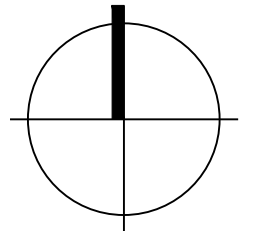
Fred Meyer
 STORE # 00220
 NEWBERG
 3300 Portland Road
 Newberg, OR 97132

THE KROGER
 COMPANY AND
 FRED MEYER
 STORES INC.
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**FRED MEYER
 FUELING
 FACILITY
 ADDITION**
 3300 Portland Road
 Newberg, OR 97132



Fred Meyer
 PARCEL 'A'

USE (square feet)	PARKING STANDARD (15.440.030)	PARKING SPACES REQUIRED
FRED MEYER STORE (143,181 SF)	1 SPACE PER 300 SF	477.27
FUELING FACILITY KIOSK (96 SF)	1 SPACE PER 300 SF	0.32
BANK (736 SF)	1 SPACE PER 400 SF	1.84
BEAUTY SHOP (1,500 SF)	1 SPACE PER 75 SF	20
KEY SHOP (288 SF)	1 SPACE PER 300 SF	0.96
PRINT SHOP (1,500 SF)	1 SPACE PER 300 SF	5
EXISTING PARKING		672
REMOVED PARKING		8
TOTAL REQUIRED PARKING		506
PARKING PROVIDED FOLLOWING PROJECT COMPLETION		664
SURPLUS PARKING		158 SPACES

GENERAL SITE DATA

- ADDRESS/LOCATION: 3300 PORTLAND ROAD
- PARCEL NUMBER: 3216-02004
- MUNICIPALITY: CITY OF NEWBERG
- EXISTING ZONING: C-2
- SURROUNDING ZONING:
 NORTH: C-2
 SOUTH: R-P-SP
 EAST: C-2
 WEST: C-2 AND R-P-SP
- EXISTING SITE USE: COMMERCIAL
- SURROUNDING USES:
 NORTH: COMMERCIAL & SINGLE FAMILY RESIDENTIAL
 SOUTH: AGRICULTURAL, VACANT & SINGLE FAMILY RESIDENTIAL
 EAST: COMMERCIAL
 WEST: COMMERCIAL
- PROJECT AREA DIMENSIONS:
 LENGTH: VARIES
 WIDTH: VARIES
 GROSS TOTAL AREA: 17.36 AC. (756,250 S.F.)
- PARKING:
 COMPACT PROVIDED: 56 (8.4 PERCENT)
 HANDICAPPED PROVIDED: 17 (2.5 PERCENT)
- SITE LANDSCAPING: 116,360 S.F. (15.38 PERCENT)

OWNERS

BANK OF NEW YORK MELLON TRUST COMPANY
 700 S. FLOWER STREET, SUITE 500
 LOS ANGELES, CA 90017

ENGINEER/PLANNER

BARGHAUSEN CONSULTING ENGINEERS, INC.
 18215 72ND AVE. SOUTH
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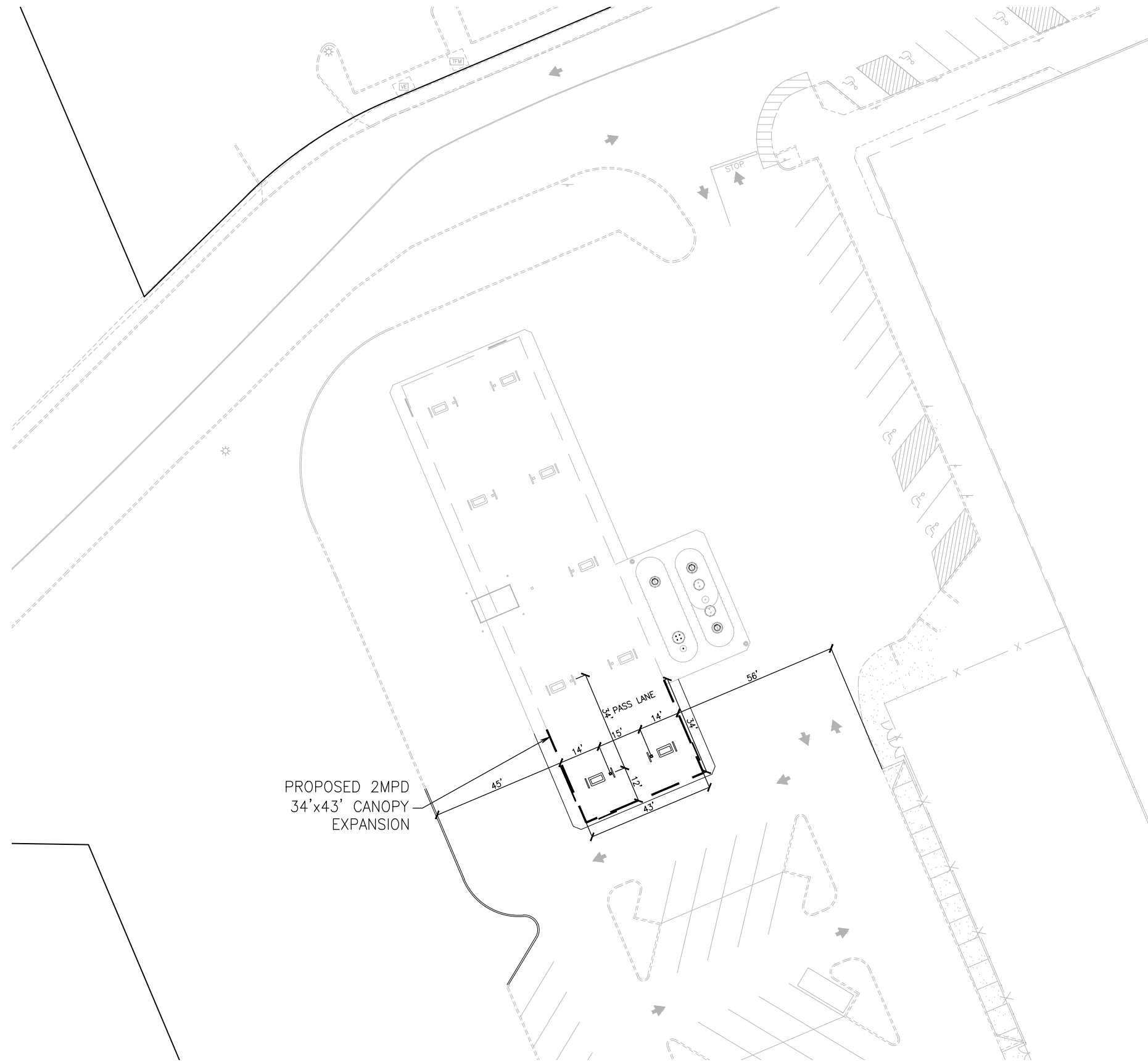
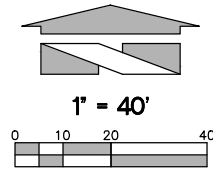
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DATE	DESCRIPTION

BCE #10315
 PM: CHRIS FERKO
 DATE: 10/30/14

**OVERALL
 SITE PLAN
 EXHIBIT**

DETAILED SITE PLAN



PROPOSED 2MPD
34'x43' CANOPY
EXPANSION

LANDSCAPE AREA TO BE REMOVED = 612 SF

DISCLAIMER:

1. THIS PRELIMINARY SITE PLAN IS BASED ON -- BY -- DATED --. THIS PLAN HAS BEEN PREPARED WITHOUT THE BENEFIT OF COMPLETE ZONING INFORMATION OR CONFIRMATION OF SITE DEVELOPMENT STANDARDS AND THUS THE SITE DEVELOPMENT FEASIBILITY IS NOT IMPLIED OR GUARANTEED.

Fred Meyer

STORE #00220
NEWBERG
3300 PORTLAND ROAD
NEWBERG, OR

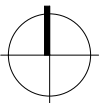
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FRED MEYER CANOPY EXPANSION

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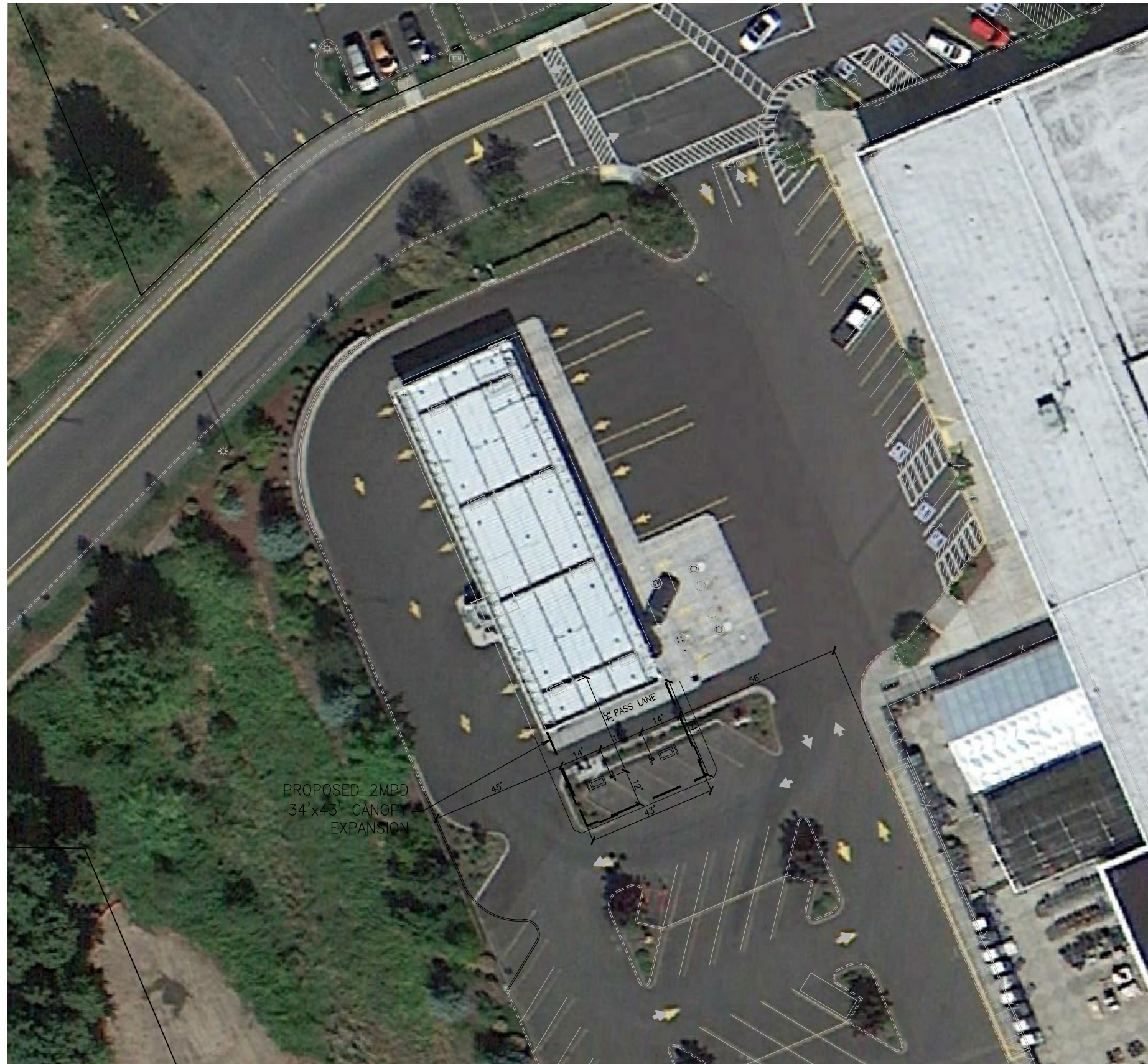
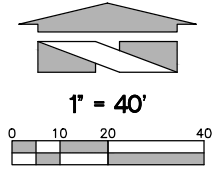
PARKING STATISTICS

EXISTING PARKING	554
PROPOSED PARKING DELETED	-5
PROPOSED PARKING ADDED	+0
PROPOSED TOTAL PARKING	549
REQUIRED PARKING =	507

BCE #10315
PM: CHRIS FERKO
DATE: 11/13/13

DD-2 DETAILED SITE PLAN

DETAILED SITE PLAN



DISCLAIMER:

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LANDSCAPE AREA TO BE REMOVED = 612 SF

PARKING STATISTICS	
EXISTING PARKING	554
PROPOSED PARKING DELETED	-5
PROPOSED PARKING ADDED	+0
PROPOSED TOTAL PARKING	549
REQUIRED PARKING =	507

Fred Meyer

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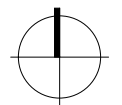
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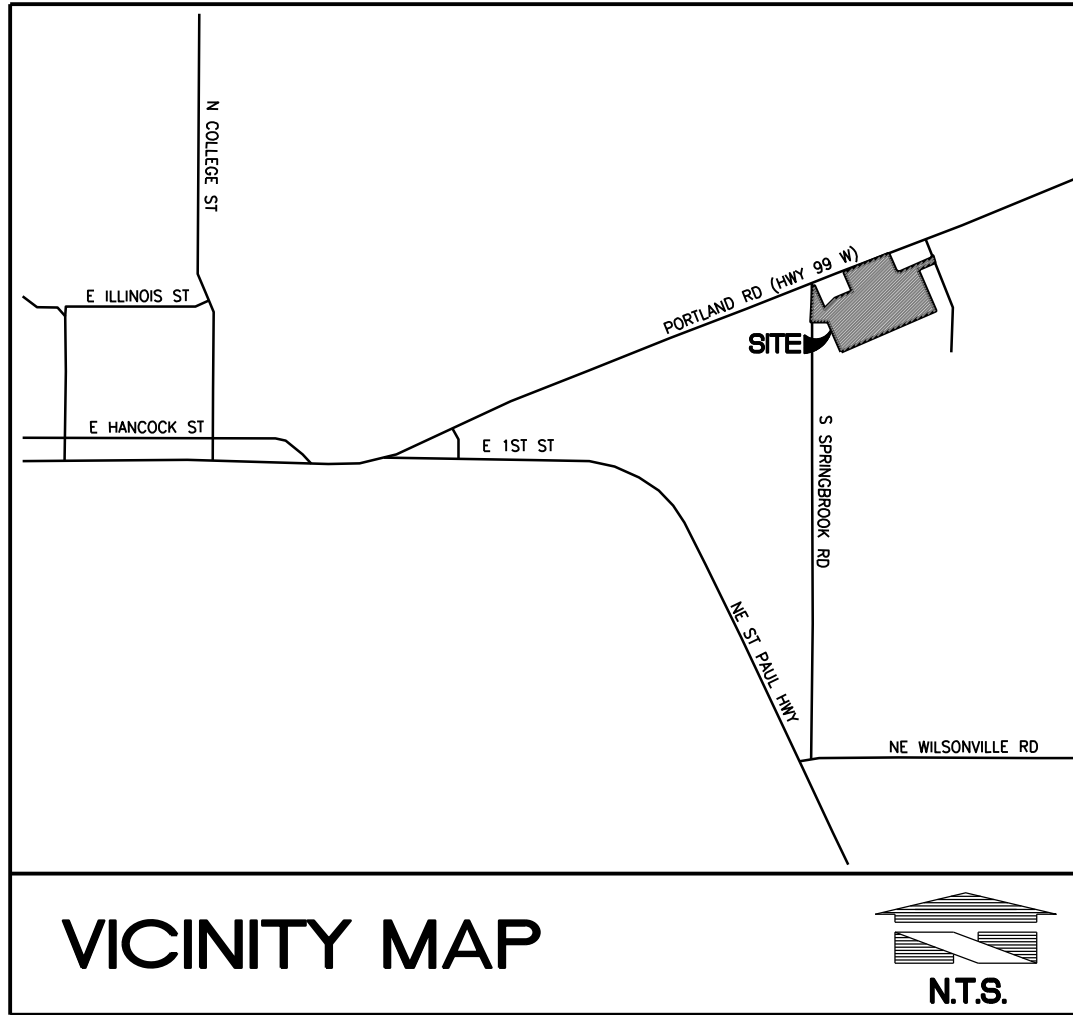
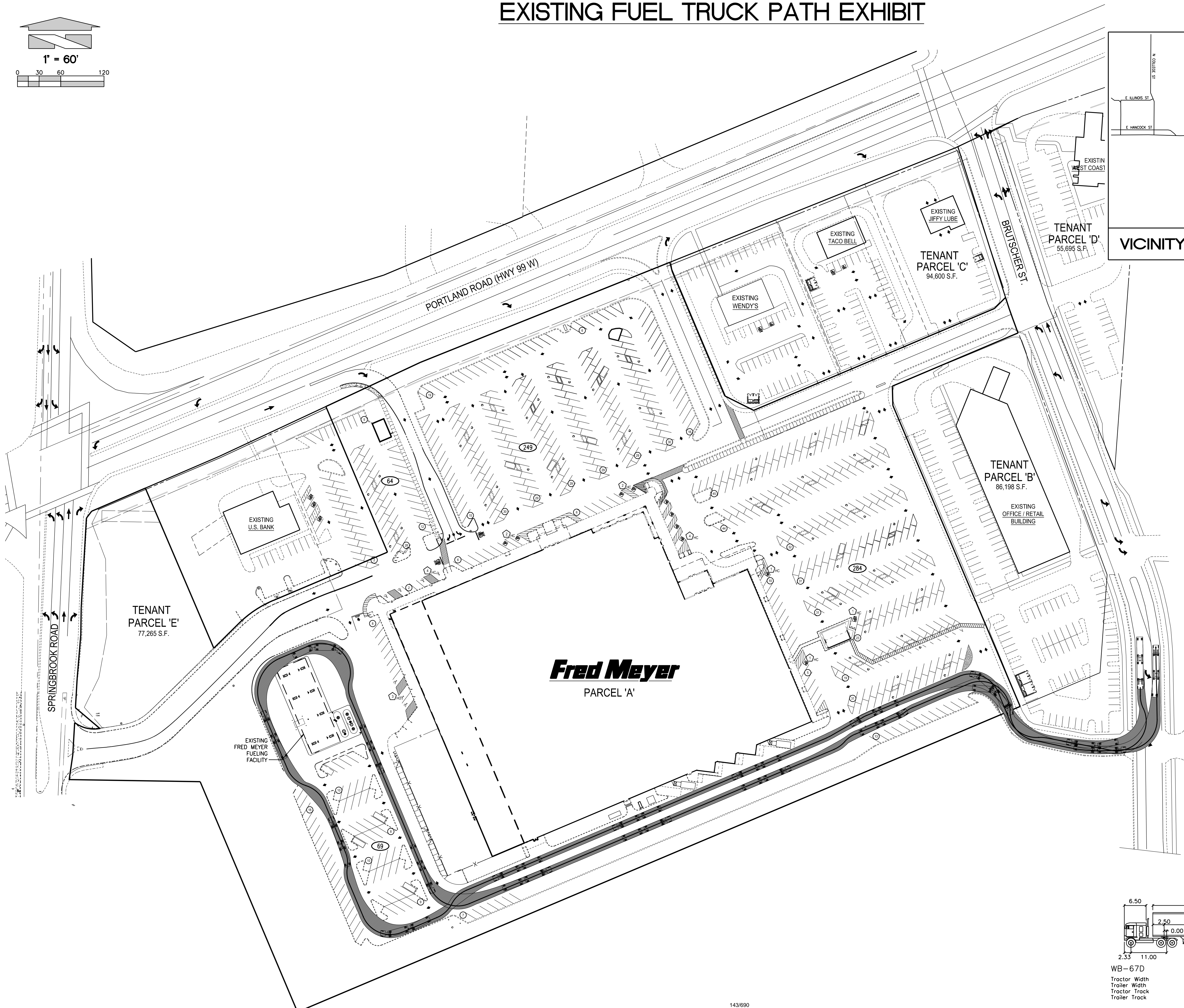
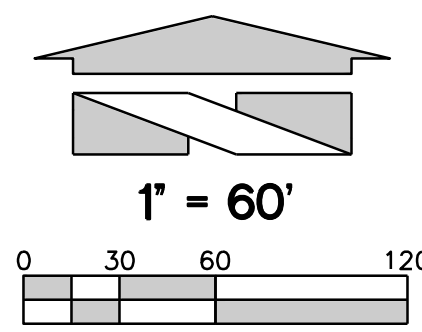


DATE	DESCRIPTION

BCE #10315
PM: CHRIS FERKO
DATE: 11/13/13

**DD-2
DETAILED
SITE PLAN**

EXISTING FUEL TRUCK PATH EXHIBIT

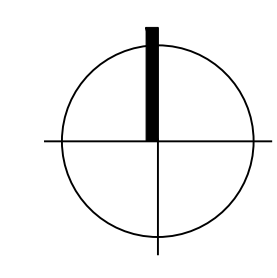


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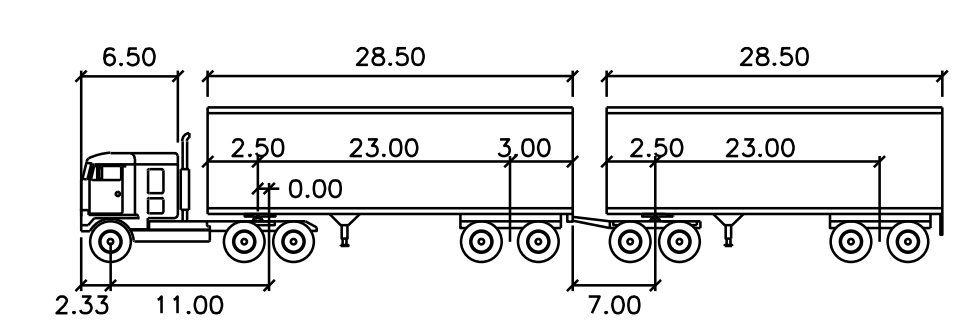
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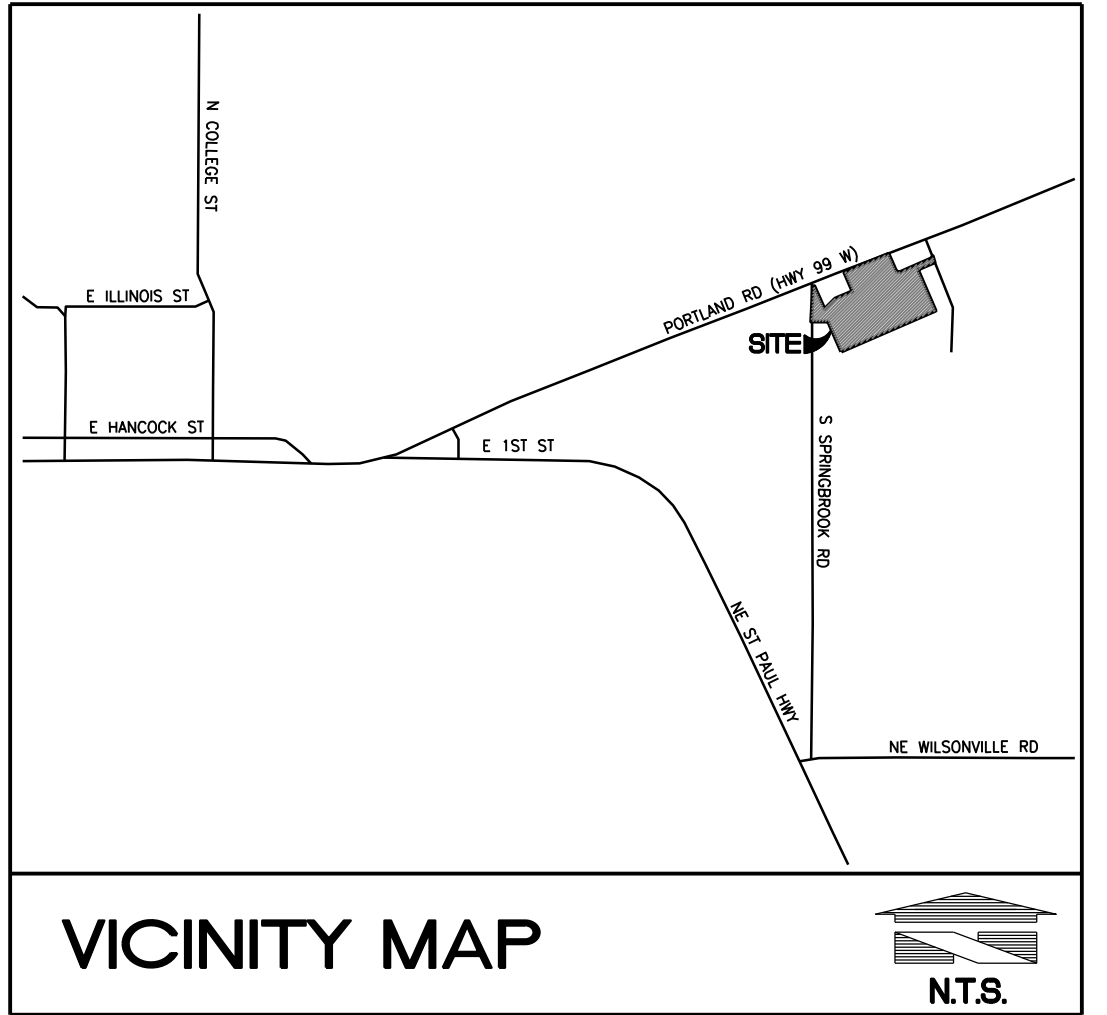
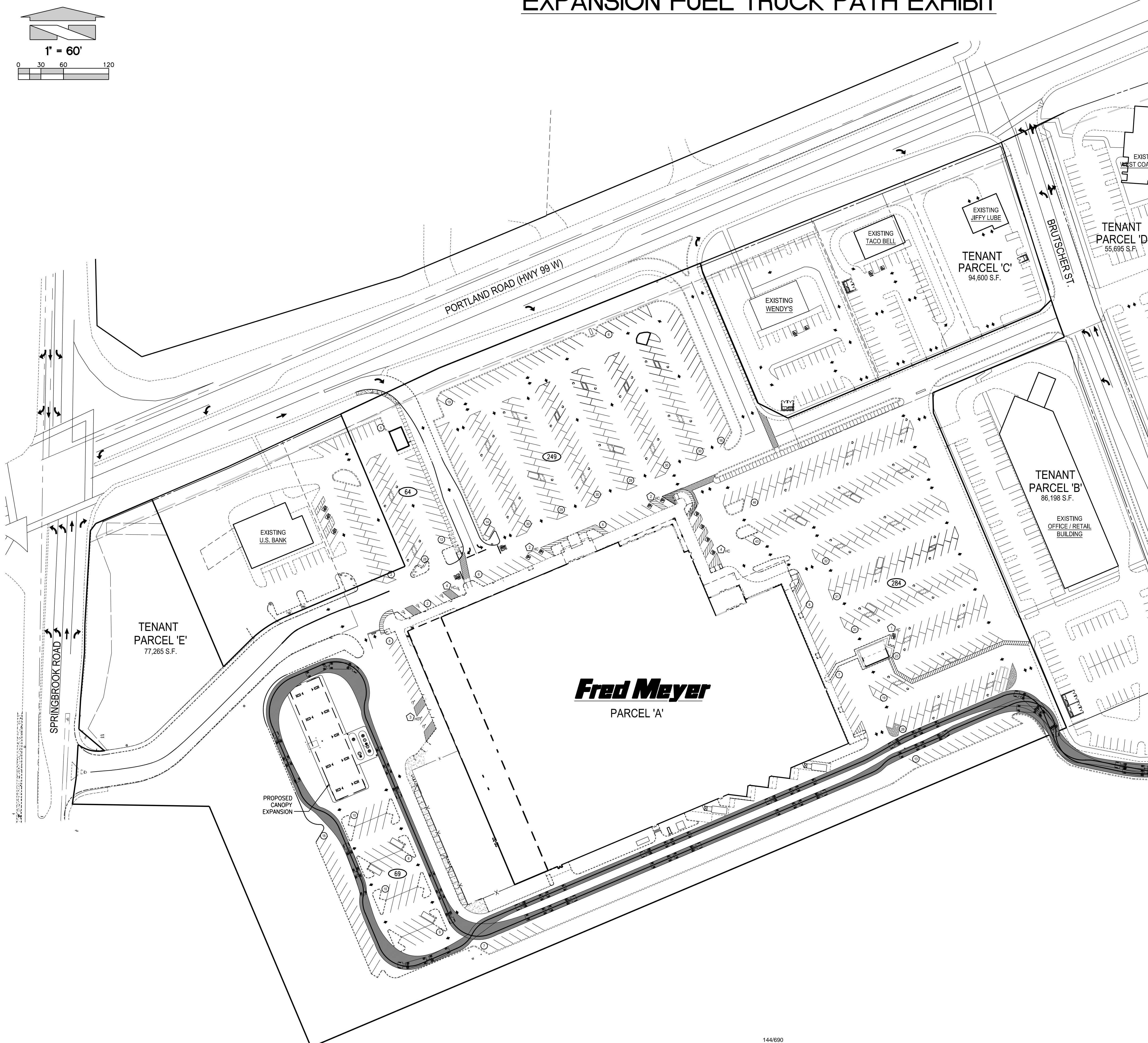
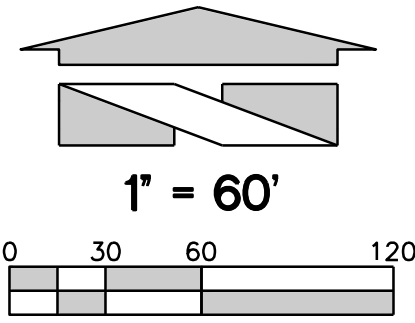
Δ	DATE	DESCRIPTION

PLOTTED ON: Nov 03, 2014 - 4:41pm BY: jastalack
BCE #10315
PM: CHRIS FERKO
DATE: 10/30/14



WB-67D feet
Tractor Width : 8.00 Lock to Lock Time : 6.00
Trailer Width : 8.50 Steering Angle : 15.6
Tractor Track : 8.00 Articulating Angle : 70.0
Trailer Track : 8.50

EXPANSION FUEL TRUCK PATH EXHIBIT



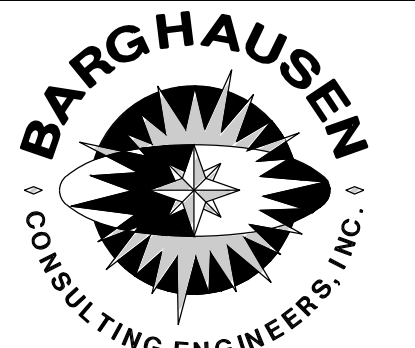
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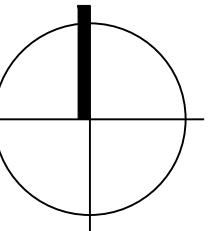
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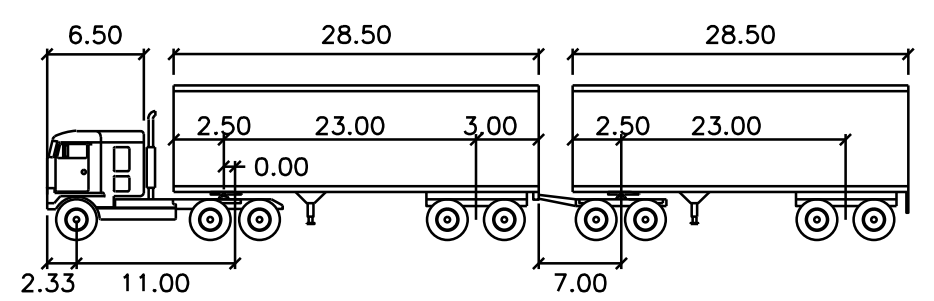


Fred Meyer
PARCEL 'A'

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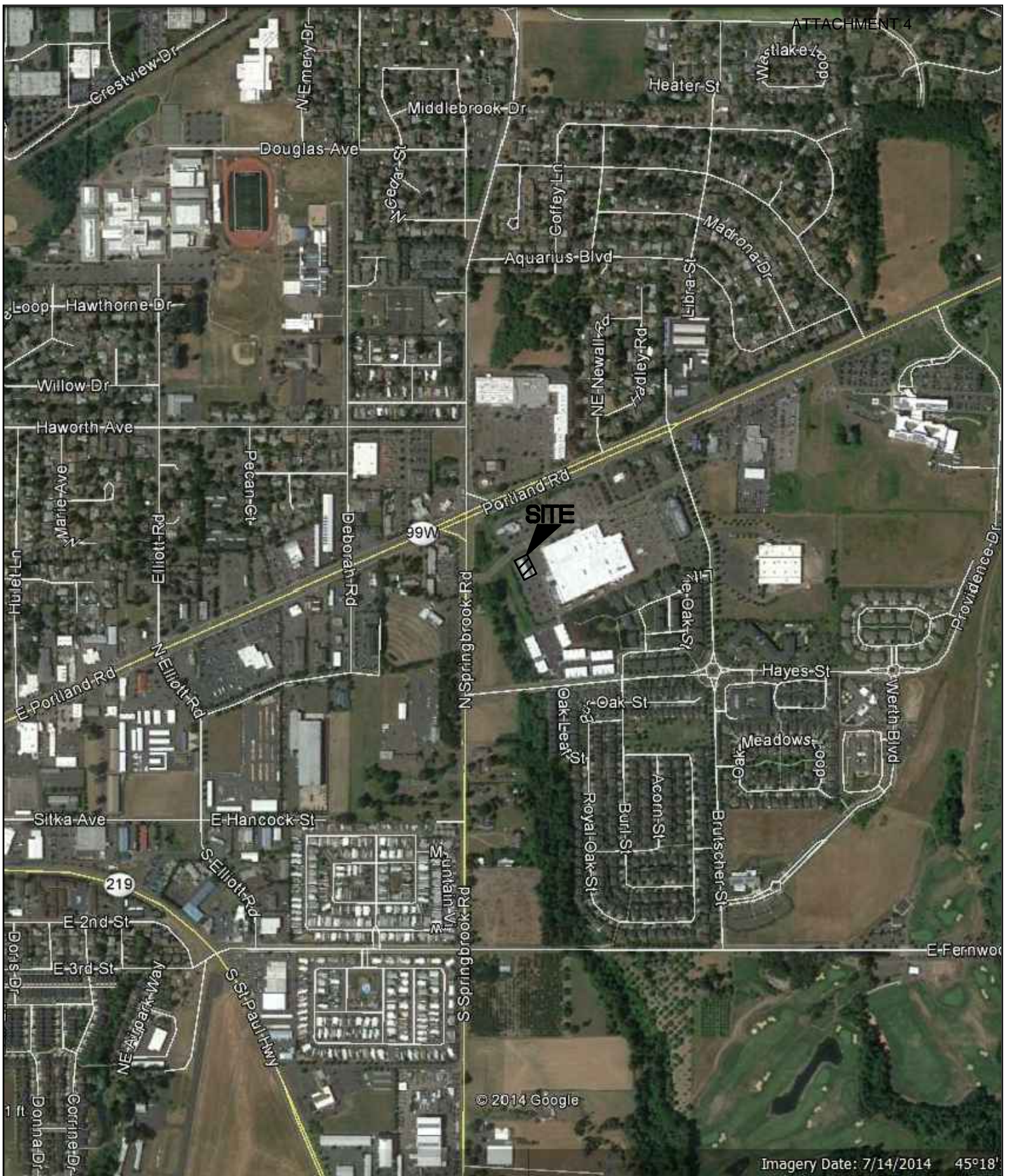
△	DATE	DESCRIPTION

BCE #10315
PM: CHRIS FERKO
DATE: 10/30/14



WB-67D	feet		
Tractor Width	: 8.00	Lock to Lock Time	: 6.00
Tractor Track	: 8.50	Steering Angle	: 15.6
Tractor Wheelbase	: 11.00	Articulating Angle	: 70.0
Trailer Width	: 2.50		
Trailer Track	: 7.00		
Trailer Wheelbase	: 23.00		

APPENDIX B
**TRAFFIC VOLUME
FIGURES**



Imagery Date: 7/14/2014 45°18'



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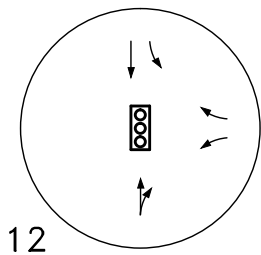
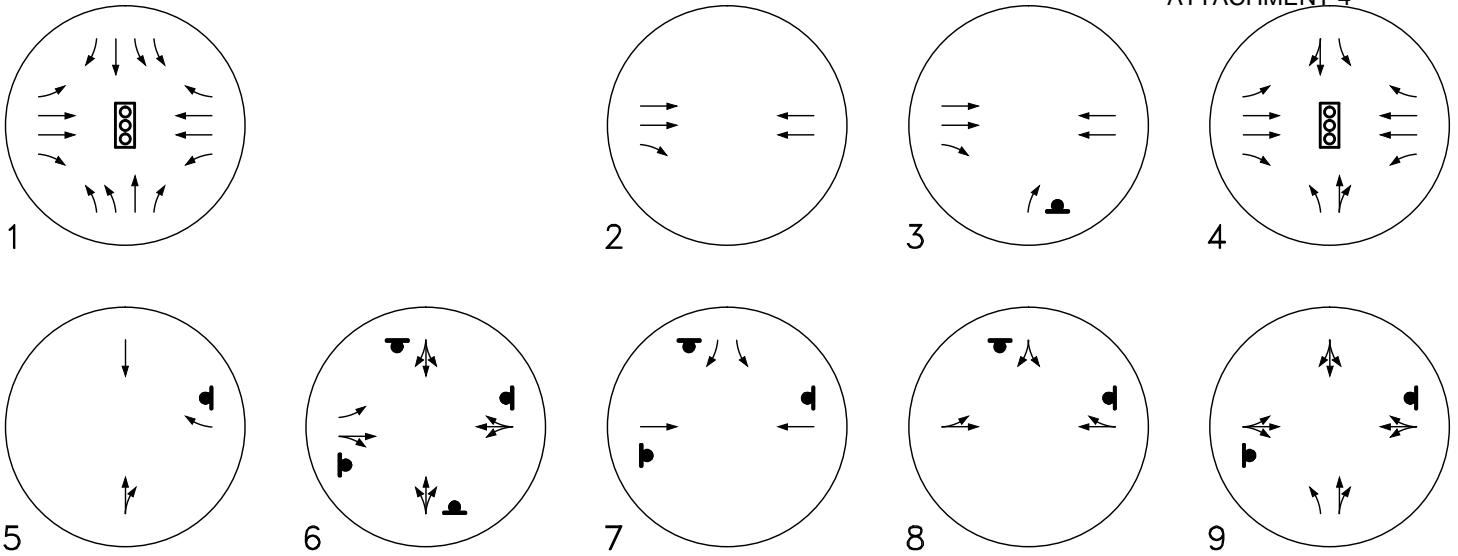
JOB NO:
 2140436.0046/690

VICINITY MAP

FRED MEYER #220 FUEL EXPANSION
NEWBERG, OREGON

FIGURE

1



LEGEND

- LANES/MOVEMENTS
- TRAFFIC SIGNAL CONTROL
- ROUNDABOUT
- STOP SIGN CONTROL
- YIELD SIGN CONTROL



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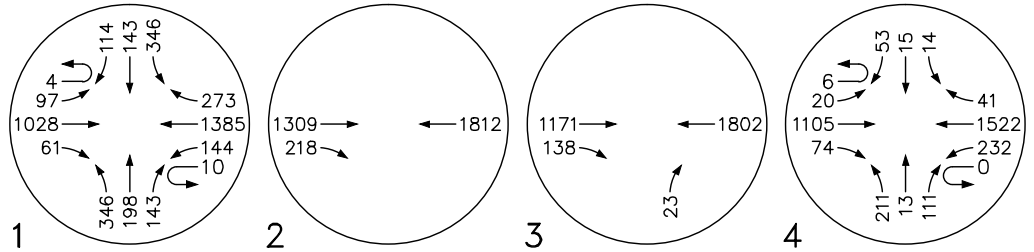
EXISTING LANE CONFIGURATIONS AND TRAFFIC CONTROLS

**FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON**

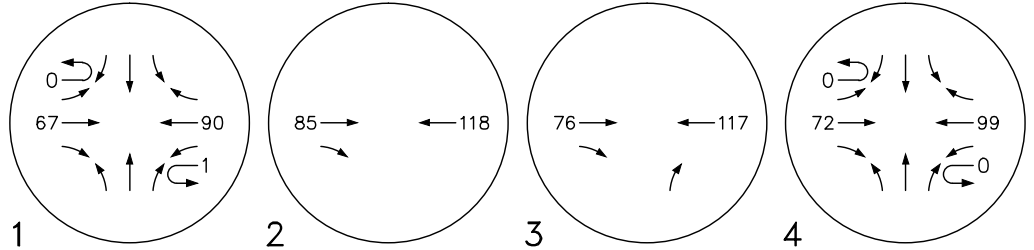
FIGURE

2

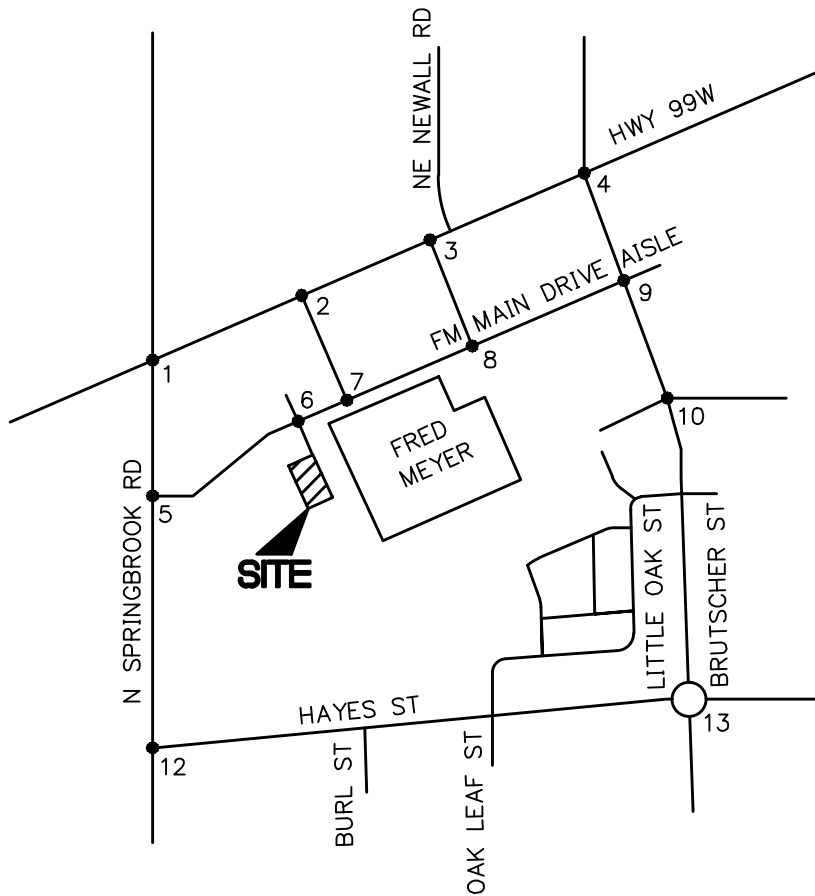
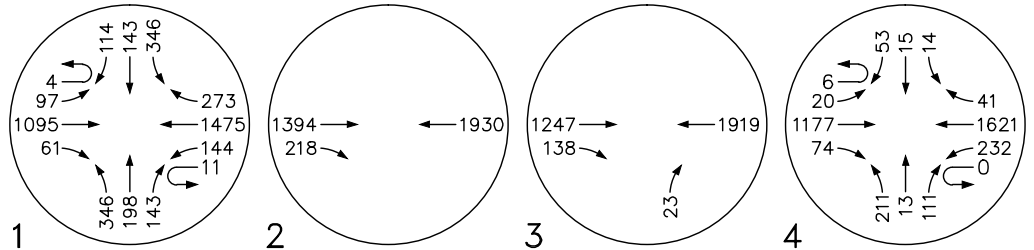
**EXISTING WEEKDAY
PM PEAK HOUR VOLUMES**



SEASONAL ADJUSTMENT



**ADJUSTED EXISTING
30TH HIGHEST WEEKDAY
PM PEAK HOUR VOLUMES**



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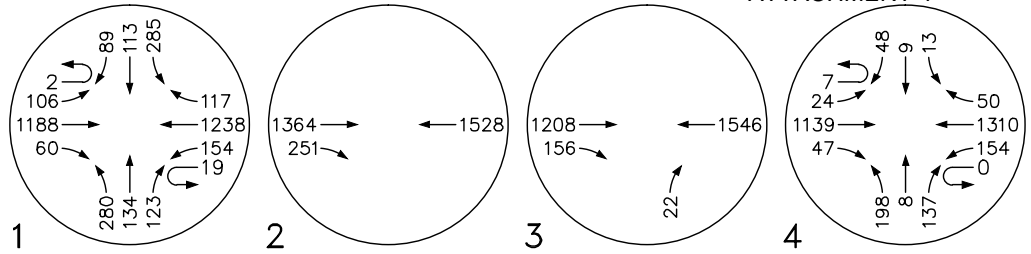
**ODOT 30TH HIGHEST HOUR
 ADJUSTMENT WEEKDAY PM
 PEAK HOUR**

**FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON**

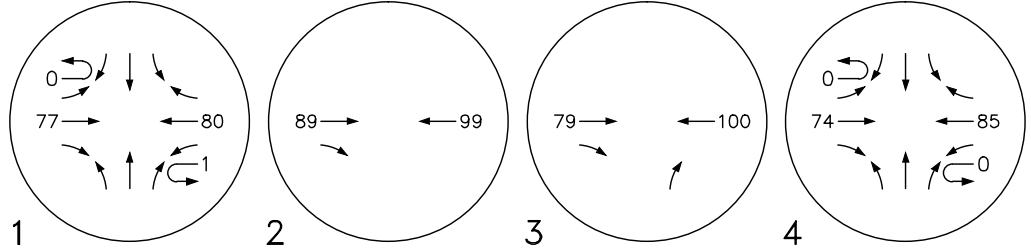
FIGURE

3A

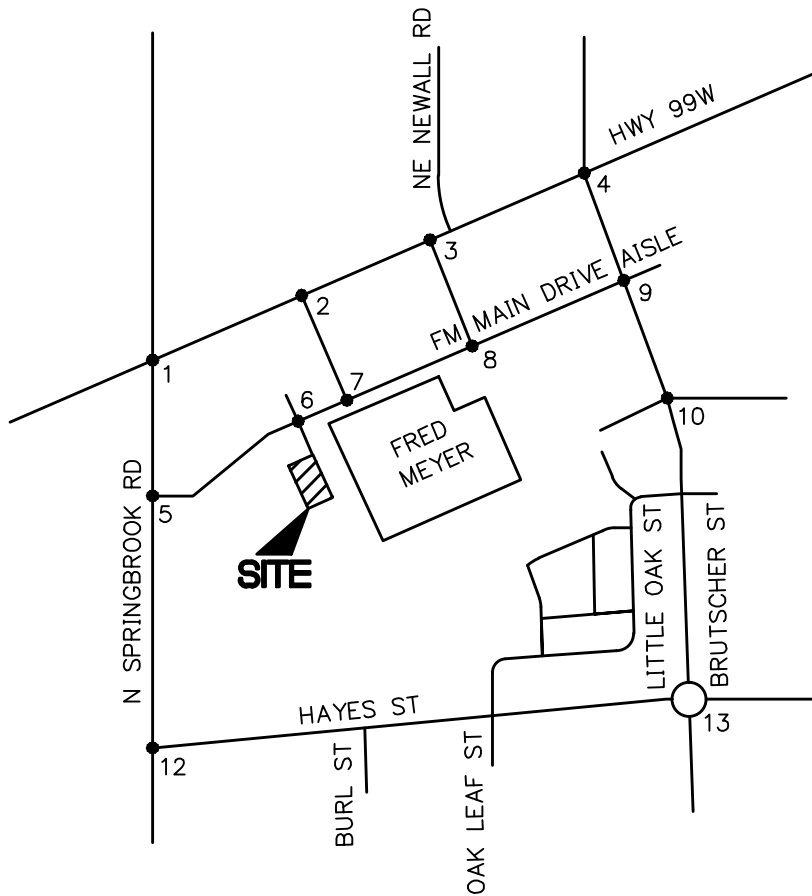
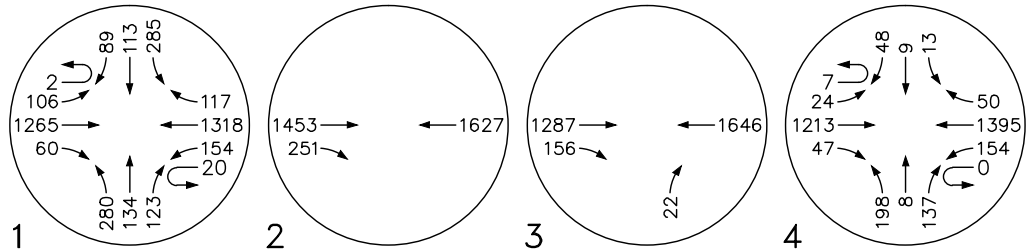
EXISTING SATURDAY MID-DAY PEAK HOUR VOLUMES



SEASONAL ADJUSTMENT



ADJUSTED EXISTING 30TH HIGHEST WEEKDAY PM PEAK HOUR VOLUMES



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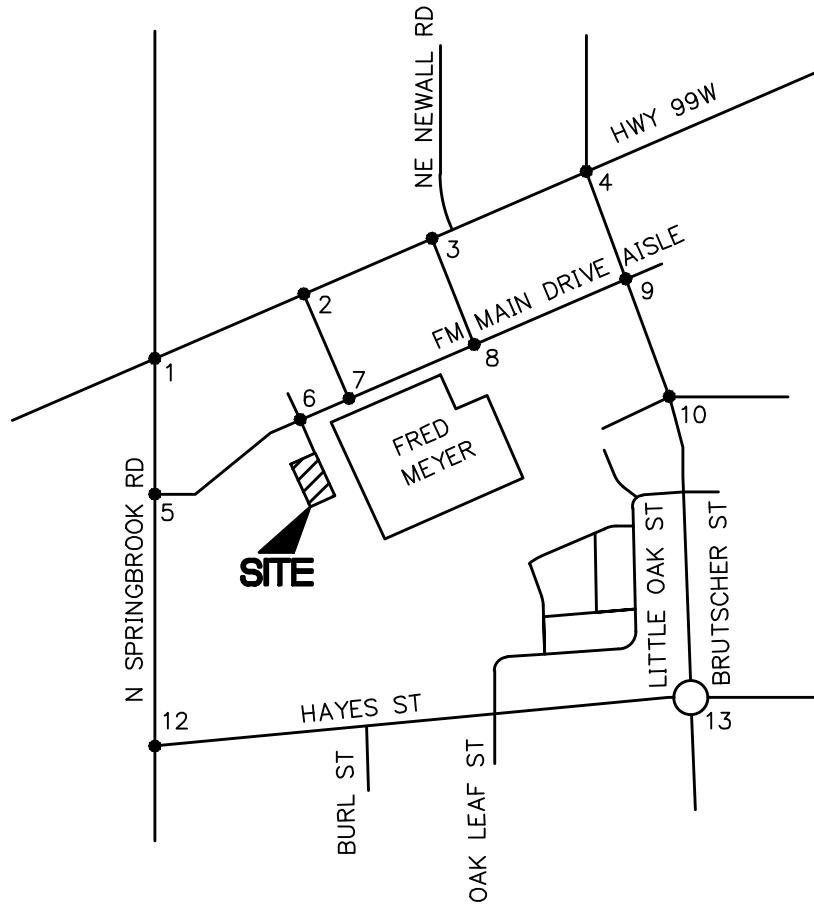
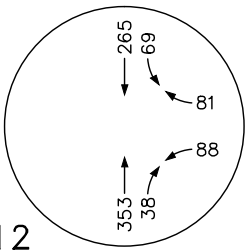
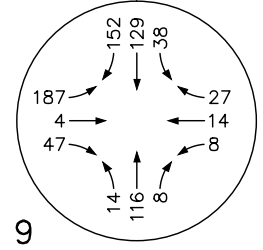
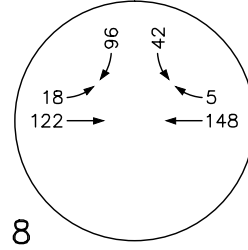
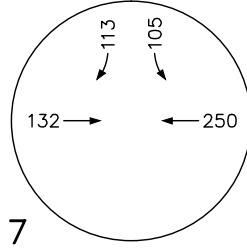
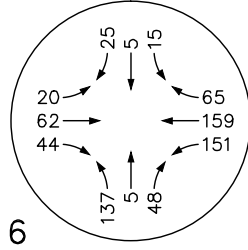
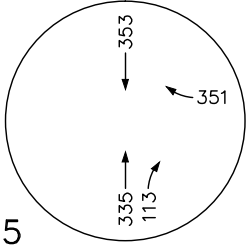
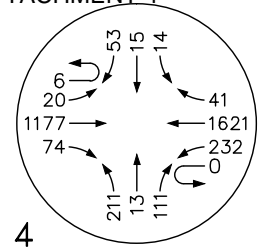
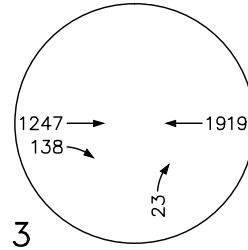
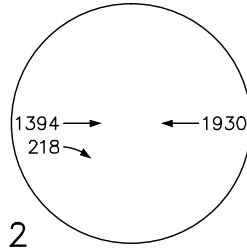
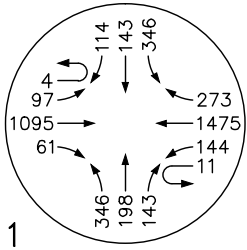
2140436.00 149/690

ODOT 30TH HIGHEST HOUR
 ADJUSTMENT SATURDAY
 MID-DAY PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE

3B



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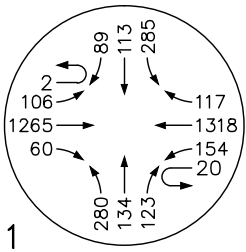
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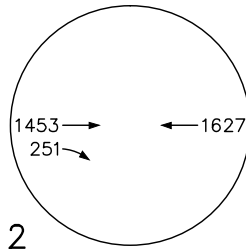
2014 EXISTING 30TH HIGHEST
 HOUR VOLUMES
 WEEKDAY PM PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

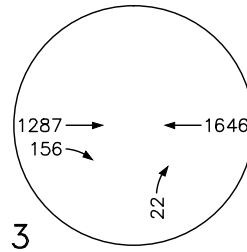
FIGURE
 3C



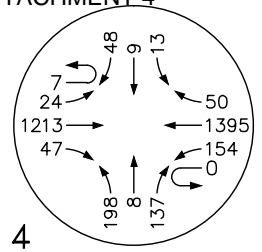
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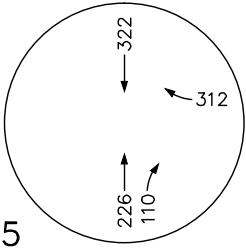
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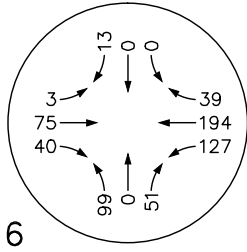
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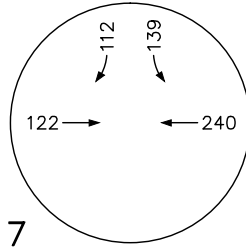
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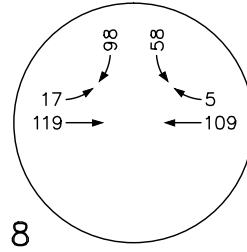
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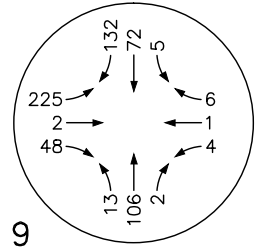
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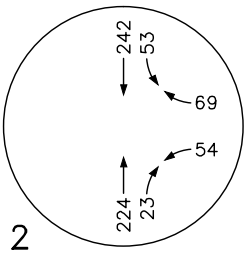
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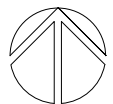
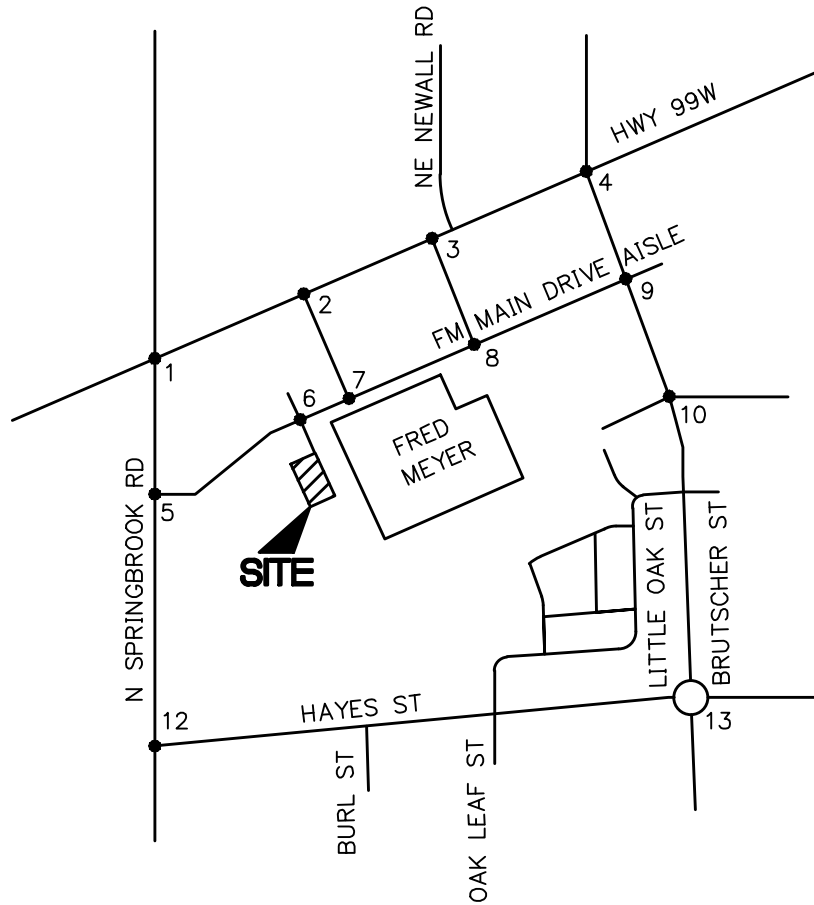
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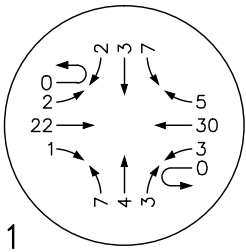
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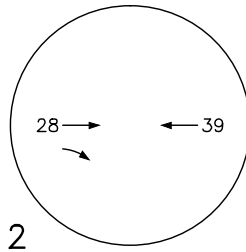
**2014 EXISTING 30TH HIGHEST
 HOUR VOLUMES SATURDAY
 MID-DAY PEAK HOUR**

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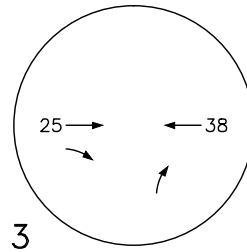
**FIGURE
 3D**



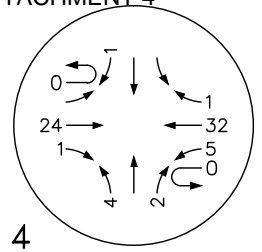
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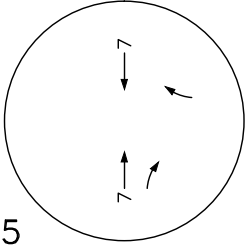
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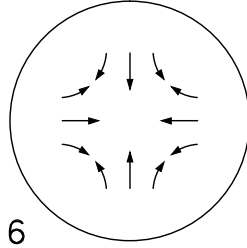
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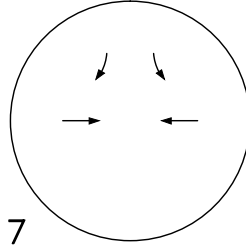
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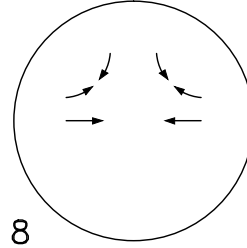
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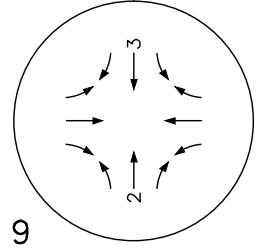
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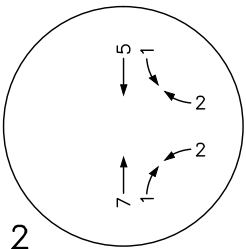
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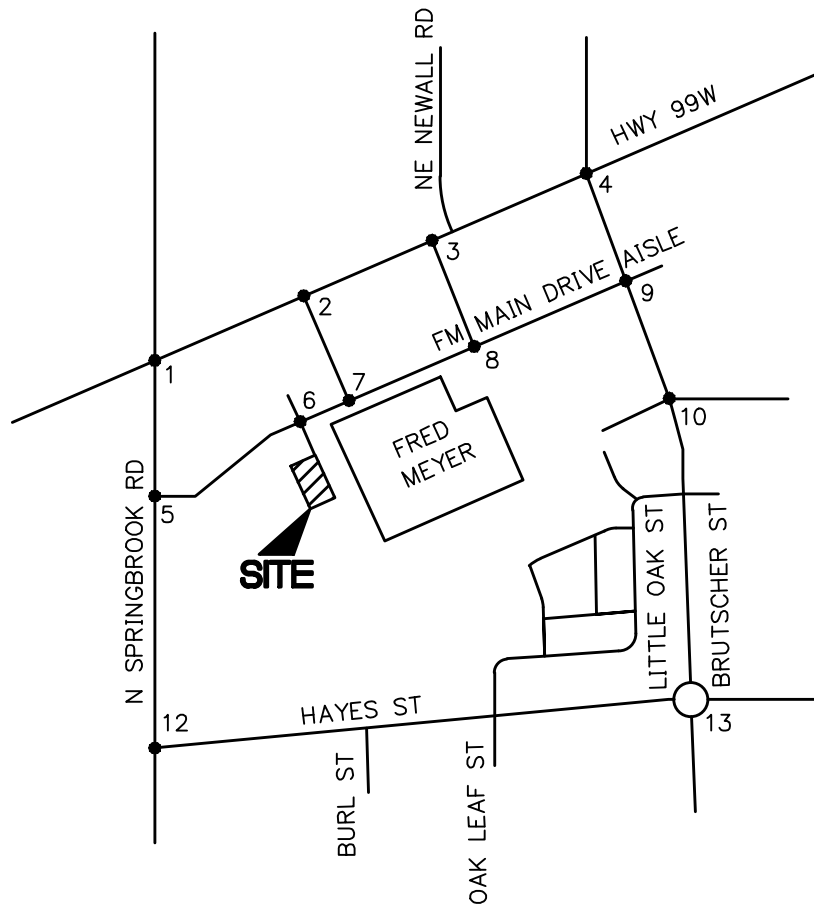
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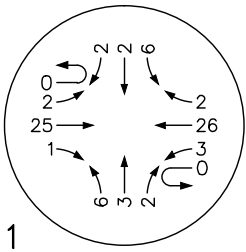
JOB NO:
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2015 BACKGROUND GROWTH
 1 YEAR AT 2% PER YEAR
 WEEKDAY PM PEAK HOUR

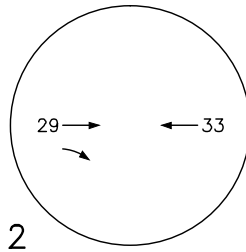
FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE

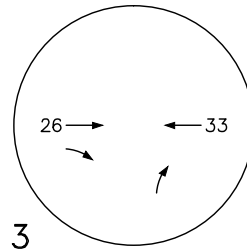
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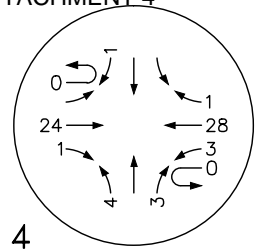
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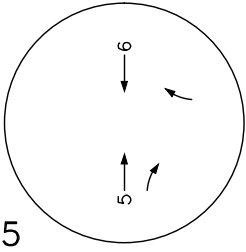
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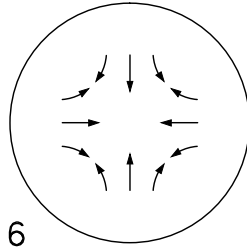
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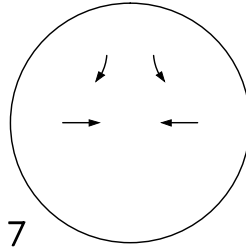
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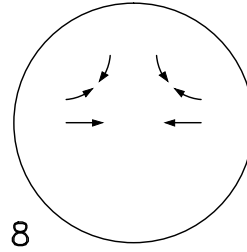
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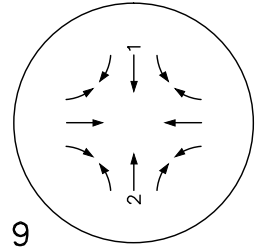
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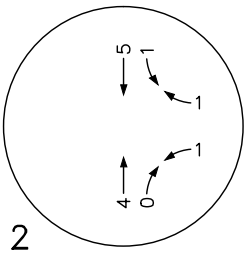
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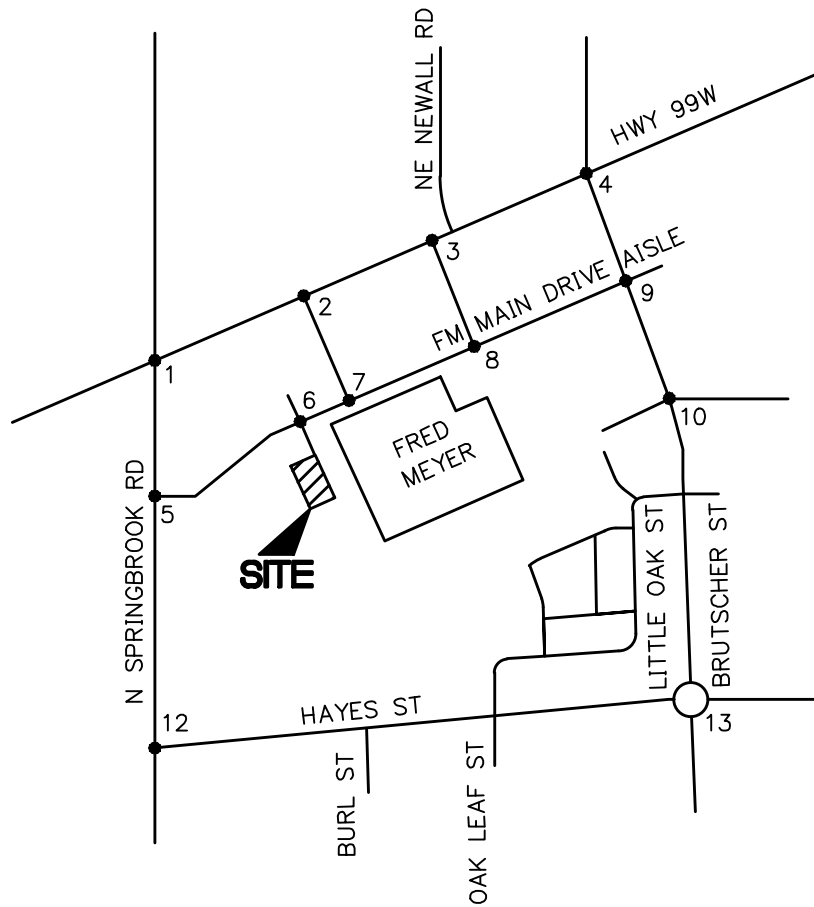
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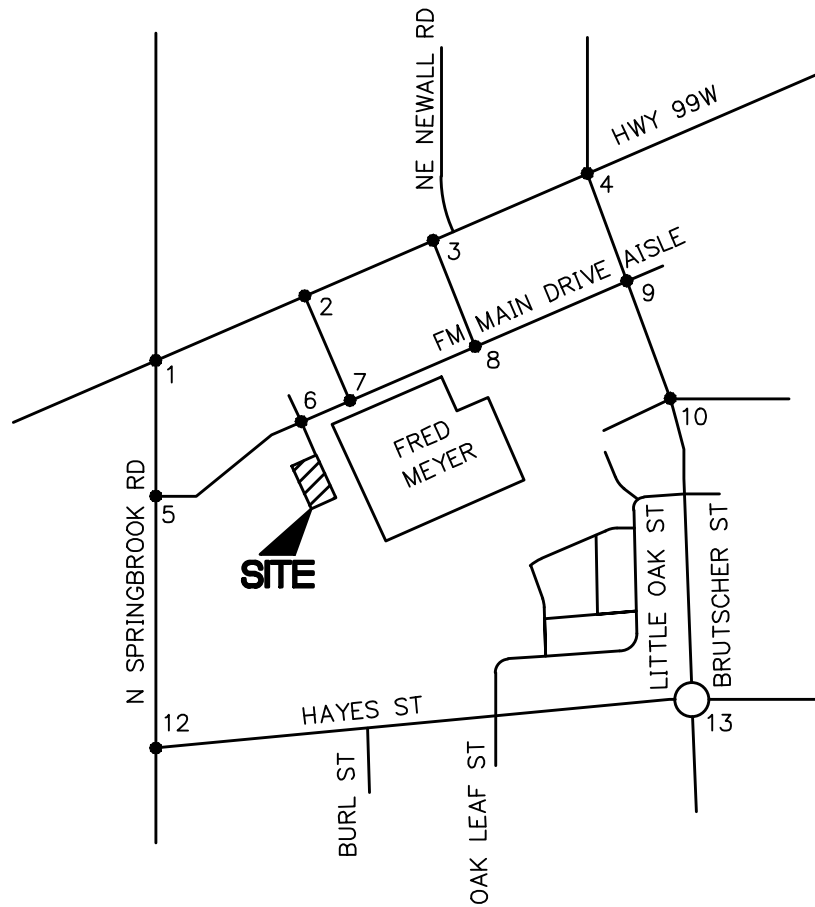
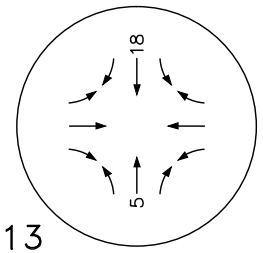
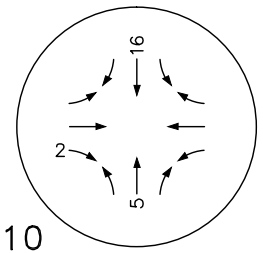
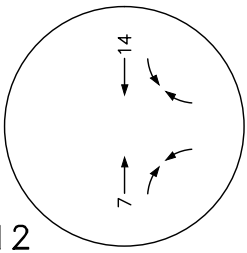
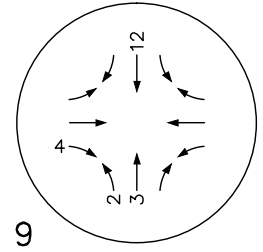
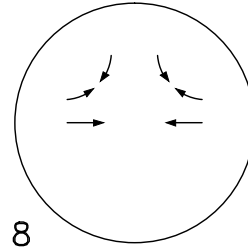
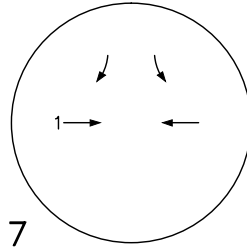
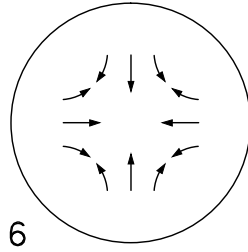
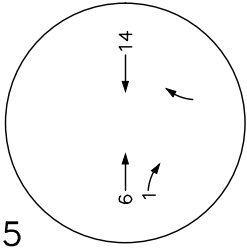
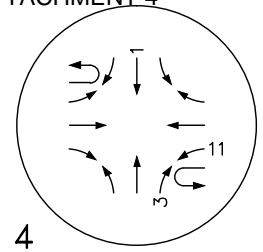
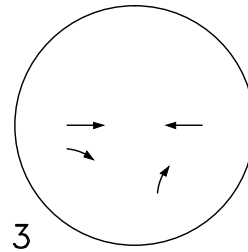
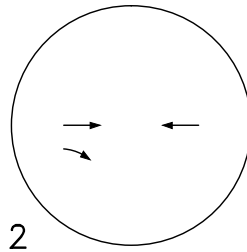
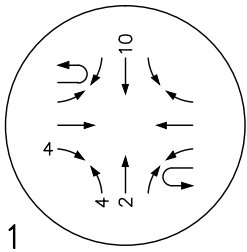
JOB NO:
 2140436.0053/690

2015 BACKGROUND GROWTH
 1 YEAR AT 2% PER YEAR
 SATURDAY MID-DAY PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE

4B



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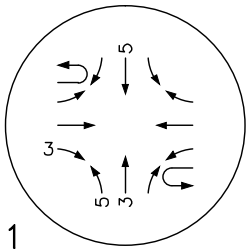
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JOB NO:
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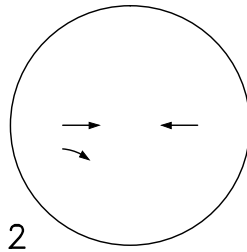
IN-PROCESS TRIPS
 WEEKDAY PM PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

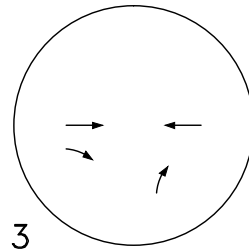
FIGURE
 5A



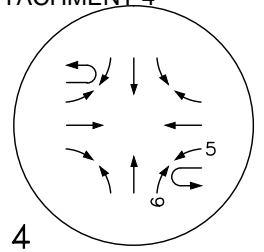
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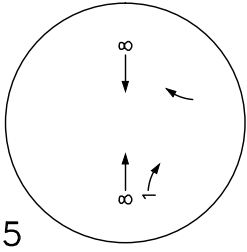
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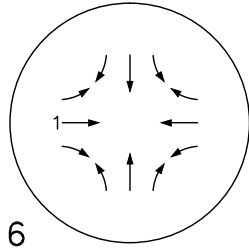
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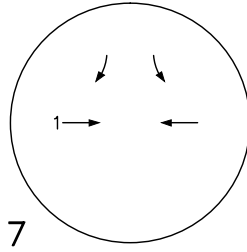
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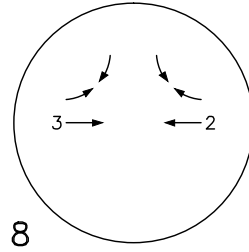
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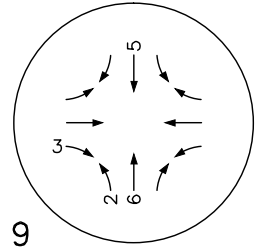
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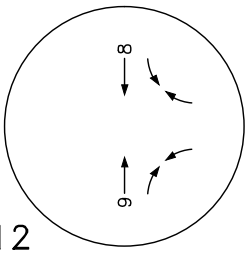
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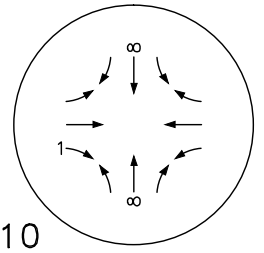
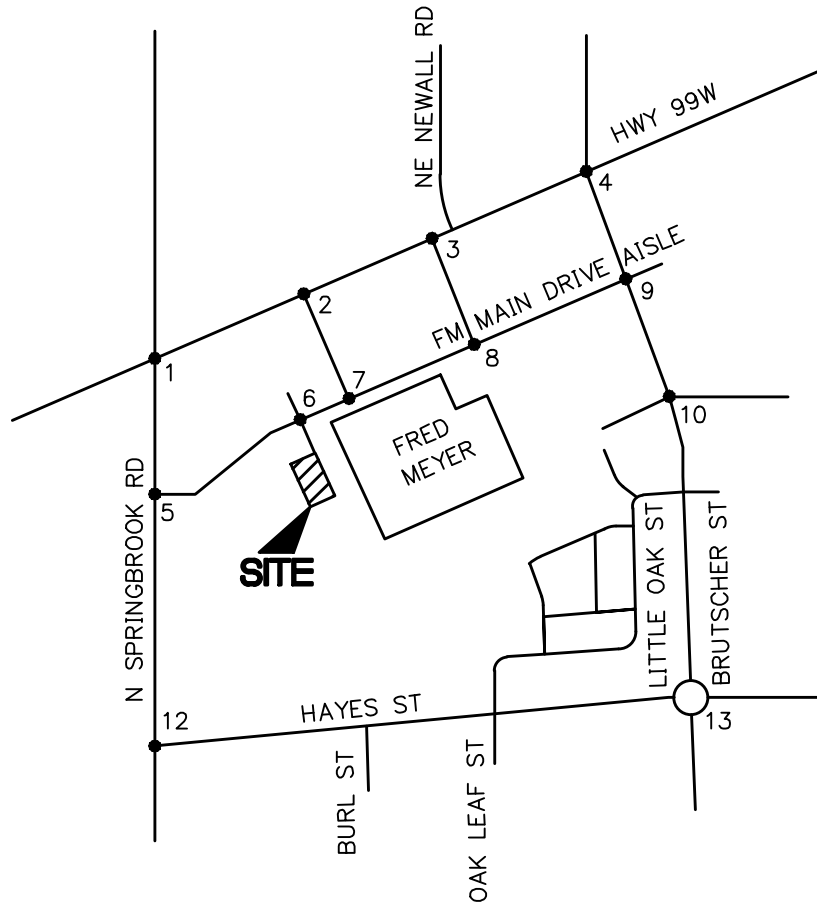
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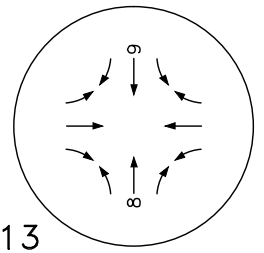
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12



10



13



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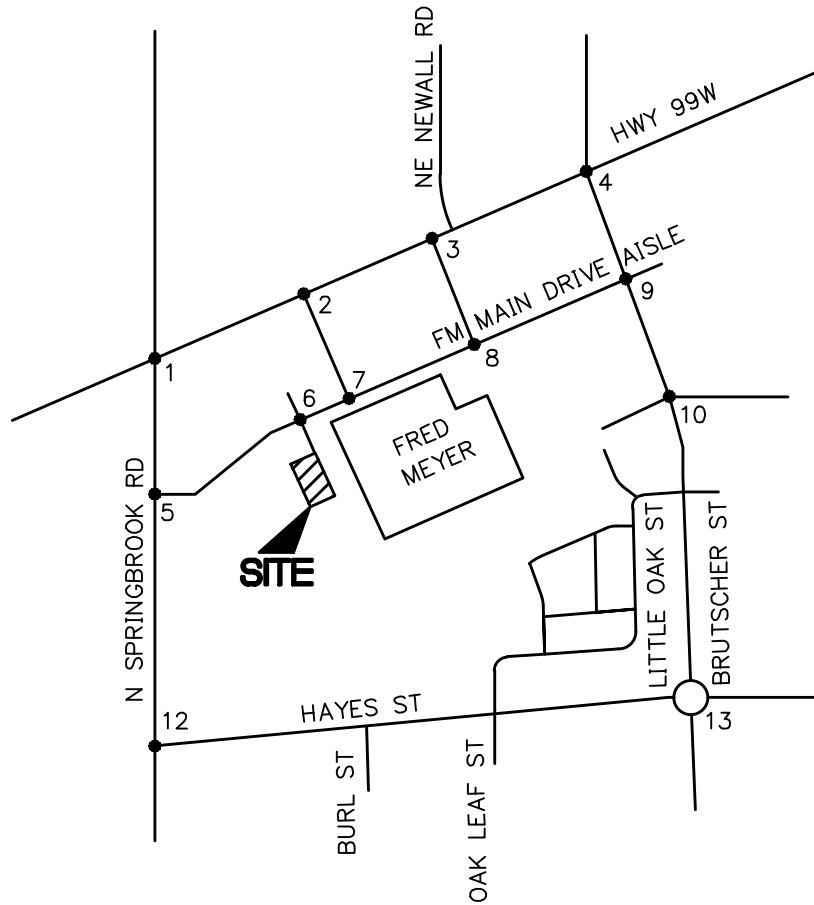
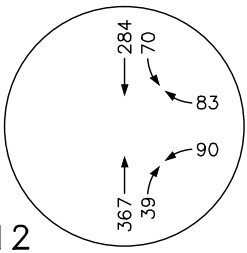
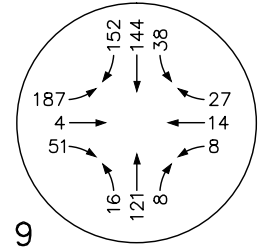
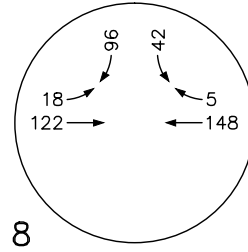
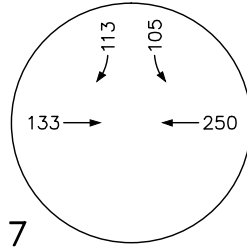
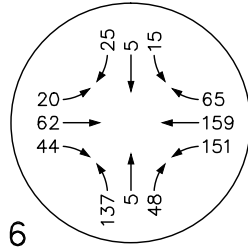
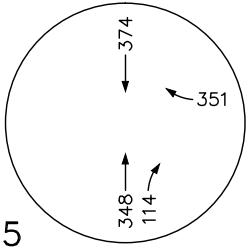
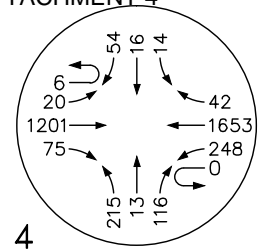
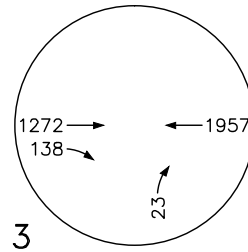
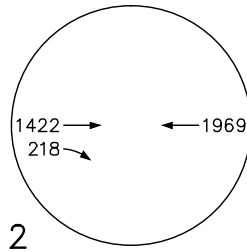
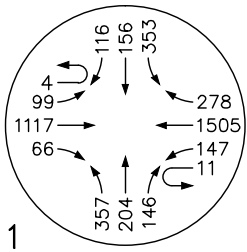
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IN-PROCESS TRIPS
 SATURDAY MID-DAY PEAK
 HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE
5B



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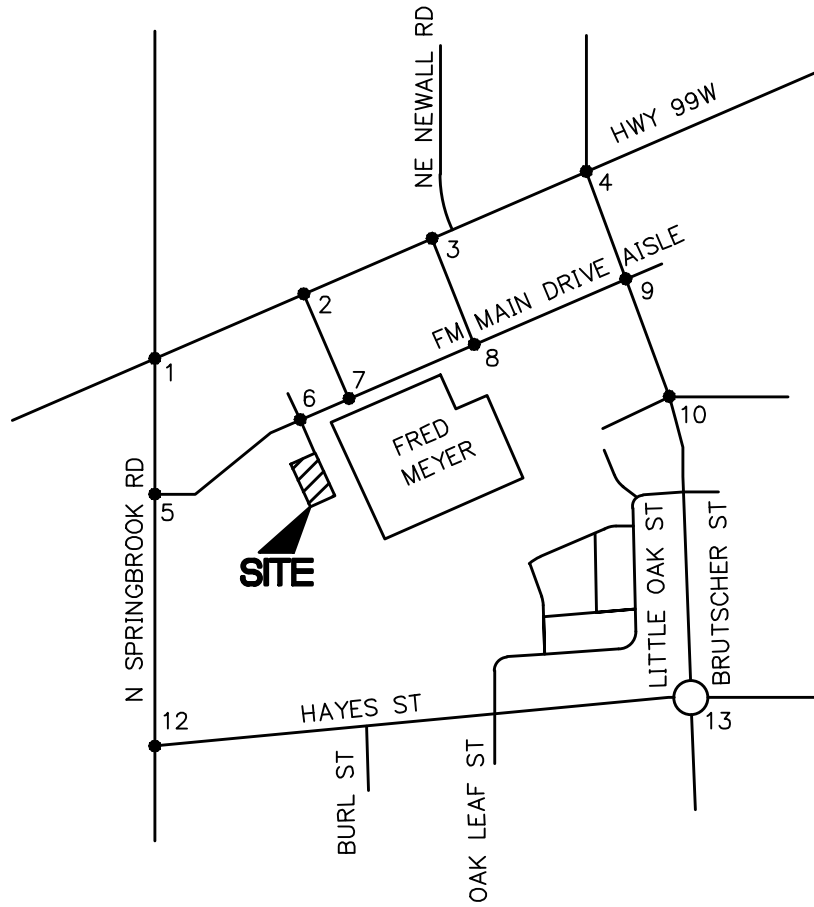
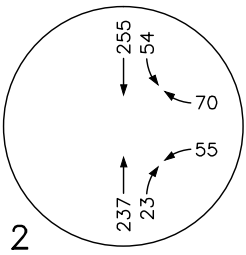
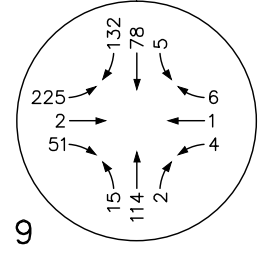
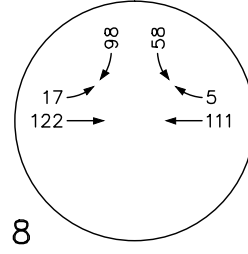
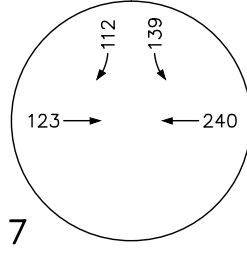
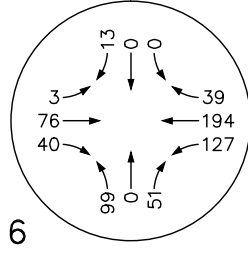
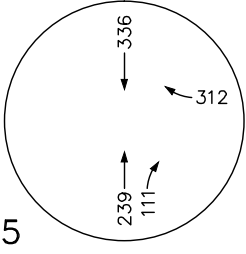
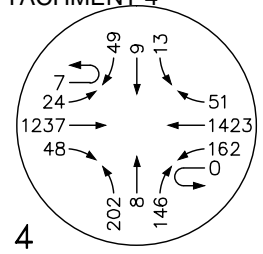
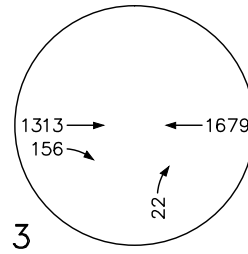
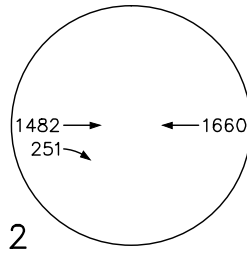
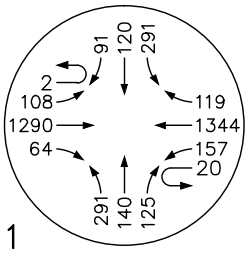
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2015 PRE-DEVELOPMENT
 VOLUMES WEEKDAY PM PEAK
 HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE
 6A



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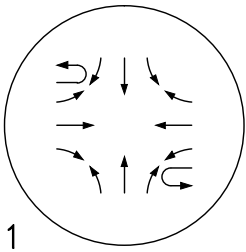
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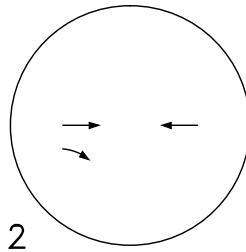
2015 PRE-DEVELOPMENT
 VOLUMESSATURDAY MID-DAY
 PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

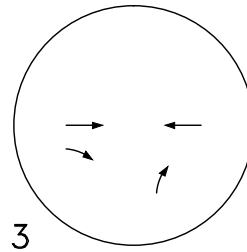
FIGURE
 6B



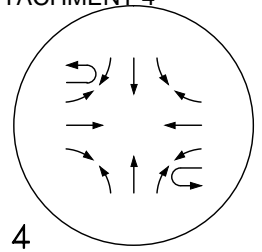
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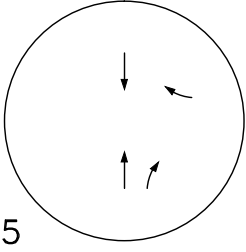
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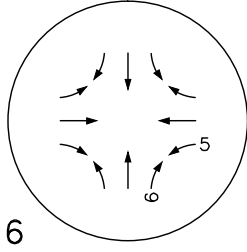
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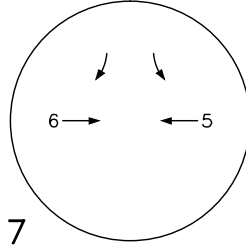
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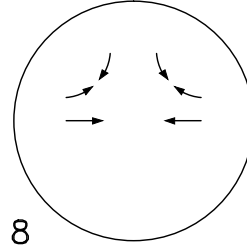
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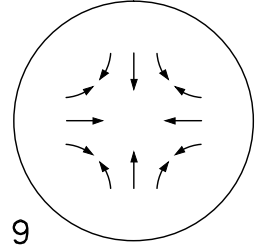
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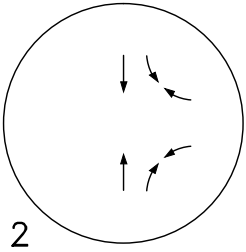
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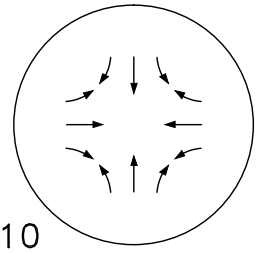
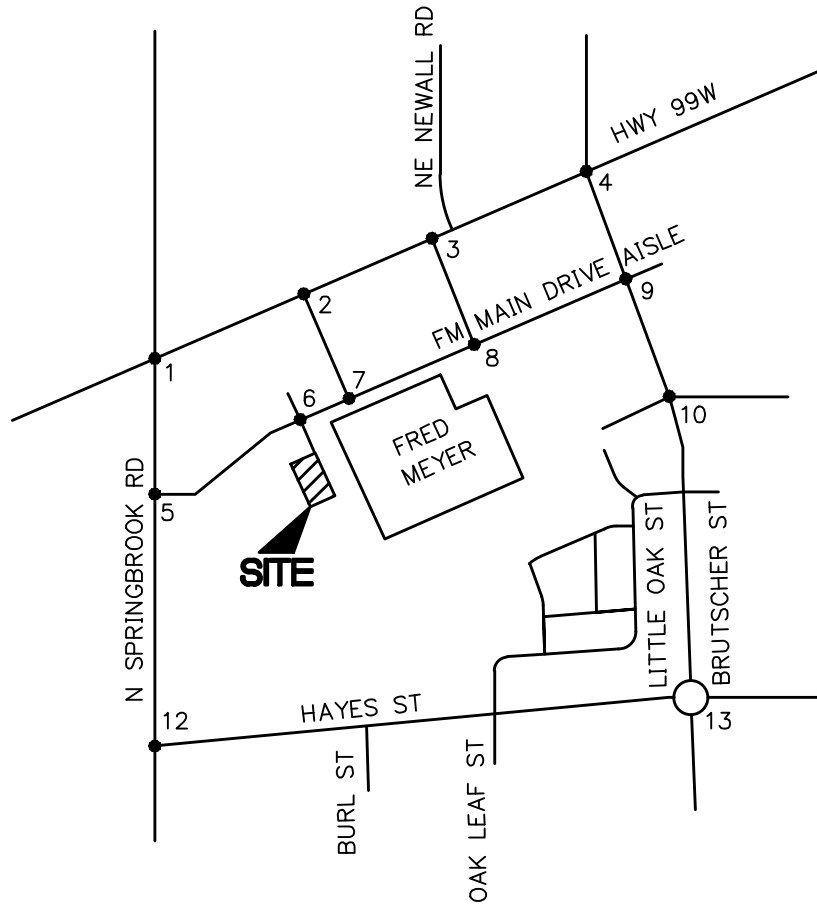
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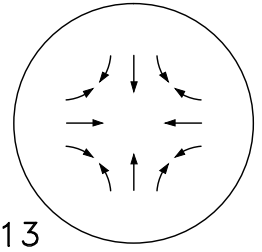
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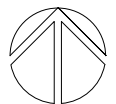
12



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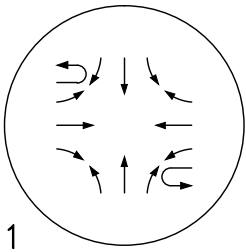
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SITE SHARED TRIP ASSIGNMENT
 WEEKDAY PM PEAK HOUR

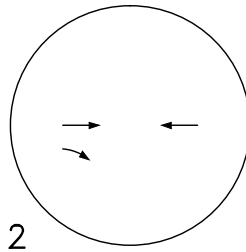
FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE

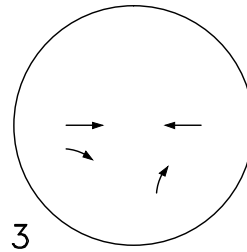
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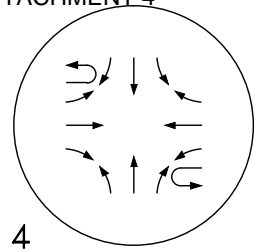
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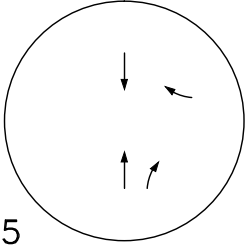
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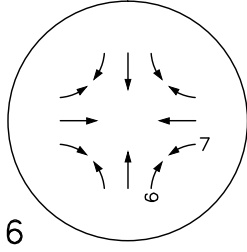
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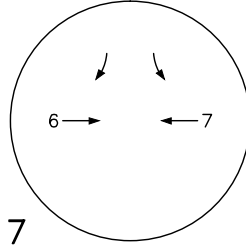
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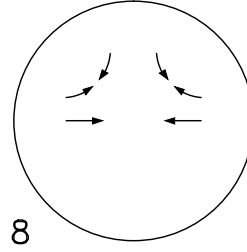
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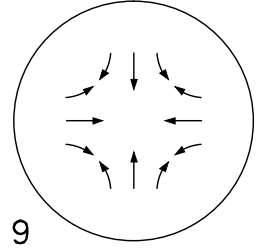
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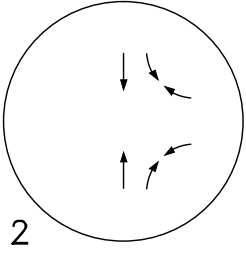
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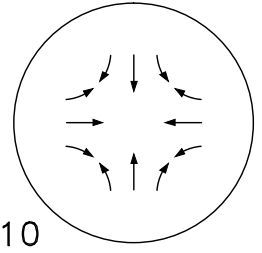
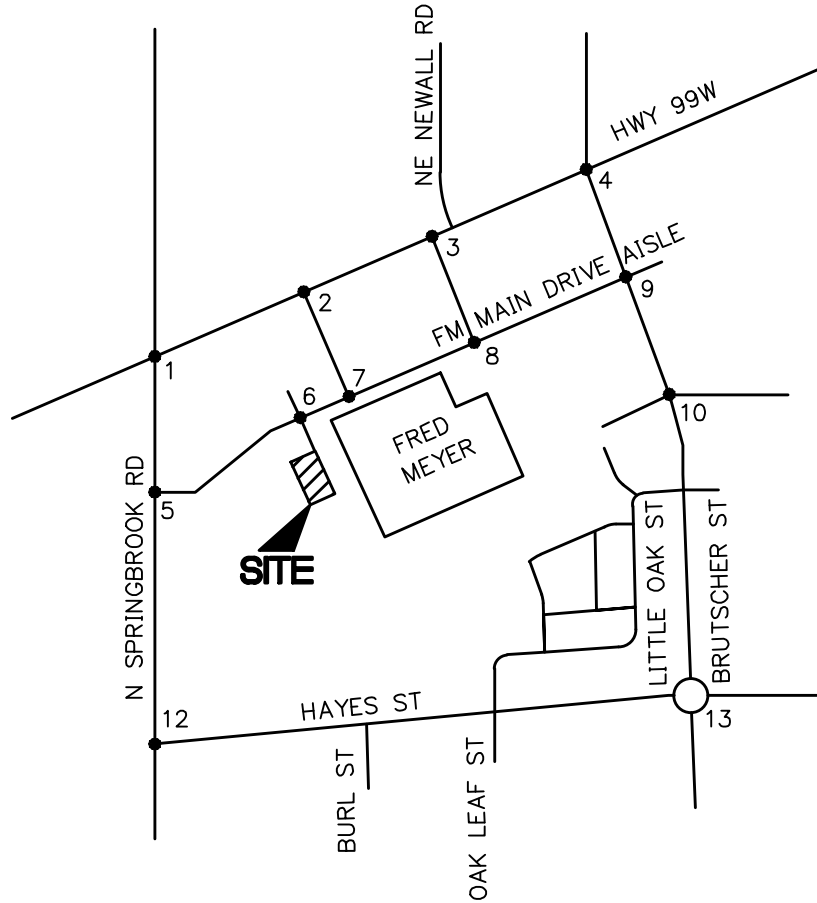
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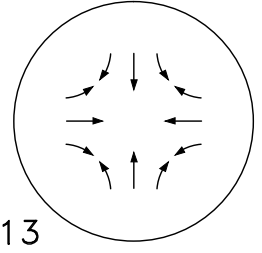
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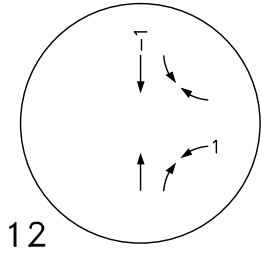
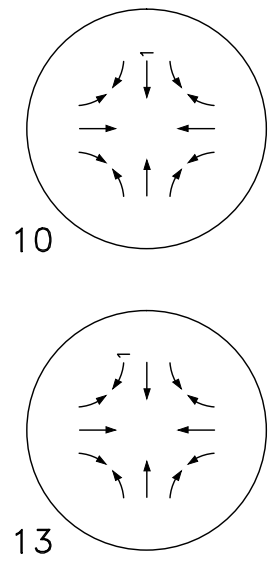
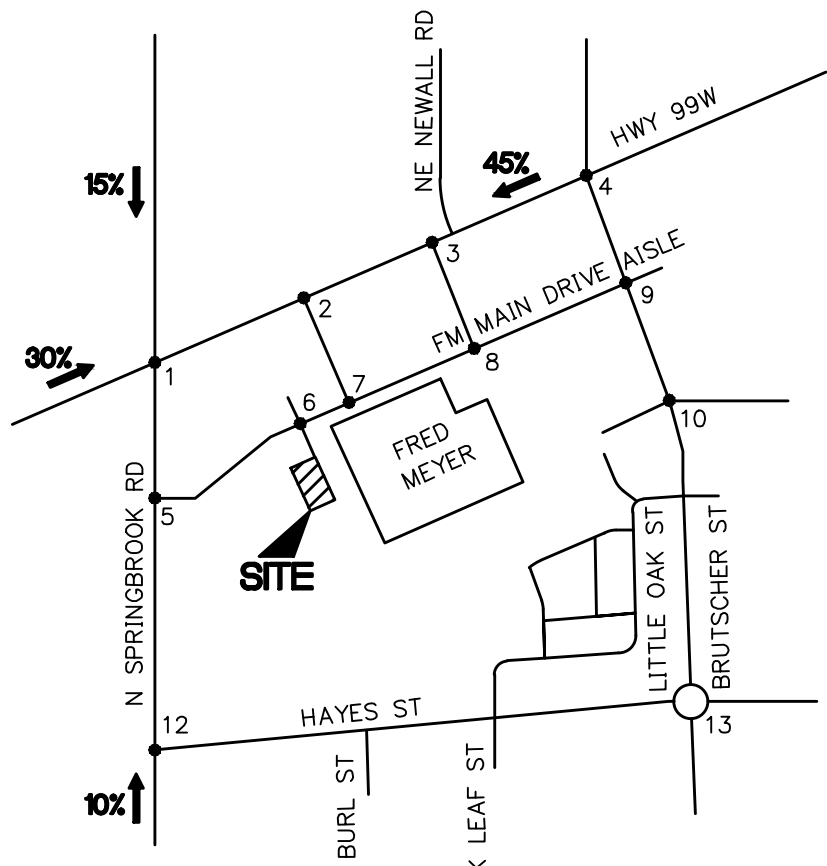
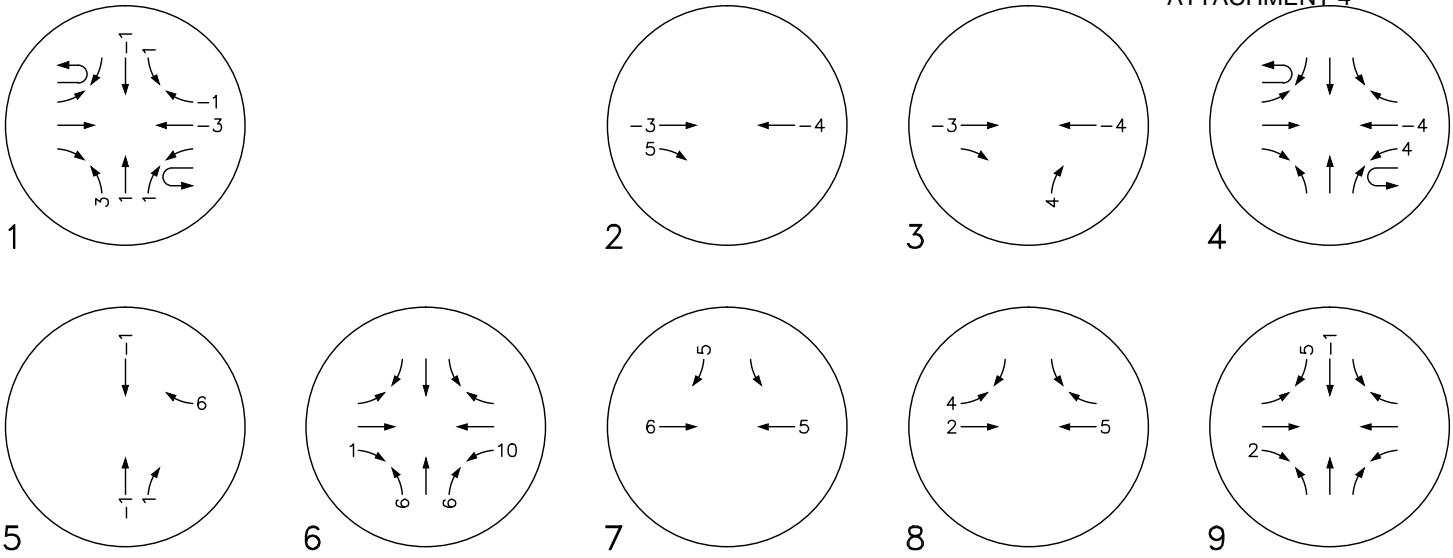
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SITE SHARED TRIP ASSIGNMENT
 SATURDAY MID-DAY PEAK
 HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE
 7B



WEEKDAY PM PEAK HOUR
SITE PASS-BY TRIPS

ENTER = 11
EXIT = 12
TOTAL = 23



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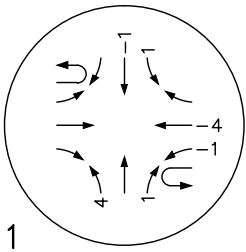
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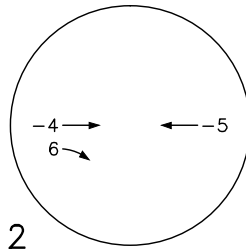
SITE PASS-BY TRIP
DISTRIBUTION AND ASSIGNMENT
WEEKDAY PM PEAK HOUR
FRED MEYER #220 FUEL EXPANSION
NEWBERG, OREGON

FIGURE
8A

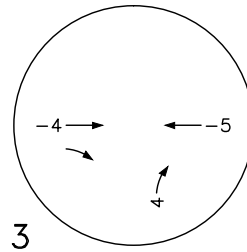
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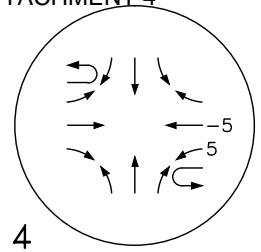
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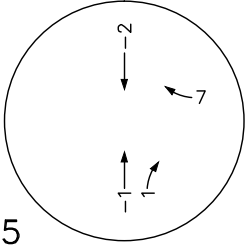
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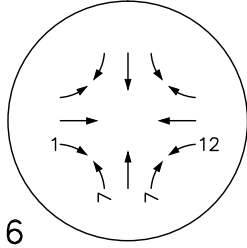
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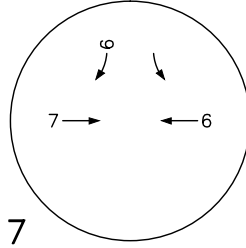
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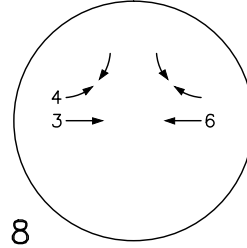
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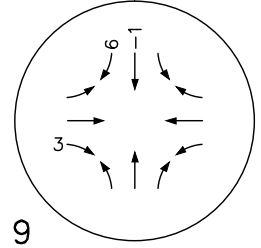
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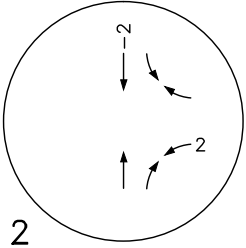
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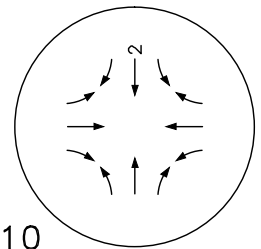
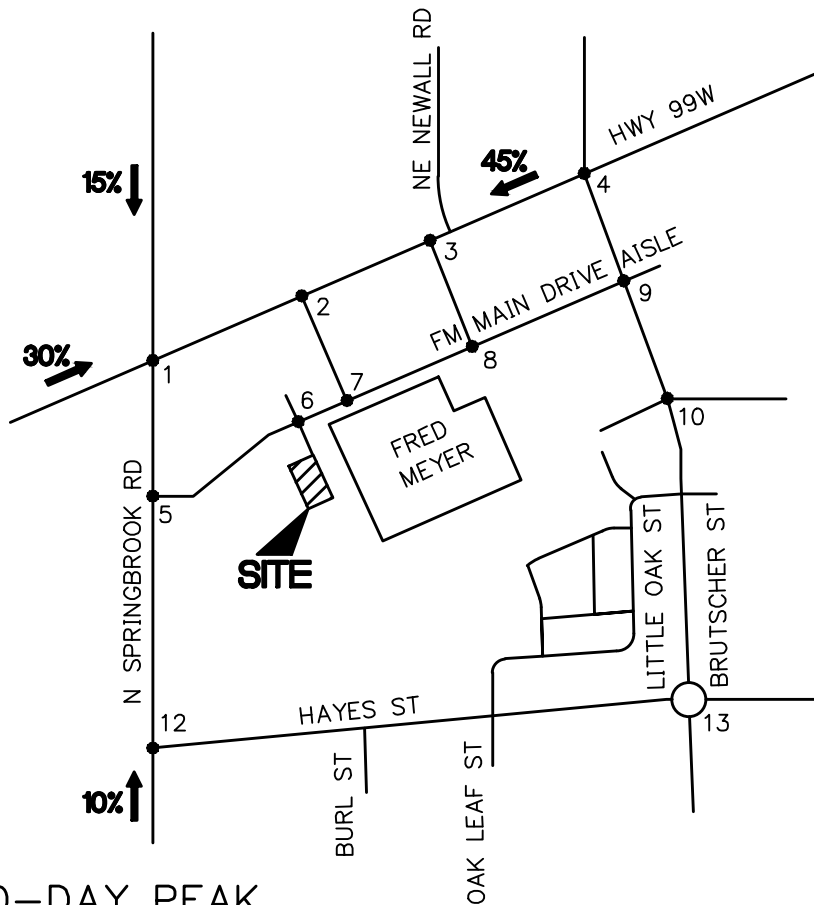
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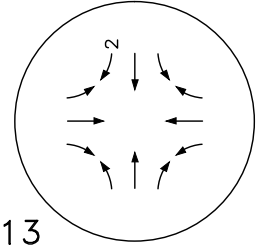
9



12



10



13

SATURDAY MID-DAY PEAK HOUR SITE PASS-BY TRIPS

ENTER = 13

EXIT = 14

TOTAL = 27



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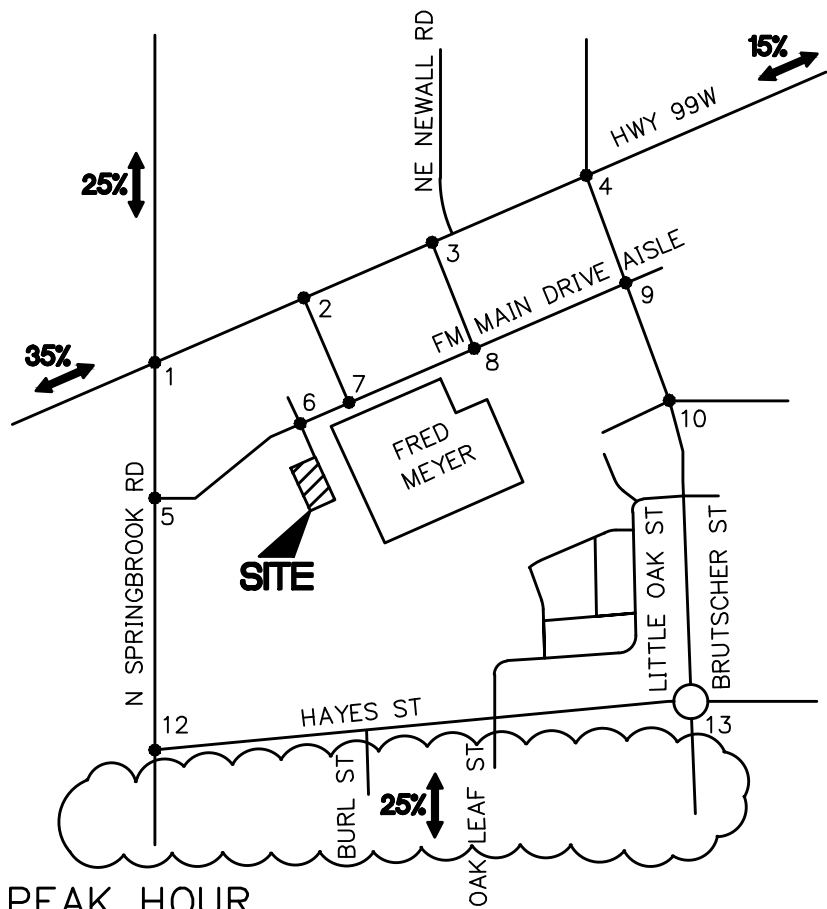
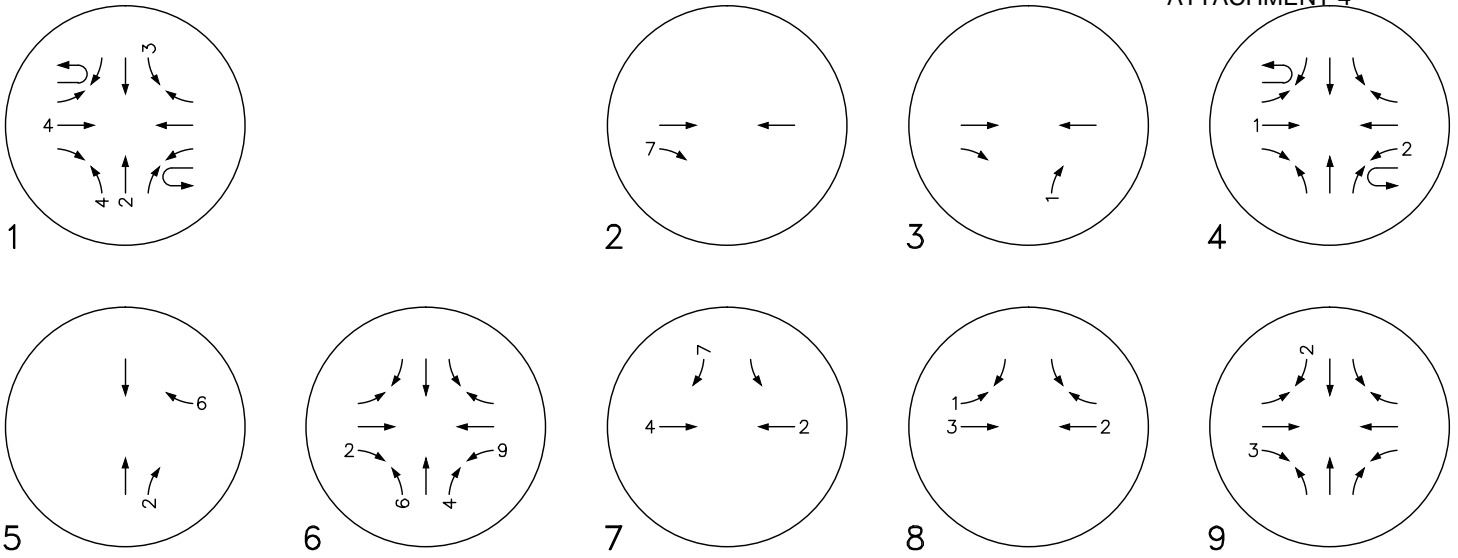
JOB NO:
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SITE PASS-BY TRIP DISTRIBUTION
AND ASSIGNMENT SATURDAY MID-DAY
PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
NEWBERG, OREGON

FIGURE

8B



WEEKDAY PM PEAK HOUR
SITE PRIMARY TRIPS

ENTER = 11
EXIT = 10
TOTAL = 21



NOT TO SCALE



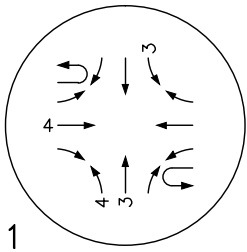
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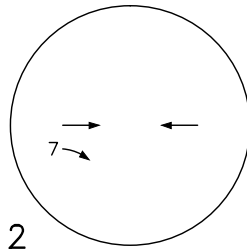
SITE PRIMARY TRIP
DISTRIBUTION AND ASSIGNMENT
WEEKDAY PM PEAK HOUR
FRED MEYER #220 FUEL EXPANSION
NEWBERG, OREGON

FIGURE
9A

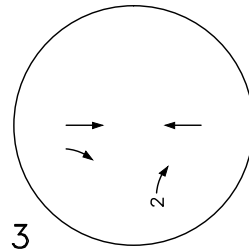
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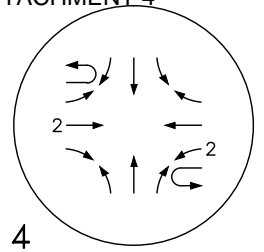
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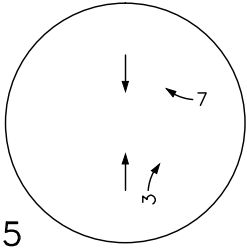
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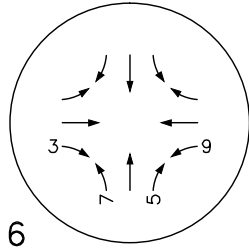
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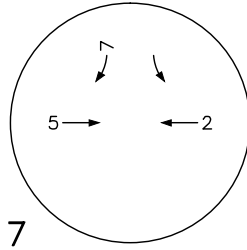
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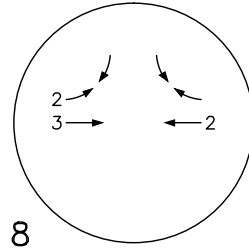
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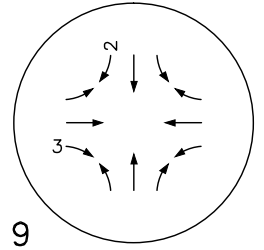
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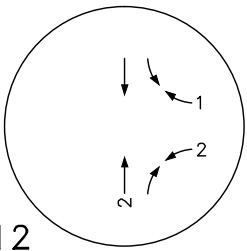
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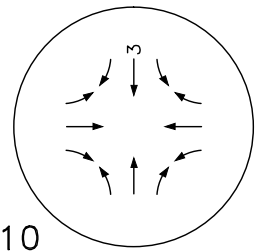
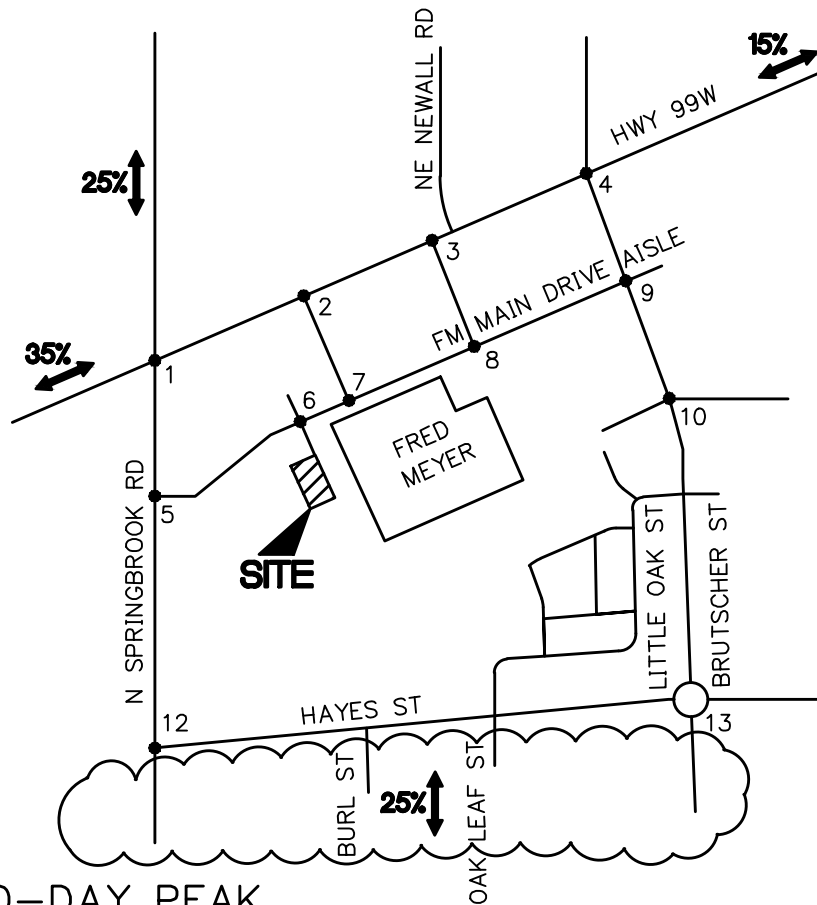
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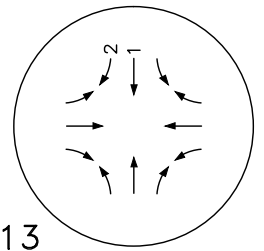
9



12



10



13

SATURDAY MID-DAY PEAK HOUR SITE PRIMARY TRIPS

ENTER = 12

EXIT = 12

TOTAL = 24



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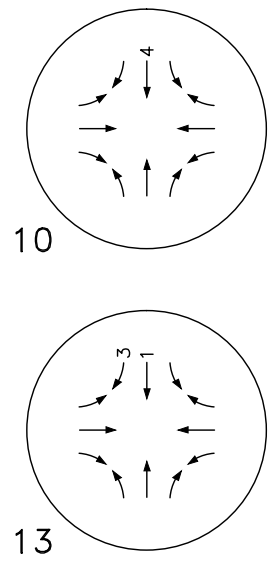
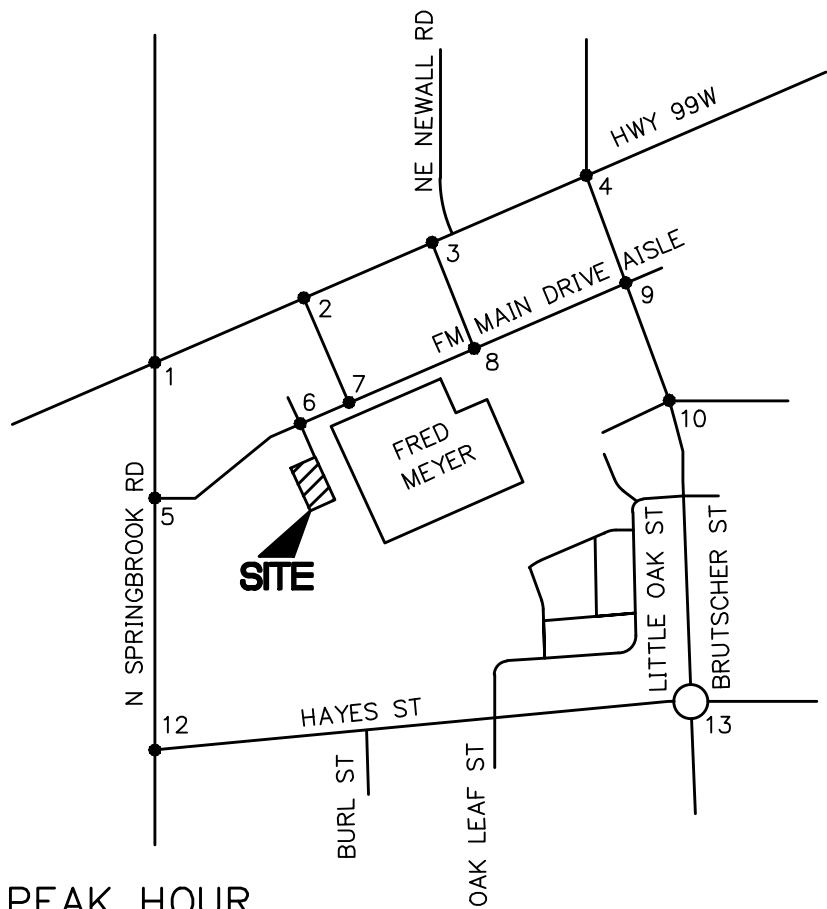
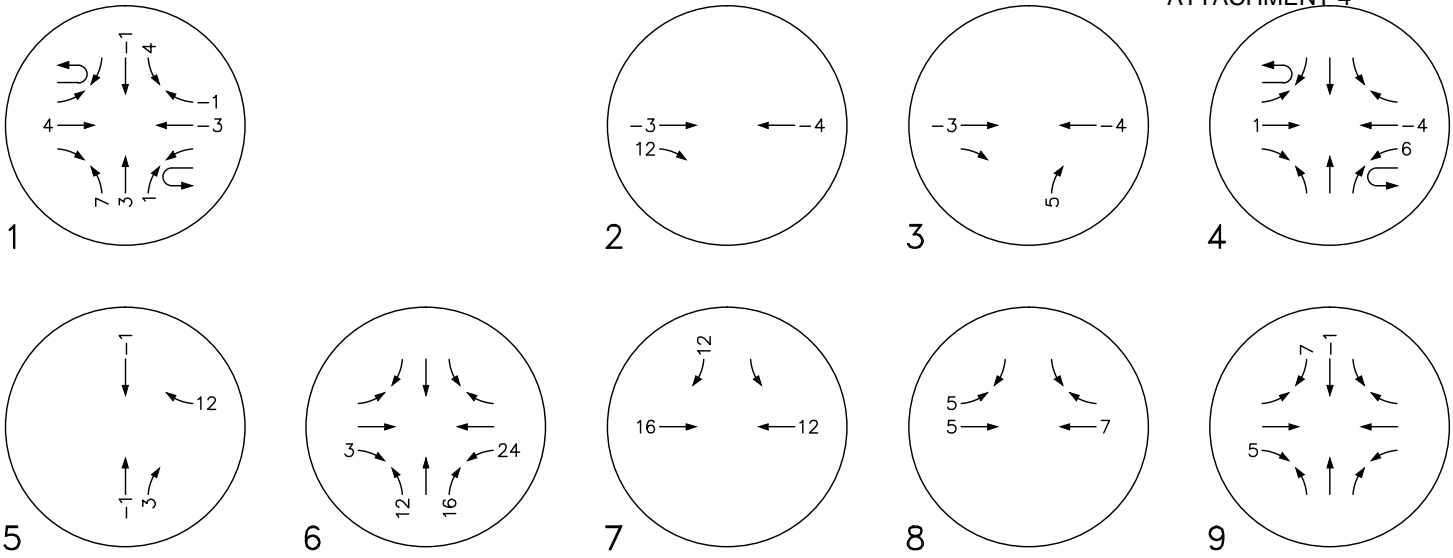
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SITE PRIMARY TRIP DISTRIBUTION
 AND ASSIGNMENT SATURDAY
 MID-DAY PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE
 9B



WEEKDAY PM PEAK HOUR
TOTAL SITE TRIPS

ENTER = 27
EXIT = 28
TOTAL = 55



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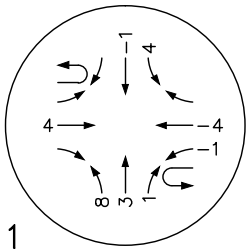
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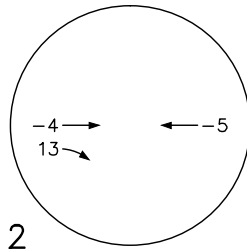
TOTAL SITE TRIP ASSIGNMENT
WEEKDAY PM PEAK HOUR

FRED MEYER #220 FUEL EXPANSION
NEWBERG, OREGON

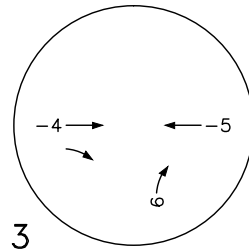
FIGURE
10A



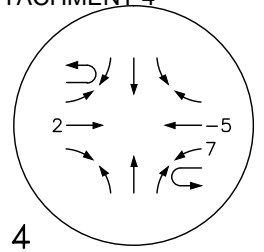
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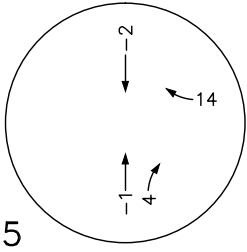
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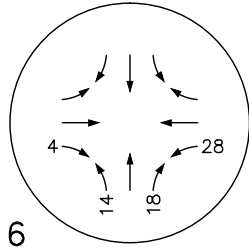
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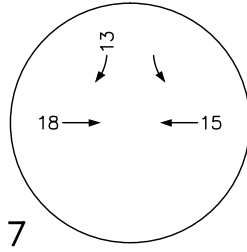
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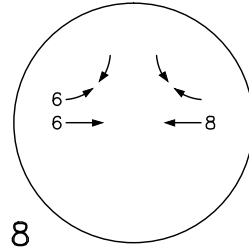
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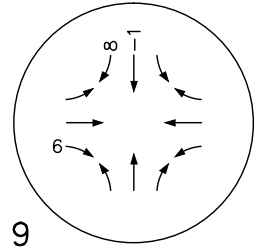
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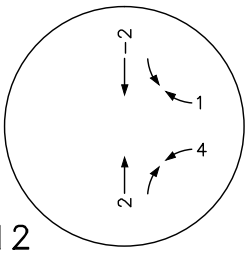
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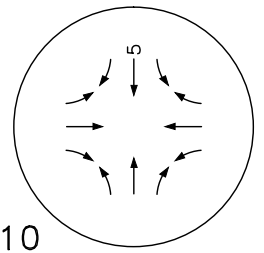
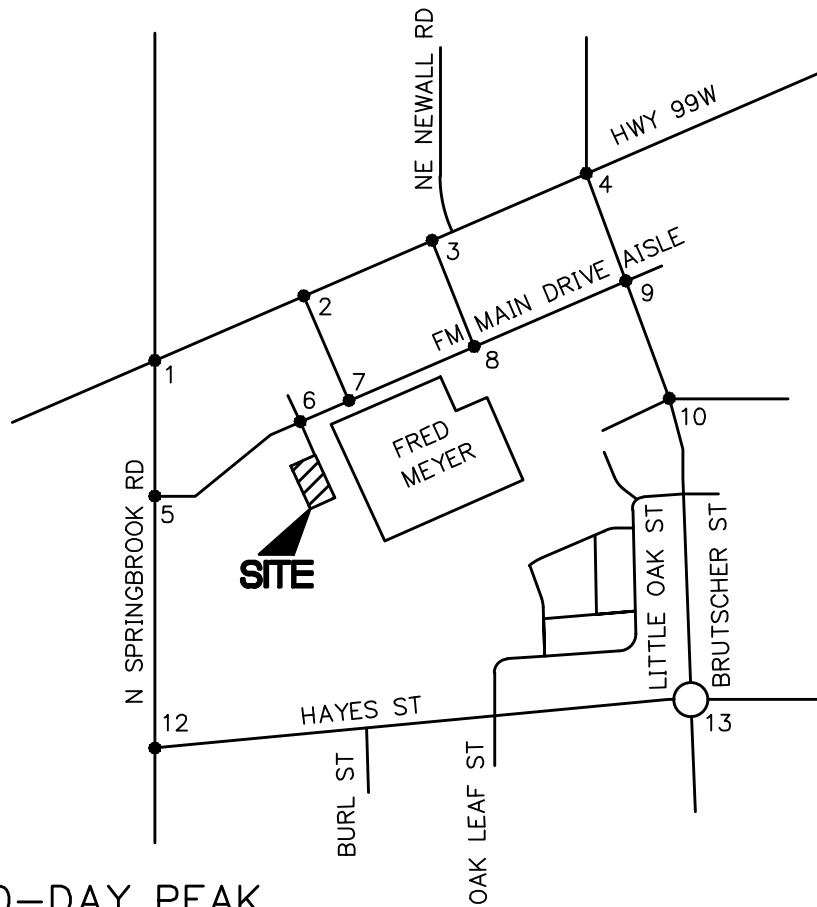
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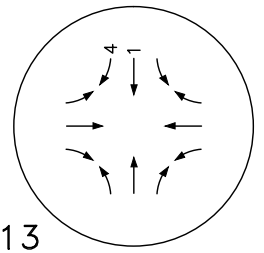
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12



10



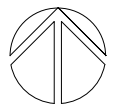
13

SATURDAY MID-DAY PEAK
 HOUR TOTAL SITE TRIPS

ENTER = 32

EXIT = 32

TOTAL = 64



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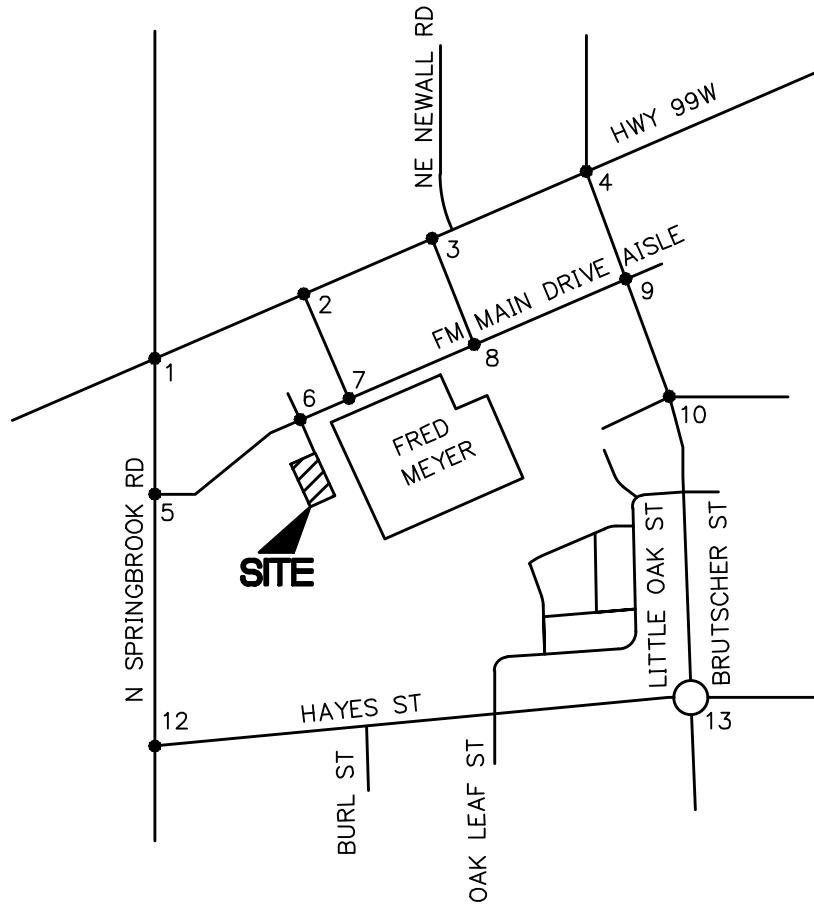
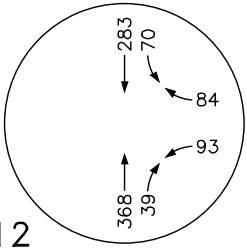
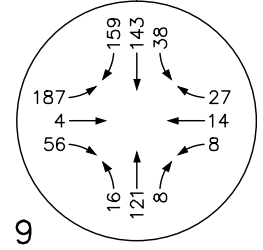
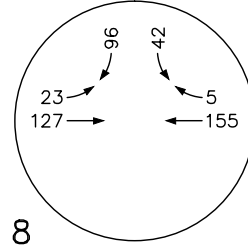
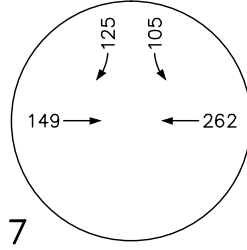
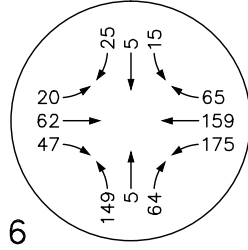
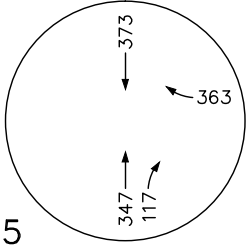
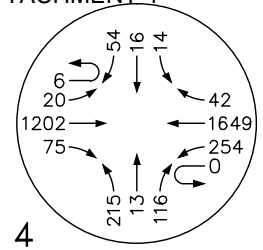
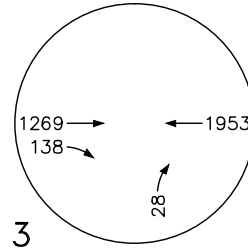
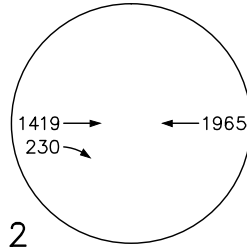
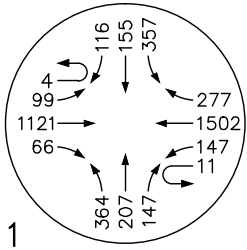
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TOTAL SITE TRIP ASSIGNMENT
 SATURDAY MID-DAY PEAK
 HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE
 10B



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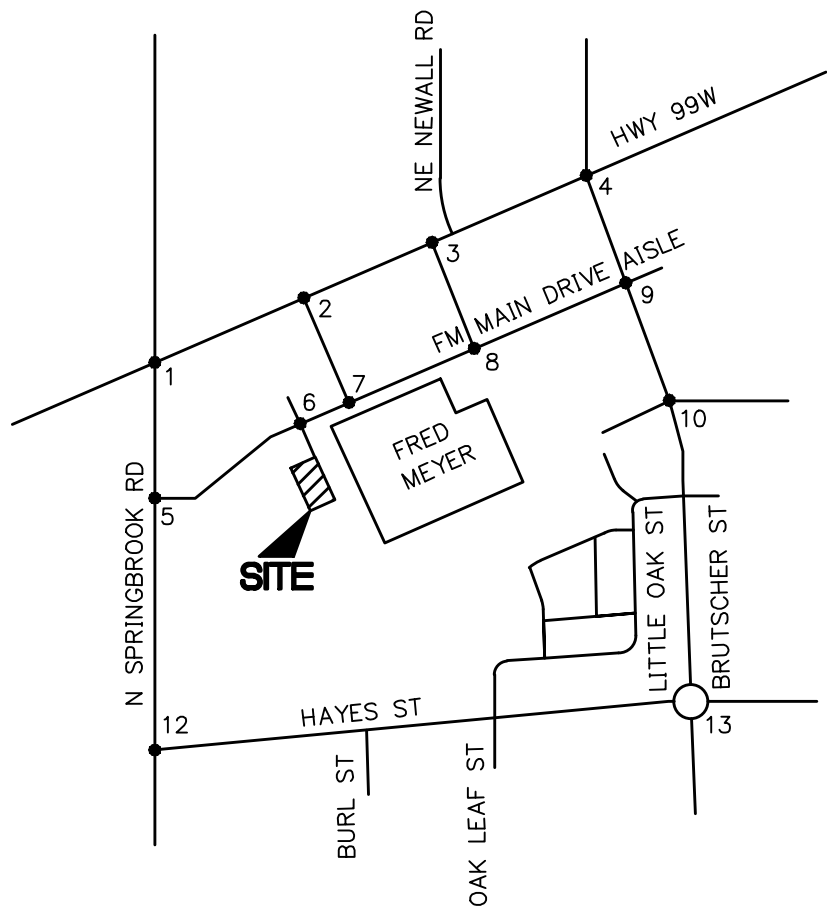
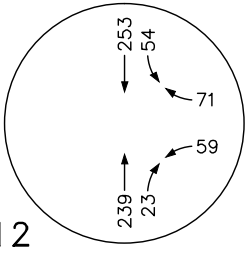
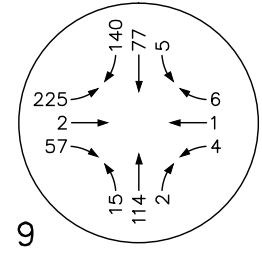
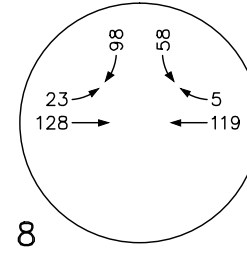
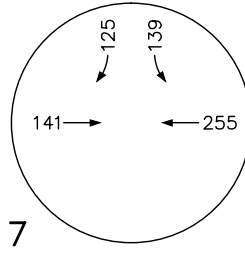
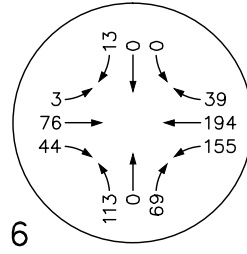
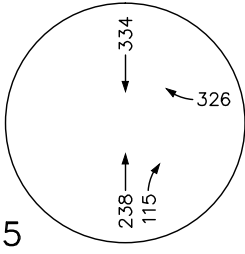
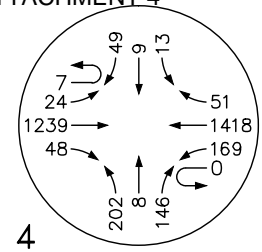
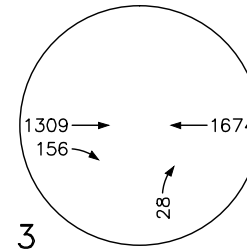
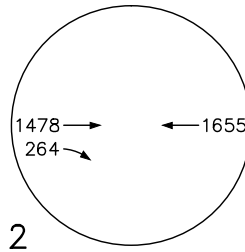
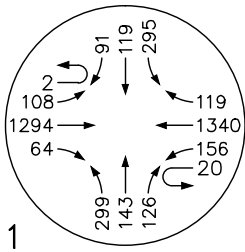
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2015 POST-DEVELOPMENT
 VOLUMES WEEKDAY PM PEAK
 HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE

11A



NOT TO SCALE



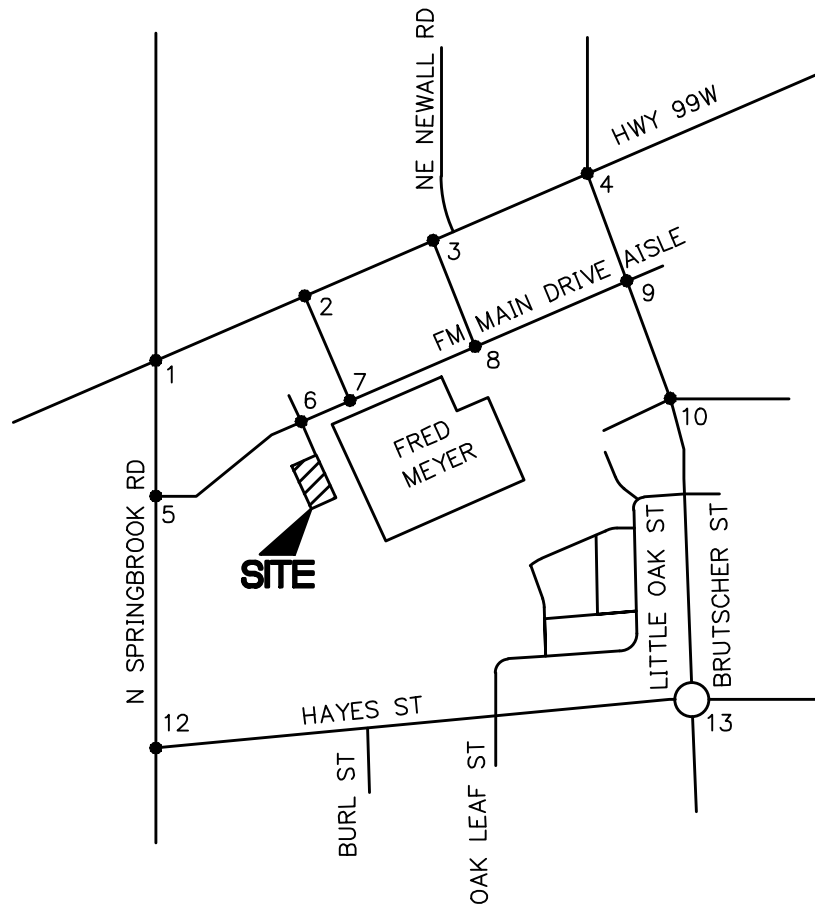
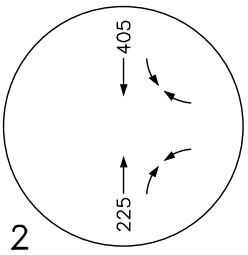
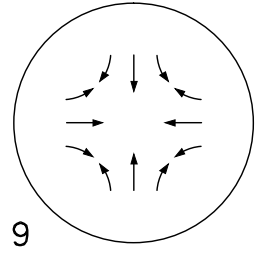
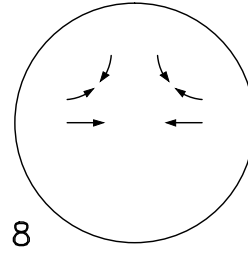
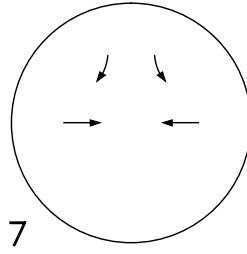
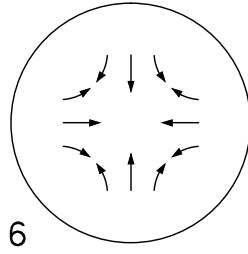
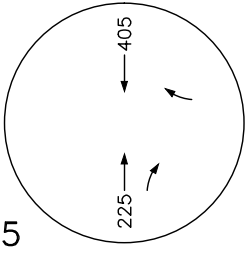
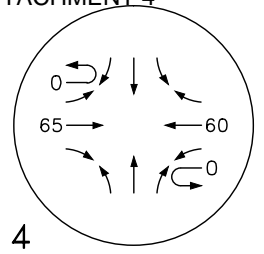
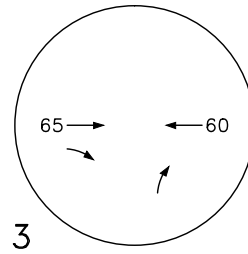
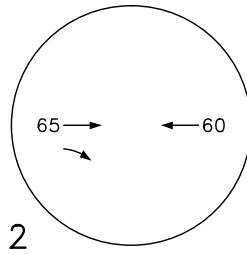
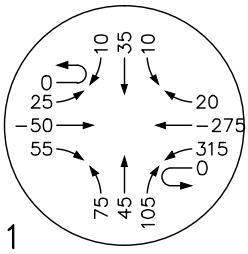
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**2015 POST-DEVELOPMENT
 VOLUMES SATURDAY MID-DAY
 PEAK HOUR**
**FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON**

**FIGURE
 11B**

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DATE: 11.03.14

DRAWN BY: DWS

CHECKED BY: BTA

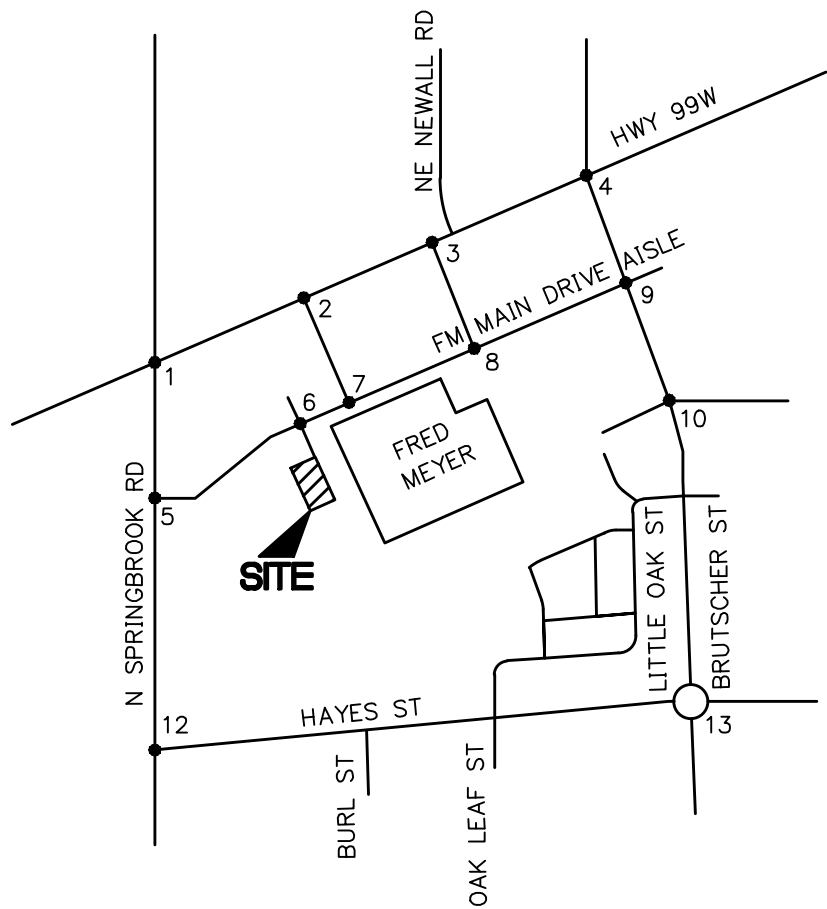
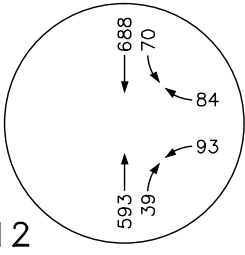
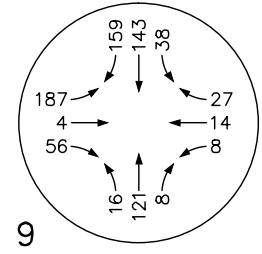
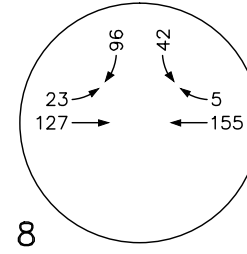
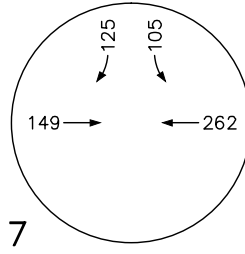
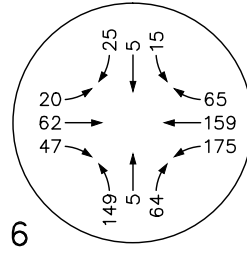
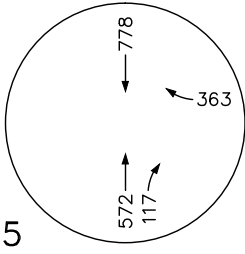
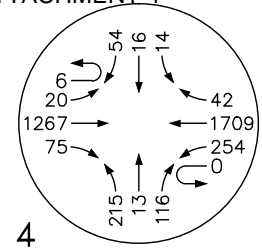
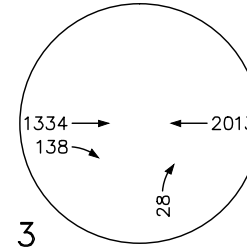
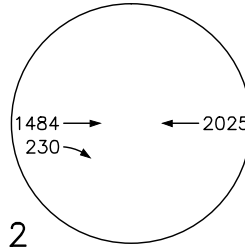
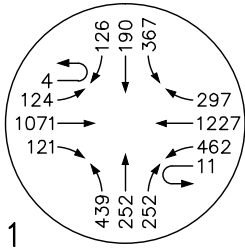
JOB NO:
 2140436.0068/690

PHASE 1 BY-PASS REROUTE
 VOLUMES WEEKDAY PM PEAK
 HOUR

FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON

FIGURE

12



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MACKENZIE
 DATE: 11.03.14
 DRAWN BY: DWS
 CHECKED BY: BTA
 JOB NO:
 2140436.0069/690

**2017 POST-DEVELOPMENT
 VOLUMES WEEKDAY PM PEAK
 HOUR**
**FRED MEYER #220 FUEL EXPANSION
 NEWBERG, OREGON**

**FIGURE
 13**

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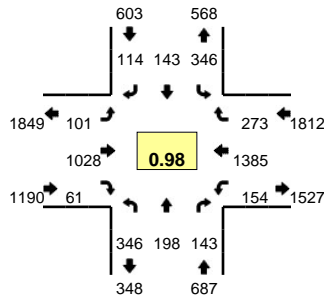
APPENDIX C
**TRAFFIC COUNT
SUMMARIES**

Type of peak hour being reported: User-Defined

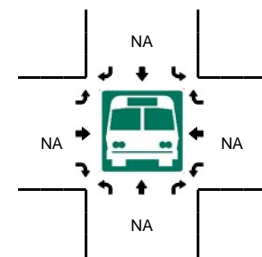
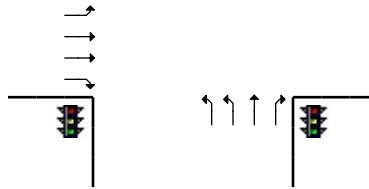
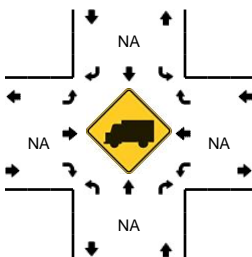
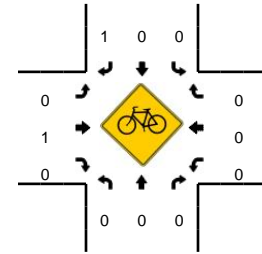
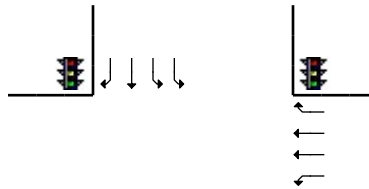
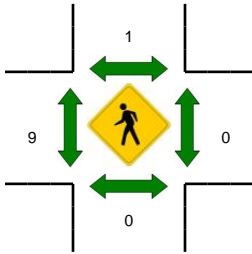
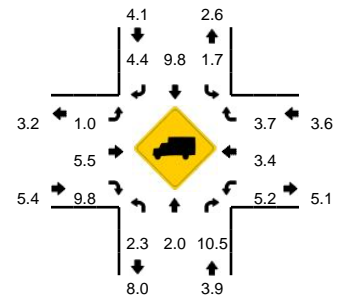
Method for determining peak hour: Total Entering Volume

LOCATION: N Springbrook Rd -- Hwy 99W
CITY/STATE: Newberg, OR

QC JOB #: 13121419
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:00 PM -- 4:15 PM



5-Min Count Period Beginning At	N Springbrook Rd (Northbound)				N Springbrook Rd (Southbound)				Hwy 99W (Eastbound)				Hwy 99W (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	26	13	8	0	25	8	13	0	11	69	8	0	9	172	22	1	385	
4:05 PM	34	20	13	0	20	6	9	0	10	85	3	0	18	122	22	1	363	
4:10 PM	32	20	12	0	34	17	19	0	10	67	2	0	13	99	22	1	348	
4:15 PM	22	11	11	0	24	11	8	0	8	108	5	0	12	124	22	1	367	
4:20 PM	38	24	11	0	35	10	8	0	3	84	5	0	10	99	30	3	360	
4:25 PM	20	11	13	0	28	5	5	0	6	86	4	0	16	130	22	0	346	
4:30 PM	30	18	12	0	41	19	12	0	5	59	9	0	2	106	26	2	341	
4:35 PM	24	12	12	0	25	6	10	0	11	106	6	0	10	125	21	0	368	
4:40 PM	44	20	8	0	38	16	6	0	3	78	3	0	17	103	28	0	364	
4:45 PM	14	14	11	0	26	12	9	0	15	121	8	2	9	110	13	0	364	
4:50 PM	39	26	19	0	34	22	11	0	10	62	2	1	14	86	21	1	348	
4:55 PM	23	9	13	0	16	11	4	0	5	103	6	1	14	109	24	0	338	4292
5:00 PM	35	18	14	0	23	13	13	0	12	64	6	0	12	69	28	0	307	4214
5:05 PM	20	11	9	0	25	10	4	0	9	103	2	1	11	138	26	0	369	4220
5:10 PM	38	29	10	0	36	17	12	0	16	75	4	0	15	96	22	0	370	4242
5:15 PM	29	16	14	0	23	10	8	0	5	84	5	0	15	132	36	0	377	4252
5:20 PM	32	19	12	0	39	18	7	0	16	80	8	0	5	97	24	1	358	4250
5:25 PM	17	12	7	0	33	11	8	0	5	91	1	0	15	135	16	4	355	4259
5:30 PM	24	21	16	0	37	15	5	0	5	76	4	0	5	91	16	0	315	4233
5:35 PM	15	14	4	0	19	12	8	0	8	101	3	0	11	134	24	1	354	4219
5:40 PM	23	19	10	0	29	15	14	0	8	81	10	0	16	103	26	3	357	4212
5:45 PM	24	9	12	0	21	13	4	0	7	84	5	0	10	127	17	0	333	4181
5:50 PM	36	15	7	0	31	13	6	0	7	70	5	0	11	77	26	0	304	4137
5:55 PM	14	14	9	0	24	7	3	0	13	90	5	0	6	102	18	0	305	4104
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	368	212	132	0	316	124	164	0	124	884	52	0	160	1572	264	12	4384	
Heavy Trucks	16	8	20		0	24	12		0	68	12		8	40	24		232	
Pedestrians	0	0	0		0	0	0		0	4	0		0	0	0		4	
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0	
Railroad																		
Stopped Buses																		

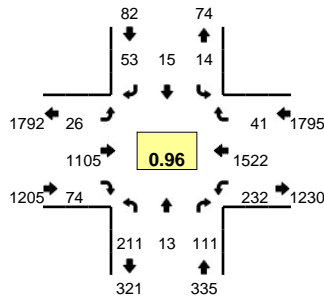
Comments:

Type of peak hour being reported: User-Defined

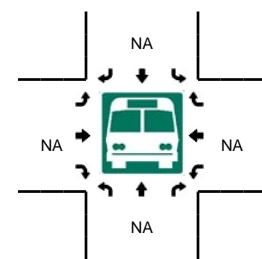
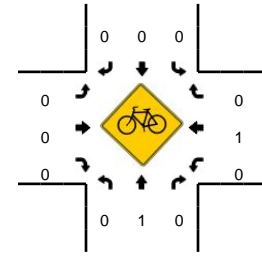
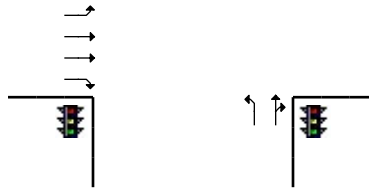
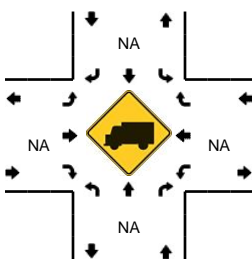
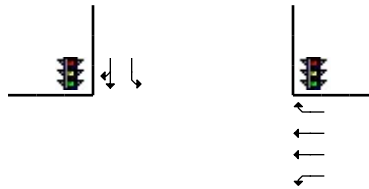
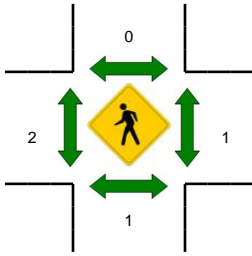
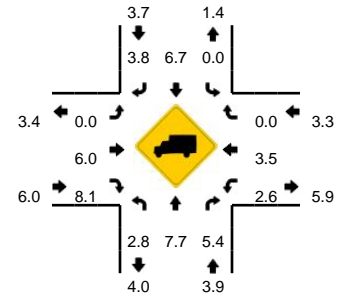
Method for determining peak hour: Total Entering Volume

LOCATION: Brutscher St -- Hwy 99W
CITY/STATE: Newberg, OR

QC JOB #: 13121417
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:25 PM -- 4:40 PM



5-Min Count Period Beginning At	Brutscher St (Northbound)				Brutscher St (Southbound)				Hwy 99W (Eastbound)				Hwy 99W (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	15	2	13	0	2	2	4	0	0	65	6	0	22	114	3	0	248	
4:05 PM	14	0	7	0	0	1	5	0	2	92	2	0	19	146	3	0	291	
4:10 PM	21	3	10	0	2	3	5	0	2	79	4	0	24	103	2	0	258	
4:15 PM	20	0	8	0	2	1	2	0	2	101	8	0	16	143	5	0	308	
4:20 PM	17	0	10	0	0	0	5	0	2	78	10	2	30	119	2	0	275	
4:25 PM	11	0	9	0	0	0	5	0	1	102	13	1	12	147	4	0	305	
4:30 PM	23	5	9	0	2	2	4	0	0	69	5	0	21	137	4	0	281	
4:35 PM	16	0	9	0	0	0	7	0	2	119	3	1	15	131	1	0	304	
4:40 PM	16	1	9	0	5	1	3	0	4	90	4	1	17	117	5	0	273	
4:45 PM	12	0	3	0	0	4	3	0	2	121	8	0	19	128	3	0	303	
4:50 PM	29	1	14	0	0	1	3	0	2	71	7	1	21	112	5	0	267	
4:55 PM	17	1	10	0	1	0	7	0	1	118	4	0	16	125	4	0	304	3417
5:00 PM	18	0	9	0	1	0	2	0	3	56	2	2	26	95	4	0	218	3387
5:05 PM	10	1	8	0	0	3	6	0	0	113	5	0	17	154	2	0	319	3415
5:10 PM	22	2	12	0	1	1	5	0	1	71	8	1	15	136	4	0	279	3436
5:15 PM	14	1	14	0	1	1	4	0	2	104	14	0	10	130	3	0	298	3426
5:20 PM	24	0	8	0	2	2	6	0	5	81	6	0	15	133	5	0	287	3438
5:25 PM	13	1	12	0	0	2	3	0	2	98	12	0	17	155	4	0	319	3452
5:30 PM	21	0	4	0	2	1	5	0	2	81	4	0	23	120	4	0	267	3438
5:35 PM	12	2	11	0	2	1	6	0	0	93	9	2	17	138	3	0	296	3430
5:40 PM	16	1	17	0	1	0	5	0	3	65	6	1	26	108	4	0	253	3410
5:45 PM	13	1	8	0	0	0	8	0	2	93	6	1	10	136	3	0	281	3388
5:50 PM	22	1	6	0	1	2	3	0	3	75	4	0	23	99	2	0	241	3362
5:55 PM	15	0	6	0	2	0	3	0	4	98	7	0	13	120	2	0	270	3328
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	200	20	108	0	8	8	64	0	12	1160	84	8	192	1660	36	0	3560	
Heavy Trucks	12	4	4		0	0	8		0	80	8		8	64	0		188	
Pedestrians		4				0				4				0			8	
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0	
Railroad																		
Stopped Buses																		

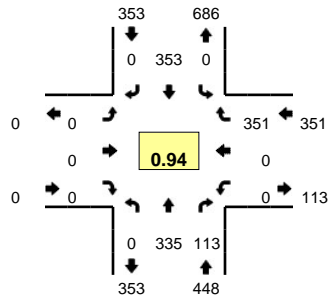
Comments:

Type of peak hour being reported: User-Defined

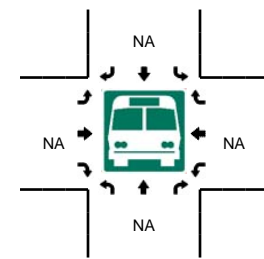
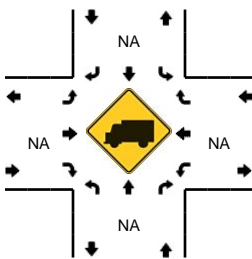
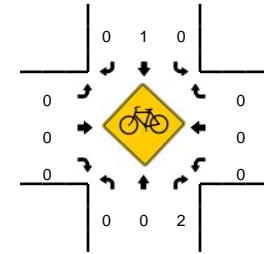
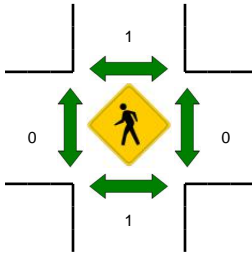
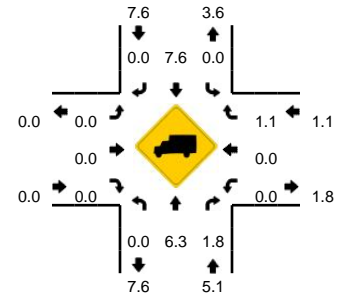
Method for determining peak hour: Total Entering Volume

LOCATION: N Springbrook Rd -- Fred Meyer Acc/Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121407
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:40 PM -- 4:55 PM



5-Min Count Period Beginning At	N Springbrook Rd (Northbound)				N Springbrook Rd (Southbound)				Fred Meyer Acc/Main Drive Aisle (Eastbound)				Fred Meyer Acc/Main Drive Aisle (Westbound)				Hourly Totals		
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
4:00 PM	0	29	8	0	0	26	0	0	0	0	0	0	0	0	0	18	0	81	
4:05 PM	0	40	13	0	0	25	0	0	0	0	0	0	0	0	0	34	0	112	
4:10 PM	0	27	8	0	0	38	0	0	0	0	0	0	0	0	0	24	0	97	
4:15 PM	0	29	11	0	0	26	0	0	0	0	0	0	0	0	0	31	0	97	
4:20 PM	0	24	9	0	0	31	0	0	0	0	0	0	0	0	0	33	0	97	
4:25 PM	0	23	9	0	0	23	0	0	0	0	0	0	0	0	0	30	0	85	
4:30 PM	0	26	11	0	0	33	0	0	0	0	0	0	0	0	0	32	0	102	
4:35 PM	0	25	12	0	0	21	0	0	0	0	0	0	0	0	0	26	0	84	
4:40 PM	0	24	10	0	0	35	0	0	0	0	0	0	0	0	0	33	0	102	
4:45 PM	0	33	8	0	0	28	0	0	0	0	0	0	0	0	0	27	0	96	
4:50 PM	0	27	6	0	0	42	0	0	0	0	0	0	0	0	0	35	0	110	
4:55 PM	0	28	8	0	0	25	0	0	0	0	0	0	0	0	0	28	0	89	1152
5:00 PM	0	24	8	0	0	37	0	0	0	0	0	0	0	0	0	37	0	106	1177
5:05 PM	0	37	7	0	0	20	0	0	0	0	0	0	0	0	0	19	0	83	1148
5:10 PM	0	30	4	0	0	38	0	0	0	0	0	0	0	0	0	36	0	108	1159
5:15 PM	0	33	9	0	0	29	0	0	0	0	0	0	0	0	0	32	0	103	1165
5:20 PM	0	28	6	0	0	33	0	0	0	0	0	0	0	0	0	23	0	90	1158
5:25 PM	0	29	10	0	0	25	0	0	0	0	0	0	0	0	0	14	0	78	1151
5:30 PM	0	22	7	0	0	30	0	0	0	0	0	0	0	0	0	32	0	91	1140
5:35 PM	0	17	4	0	0	27	0	0	0	0	0	0	0	0	0	17	0	65	1121
5:40 PM	0	28	4	0	0	43	0	0	0	0	0	0	0	0	0	21	0	96	1115
5:45 PM	0	29	10	0	0	22	0	0	0	0	0	0	0	0	0	25	0	86	1105
5:50 PM	0	24	8	0	0	36	0	0	0	0	0	0	0	0	0	26	0	94	1089
5:55 PM	0	21	7	0	0	18	0	0	0	0	0	0	0	0	0	21	0	67	1067
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total		
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
All Vehicles	0	336	96	0	0	420	0	0	0	0	0	0	0	0	380	0	1232		
Heavy Trucks	0	8	4	0	0	32	0	0	0	0	0	0	0	0	4	0	48		
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
Railroad																			
Stopped Buses																			

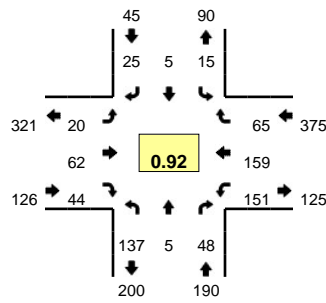
Comments:

Type of peak hour being reported: User-Defined

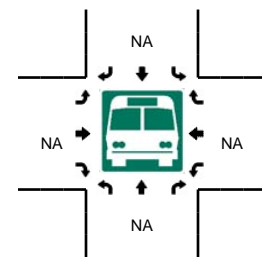
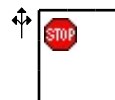
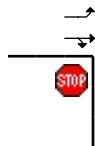
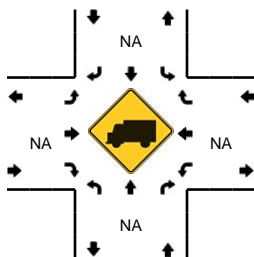
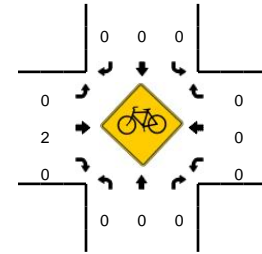
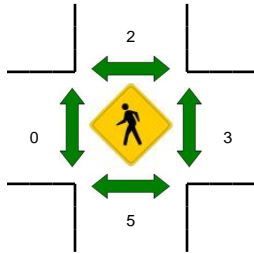
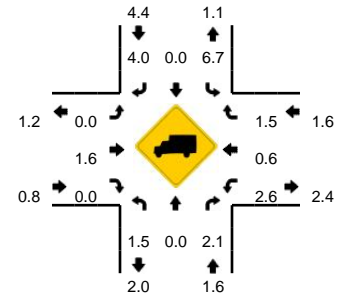
Method for determining peak hour: Total Entering Volume

LOCATION: US Bank/Fuel Drive Aisle -- Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121405
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:30 PM -- 4:45 PM



5-Min Count Period Beginning At	US Bank/Fuel Drive Aisle (Northbound)				US Bank/Fuel Drive Aisle (Southbound)				Main Drive Aisle (Eastbound)				Main Drive Aisle (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	6	0	3	0	1	2	1	0	0	8	3	0	9	16	5	0	54	
4:05 PM	15	1	5	0	1	0	1	0	0	7	6	0	17	8	2	0	63	
4:10 PM	9	1	6	0	2	0	2	0	2	5	4	0	16	13	9	0	69	
4:15 PM	11	0	8	0	0	0	1	0	2	6	5	0	9	13	6	0	61	
4:20 PM	12	0	4	0	0	1	2	0	1	4	3	0	10	16	9	0	62	
4:25 PM	9	0	2	0	0	0	3	0	3	3	3	0	14	9	9	0	55	
4:30 PM	10	0	4	0	0	1	3	0	3	6	6	0	15	19	6	0	73	
4:35 PM	12	0	6	0	3	0	1	0	3	8	3	0	15	12	3	0	66	
4:40 PM	14	1	1	0	0	0	1	0	0	6	2	0	19	15	3	0	62	
4:45 PM	14	2	0	0	4	1	3	0	2	5	4	0	7	10	8	0	60	
4:50 PM	15	0	4	0	3	0	5	0	3	1	2	0	11	13	2	0	59	
4:55 PM	10	0	5	0	1	0	2	0	1	3	3	0	9	15	3	0	52	736
5:00 PM	14	0	4	0	0	0	1	0	1	3	5	0	13	17	4	0	62	744
5:05 PM	8	0	5	0	0	1	0	0	2	1	4	0	10	13	6	0	50	731
5:10 PM	10	1	2	0	1	0	1	0	0	1	3	0	13	13	1	0	46	708
5:15 PM	9	0	2	0	3	0	1	1	0	2	5	0	7	20	3	0	53	700
5:20 PM	8	0	6	0	0	0	1	0	0	7	2	0	13	9	2	0	48	686
5:25 PM	9	0	3	0	0	0	1	0	1	6	2	0	18	6	4	0	50	681
5:30 PM	13	0	3	0	1	0	0	0	1	6	2	0	15	16	1	0	58	666
5:35 PM	8	1	6	0	0	0	0	0	0	0	3	0	6	7	4	0	35	635
5:40 PM	7	0	5	0	0	0	3	0	2	2	1	0	17	9	2	0	48	621
5:45 PM	11	0	1	0	0	0	1	0	0	3	6	0	6	14	1	0	43	604
5:50 PM	14	0	1	0	0	0	0	0	0	4	4	0	9	16	0	0	48	593
5:55 PM	8	0	5	0	0	0	0	0	1	4	2	0	7	8	3	0	38	579
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	144	4	44	0	12	4	20	0	24	80	44	0	196	184	48	0	804	
Heavy Trucks	4	0	0		0	0	0		0	4	0		4	4	0		16	
Pedestrians	0				4				0				4				8	
Bicycles	0	0	0		0	0	0		0	1	0		0	0	0		1	
Railroad																		
Stopped Buses																		

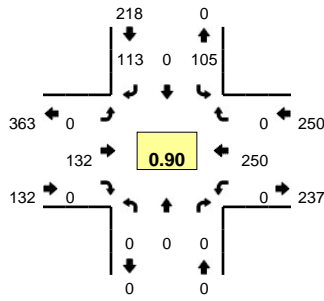
Comments:

Type of peak hour being reported: User-Defined

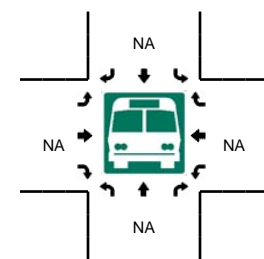
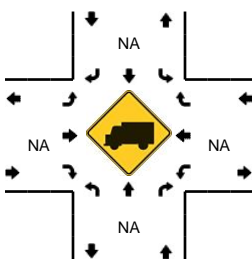
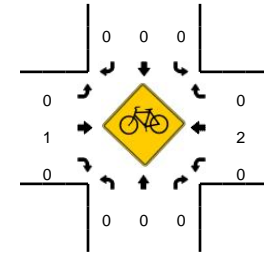
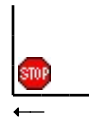
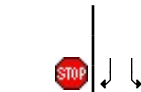
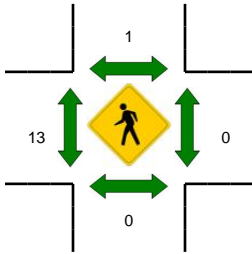
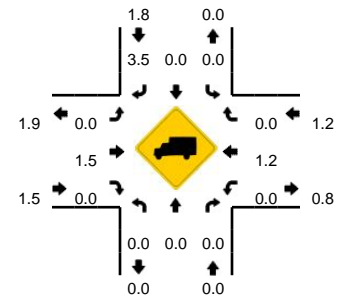
Method for determining peak hour: Total Entering Volume

LOCATION: West RI Acc -- Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121403
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:10 PM -- 4:25 PM



5-Min Count Period Beginning At	West RI Acc (Northbound)				West RI Acc (Southbound)				Main Drive Aisle (Eastbound)				Main Drive Aisle (Westbound)				Total	Hourly Totals	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
4:00 PM	0	0	0	0	6	0	6	0	0	12	0	0	0	0	23	0	0	47	
4:05 PM	0	0	0	0	9	0	12	0	0	10	0	0	0	0	17	0	0	48	
4:10 PM	0	0	0	0	7	0	11	0	0	15	0	0	0	0	26	0	0	59	
4:15 PM	0	0	0	0	13	0	12	0	0	12	0	0	0	0	15	0	0	52	
4:20 PM	0	0	0	0	8	0	8	0	0	12	0	0	0	0	27	0	0	55	
4:25 PM	0	0	0	0	11	0	13	0	0	8	0	0	0	0	18	0	0	50	
4:30 PM	0	0	0	0	4	0	7	0	0	11	0	0	0	0	29	0	0	51	
4:35 PM	0	0	0	0	7	0	13	0	0	15	0	0	0	0	17	0	0	52	
4:40 PM	0	0	0	0	7	0	7	0	0	7	0	0	0	0	29	0	0	50	
4:45 PM	0	0	0	0	11	0	9	0	0	12	0	0	0	0	14	0	0	46	
4:50 PM	0	0	0	0	9	0	9	0	0	9	0	0	0	0	18	0	0	45	
4:55 PM	0	0	0	0	13	0	6	0	0	9	0	0	0	0	17	0	0	45	600
5:00 PM	0	0	0	0	11	0	6	0	0	6	0	0	0	0	26	0	0	49	602
5:05 PM	0	0	0	0	14	0	11	0	0	9	0	0	0	0	17	0	0	51	605
5:10 PM	0	0	0	0	2	0	7	0	0	6	0	0	0	0	19	0	0	34	580
5:15 PM	0	0	0	0	11	0	8	0	0	8	0	0	0	0	24	0	0	51	579
5:20 PM	0	0	0	0	7	0	7	0	0	13	0	0	0	0	16	0	0	43	567
5:25 PM	0	0	0	0	15	0	13	0	0	10	0	0	0	0	17	0	0	55	572
5:30 PM	0	0	0	0	13	0	10	0	0	10	0	0	0	0	19	0	0	52	573
5:35 PM	0	0	0	0	17	0	10	0	0	6	0	0	0	0	10	0	0	43	564
5:40 PM	0	0	0	0	7	0	8	0	0	7	0	0	0	0	17	0	0	39	553
5:45 PM	0	0	0	0	15	0	3	0	0	6	0	0	0	0	18	0	0	42	549
5:50 PM	0	0	0	0	6	0	6	0	0	6	0	0	0	0	16	0	0	34	538
5:55 PM	0	0	0	0	14	0	5	0	0	8	0	0	0	0	14	0	0	41	534
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total		
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
All Vehicles	0	0	0	0	112	0	124	0	0	156	0	0	0	272	0	0	664		
Heavy Trucks	0	0	0	0	0	0	8	0	0	4	0	0	0	0	0	0	12		
Pedestrians	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	0	8		
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
Railroad																			
Stopped Buses																			

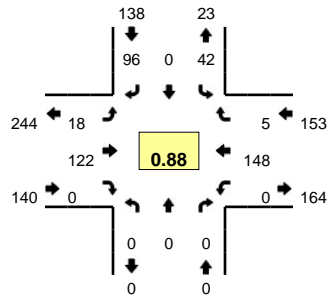
Comments:

Type of peak hour being reported: User-Defined

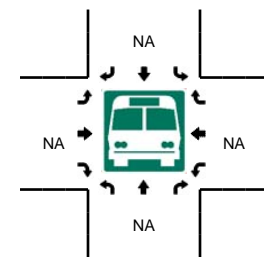
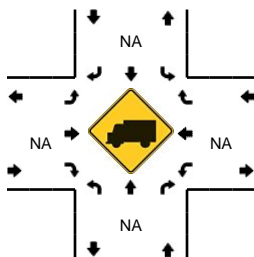
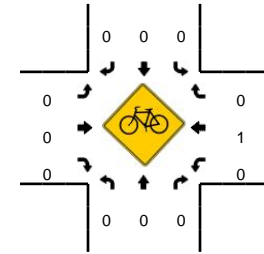
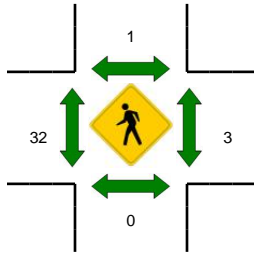
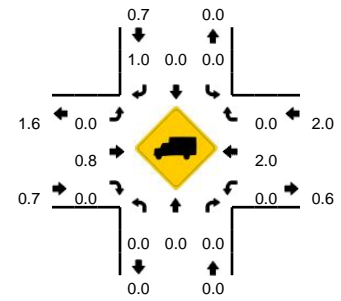
Method for determining peak hour: Total Entering Volume

LOCATION: East RIRO Acc -- Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121401
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:25 PM -- 4:40 PM



5-Min Count Period Beginning At	East RIRO Acc (Northbound)				East RIRO Acc (Southbound)				Main Drive Aisle (Eastbound)				Main Drive Aisle (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	0	0	0	0	3	0	5	0	3	6	0	0	0	12	0	0	29	
4:05 PM	0	0	0	0	2	0	9	0	1	10	0	0	0	20	1	0	43	
4:10 PM	0	0	0	0	2	0	3	0	1	10	0	0	0	11	0	0	27	
4:15 PM	0	0	0	0	2	0	13	0	0	12	0	0	0	13	0	0	40	
4:20 PM	0	0	0	0	5	0	7	0	1	10	0	0	0	16	1	0	40	
4:25 PM	0	0	0	0	7	0	7	0	2	14	0	0	0	11	0	0	41	
4:30 PM	0	0	0	0	4	0	8	0	2	12	0	0	0	13	0	0	39	
4:35 PM	0	0	0	0	2	0	13	0	2	10	0	0	0	15	0	0	42	
4:40 PM	0	0	0	0	2	0	9	0	2	3	0	0	0	11	1	0	28	
4:45 PM	0	0	0	0	6	0	7	0	0	15	0	0	0	7	0	0	35	
4:50 PM	0	0	0	0	6	0	5	0	1	11	0	0	0	7	1	0	31	
4:55 PM	0	0	0	0	1	0	10	0	3	9	0	0	0	12	1	0	36	431
5:00 PM	0	0	0	0	3	0	3	0	1	10	0	0	0	8	1	0	26	428
5:05 PM	0	0	0	0	4	0	7	0	1	11	0	0	0	11	0	0	34	419
5:10 PM	0	0	0	0	4	0	4	0	1	13	0	0	0	5	0	0	27	419
5:15 PM	0	0	0	0	3	0	8	0	4	10	0	0	0	14	0	0	39	418
5:20 PM	0	0	0	0	3	0	4	0	1	8	0	0	0	8	0	0	24	402
5:25 PM	0	0	0	0	2	0	11	0	0	13	0	0	0	13	1	0	40	401
5:30 PM	0	0	0	0	2	0	3	0	0	7	0	0	0	5	0	0	17	379
5:35 PM	0	0	0	0	3	0	12	0	4	8	0	0	0	8	0	0	35	372
5:40 PM	0	0	0	0	7	0	8	0	0	12	0	0	0	13	0	0	40	384
5:45 PM	0	0	0	0	4	0	7	0	1	13	0	0	0	8	0	0	33	382
5:50 PM	0	0	0	0	1	0	5	0	5	4	0	0	0	6	0	0	21	372
5:55 PM	0	0	0	0	5	0	2	0	1	9	0	0	0	11	0	0	28	364
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	0	0	0	0	52	0	112	0	24	144	0	0	0	156	0	0	488	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8	
Pedestrians	0	0	0	0	0	0	0	0	0	16	0	0	0	0	0	0	16	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	
Railroad																		
Stopped Buses																		

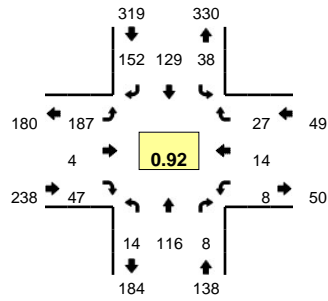
Comments:

Type of peak hour being reported: User-Defined

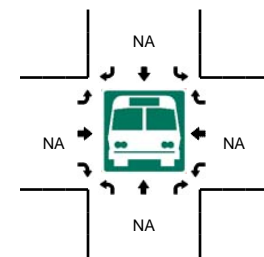
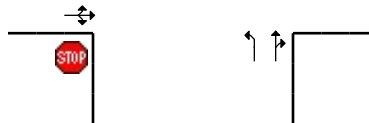
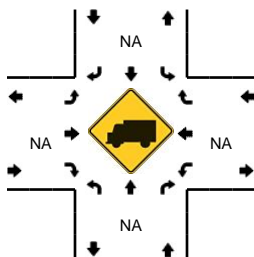
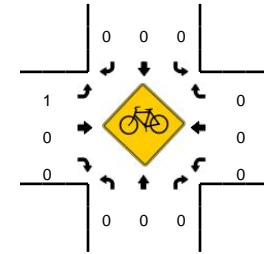
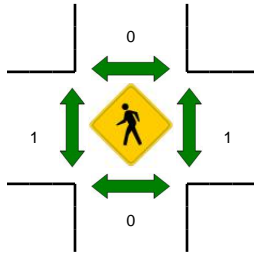
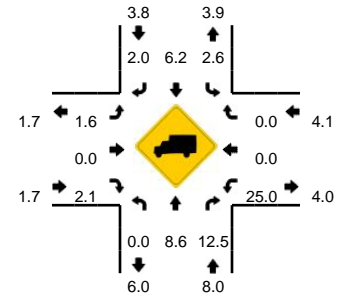
Method for determining peak hour: Total Entering Volume

LOCATION: Brutscher St -- Fred Meyer North Acc/Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121415
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:20 PM -- 4:35 PM



5-Min Count Period Beginning At	Brutscher St (Northbound)				Brutscher St (Southbound)				Fred Meyer North Acc/Main Drive Aisle (Eastbound)				Fred Meyer North Acc/Main Drive Aisle (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	0	10	1	0	3	11	11	0	15	1	4	0	0	2	1	0	59	
4:05 PM	1	10	2	0	0	6	20	0	14	0	3	0	1	0	0	0	57	
4:10 PM	1	14	1	0	4	10	11	0	16	0	6	0	0	1	3	0	67	
4:15 PM	0	9	0	0	4	10	15	0	14	0	3	0	1	0	2	0	58	
4:20 PM	3	10	0	0	7	17	12	0	15	0	2	0	0	1	2	0	69	
4:25 PM	0	3	0	0	5	15	10	0	17	1	5	0	1	4	5	0	66	
4:30 PM	3	13	0	0	4	11	9	0	13	0	6	0	1	2	5	0	67	
4:35 PM	1	11	2	0	3	6	13	0	15	1	8	0	0	0	3	0	63	
4:40 PM	1	5	0	0	3	7	9	0	16	0	0	0	2	0	1	0	44	
4:45 PM	0	6	2	0	3	15	16	0	13	0	5	0	0	2	4	0	66	
4:50 PM	2	15	0	0	1	13	9	0	22	0	1	0	1	2	1	0	67	
4:55 PM	2	10	0	0	1	8	17	0	17	1	4	0	1	0	0	0	61	744
5:00 PM	2	8	0	0	2	12	10	0	13	0	5	0	0	0	3	0	55	740
5:05 PM	3	8	0	0	1	12	16	0	21	0	4	0	0	0	2	0	67	750
5:10 PM	0	7	0	0	4	9	7	0	15	0	5	0	1	0	4	0	52	735
5:15 PM	0	13	1	0	1	14	13	0	15	0	5	0	1	0	3	0	66	743
5:20 PM	1	11	0	0	1	7	11	0	16	0	2	0	0	0	1	0	50	724
5:25 PM	1	14	0	0	0	14	21	0	17	0	5	0	0	0	1	0	73	731
5:30 PM	1	11	0	0	0	7	15	0	9	0	6	0	0	0	0	0	49	713
5:35 PM	0	12	0	0	1	9	21	0	17	0	1	0	0	0	1	0	62	712
5:40 PM	0	8	1	0	1	14	14	0	18	0	8	0	0	0	1	0	65	733
5:45 PM	1	8	0	0	0	11	8	0	19	0	5	0	0	1	0	0	53	720
5:50 PM	0	13	1	0	0	12	10	0	9	0	2	0	0	0	0	0	47	700
5:55 PM	3	12	0	0	0	14	12	0	17	0	3	0	1	0	0	0	62	701
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	24	104	0	0	64	172	124	0	180	4	52	0	8	28	48	0	808	
Heavy Trucks	0	4	0	0	0	16	4	0	4	0	0	0	0	0	0	0	28	
Pedestrians	0	0	0	0	0	0	0	0	0	0	4	0	0	4	0	0	8	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad																		
Stopped Buses																		

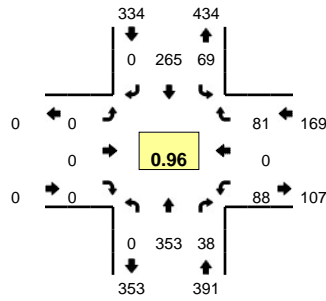
Comments:

Type of peak hour being reported: User-Defined

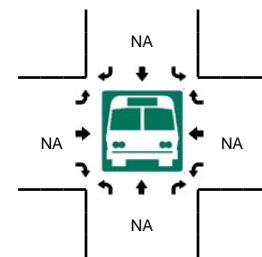
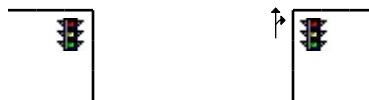
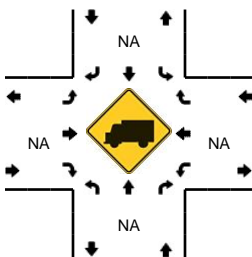
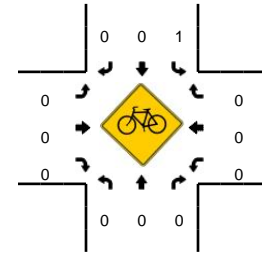
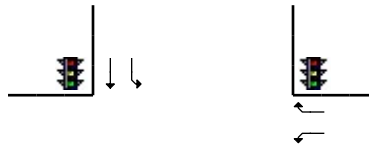
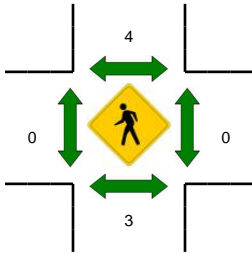
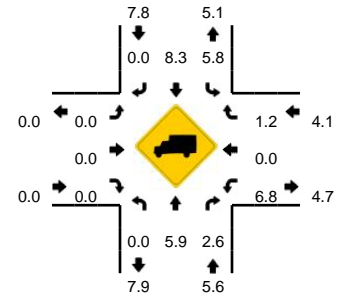
Method for determining peak hour: Total Entering Volume

LOCATION: N Springbrook Rd -- Hayes St
CITY/STATE: Newberg, OR

QC JOB #: 13121409
DATE: Thu, Oct 16 2014



Peak-Hour: 4:00 PM -- 5:00 PM
Peak 15-Min: 4:40 PM -- 4:55 PM



5-Min Count Period Beginning At	N Springbrook Rd (Northbound)				N Springbrook Rd (Southbound)				Hayes St (Eastbound)				Hayes St (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	0	38	0	0	6	18	0	0	0	0	0	0	2	0	6	0	70	
4:05 PM	0	34	7	0	4	20	0	0	0	0	0	0	8	0	5	0	78	
4:10 PM	0	33	5	0	9	26	0	0	0	0	0	0	5	0	4	0	82	
4:15 PM	0	27	3	0	6	20	0	0	0	0	0	0	12	0	5	0	73	
4:20 PM	0	27	2	0	6	24	0	0	0	0	0	0	11	0	5	0	75	
4:25 PM	0	24	3	0	6	15	0	0	0	0	0	0	5	0	7	0	60	
4:30 PM	0	33	4	0	6	27	0	0	0	0	0	0	5	0	10	0	85	
4:35 PM	0	28	6	0	3	15	0	0	0	0	0	0	10	0	7	0	69	
4:40 PM	0	26	1	0	4	31	0	0	0	0	0	0	10	0	6	0	78	
4:45 PM	0	36	4	0	7	18	0	0	0	0	0	0	4	0	8	0	77	
4:50 PM	0	22	2	0	8	32	0	0	0	0	0	0	8	0	7	0	79	
4:55 PM	0	25	1	0	4	19	0	0	0	0	0	0	8	0	11	0	68	894
5:00 PM	0	21	3	0	8	23	0	0	0	0	0	0	11	0	10	0	76	900
5:05 PM	0	34	4	0	4	17	0	0	0	0	0	0	5	0	7	0	71	893
5:10 PM	0	30	3	0	4	27	0	0	0	0	0	0	7	0	9	0	80	891
5:15 PM	0	33	3	0	9	22	0	0	0	0	0	0	10	0	3	0	80	898
5:20 PM	0	28	4	0	10	20	0	0	0	0	0	0	5	0	3	0	70	893
5:25 PM	0	32	3	0	6	20	0	0	0	0	0	0	6	0	9	0	76	909
5:30 PM	0	27	8	0	9	18	0	0	0	0	0	0	6	0	2	0	70	894
5:35 PM	0	12	5	0	5	20	0	0	0	0	0	0	4	0	10	0	56	881
5:40 PM	0	21	2	0	10	33	0	0	0	0	0	0	7	0	9	0	82	885
5:45 PM	0	31	2	0	5	17	0	0	0	0	0	0	3	0	4	0	62	870
5:50 PM	0	26	2	0	8	19	0	0	0	0	0	0	3	0	5	0	63	854
5:55 PM	0	17	2	0	3	10	0	0	0	0	0	0	6	0	6	0	44	830
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	0	336	28	0	76	324	0	0	0	0	0	0	88	0	84	0	936	
Heavy Trucks	0	12	4	0	4	28	0	0	0	0	0	0	8	0	0	0	56	
Pedestrians	0	0	0	0	0	8	0	0	0	0	0	0	0	0	0	0	8	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad																		
Stopped Buses																		

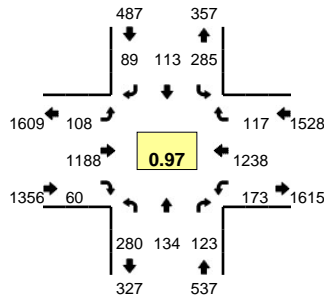
Comments:

Type of peak hour being reported: User-Defined

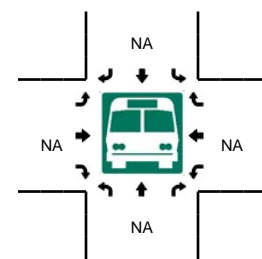
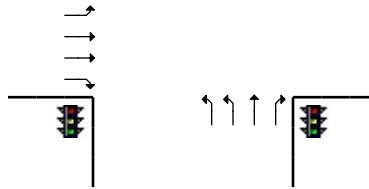
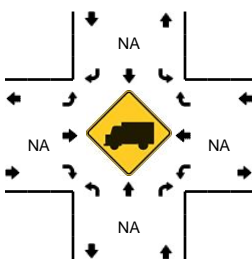
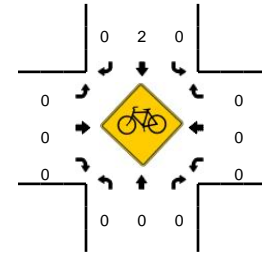
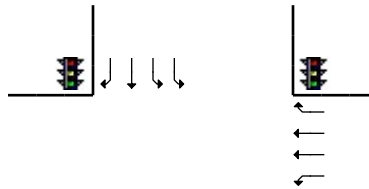
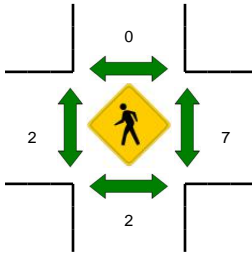
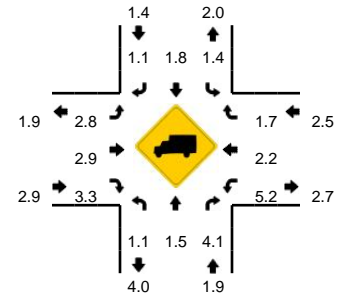
Method for determining peak hour: Total Entering Volume

LOCATION: N Springbrook Rd -- Hwy 99W
CITY/STATE: Newberg, OR

QC JOB #: 13121420
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:30 PM -- 1:45 PM



15-Min Count Period Beginning At	N Springbrook Rd (Northbound)				N Springbrook Rd (Southbound)				Hwy 99W (Eastbound)				Hwy 99W (Westbound)				Total	Hourly Totals	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
11:00 AM	71	42	26	0	94	25	32	0	20	260	17	1	12	286	31	0	917		
11:15 AM	82	27	20	1	103	24	29	0	22	281	18	0	13	322	43	4	989		
11:30 AM	77	25	31	0	68	18	25	0	29	284	21	1	21	287	35	5	927		
11:45 AM	60	41	27	0	77	22	19	0	39	286	15	0	21	314	37	6	964	3797	
12:00 PM	69	37	39	0	71	24	25	0	27	284	10	1	17	324	33	8	969	3849	
12:15 PM	59	27	31	0	66	20	31	0	24	281	15	2	17	342	30	5	950	3810	
12:30 PM	63	40	20	0	70	20	28	0	28	283	10	1	40	330	31	4	968	3851	
12:45 PM	63	37	33	0	74	21	34	0	28	253	16	0	42	306	40	6	953	3840	
1:00 PM	74	33	32	0	68	21	24	0	25	282	17	0	49	316	29	4	974	3845	
1:15 PM	79	27	33	0	75	28	22	0	29	287	13	1	30	278	26	8	936	3831	
1:30 PM	54	37	31	0	76	36	25	0	27	304	16	1	45	332	25	3	1012	3875	
1:45 PM	73	37	27	0	66	28	18	0	25	315	14	0	30	312	37	4	986	3908	
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total		
All Vehicles	216	148	124	0	304	144	100	0	108	1216	64	4	180	1328	100	12	4048		
Heavy Trucks	0	0	8		12	8	4		0	28	0		16	32	0		108		
Pedestrians	0	0	0		0	0	0		0	4	0		0	0	0		4		
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0		
Railroad																			
Stopped Buses																			

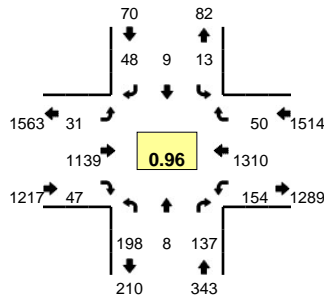
Comments:

Type of peak hour being reported: User-Defined

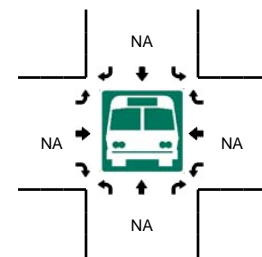
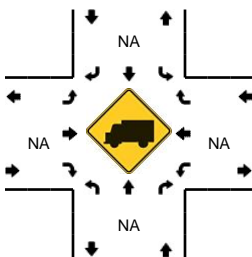
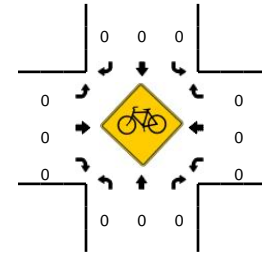
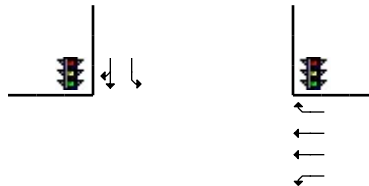
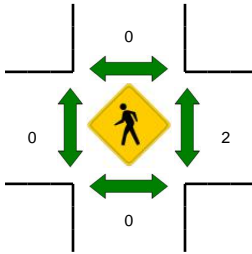
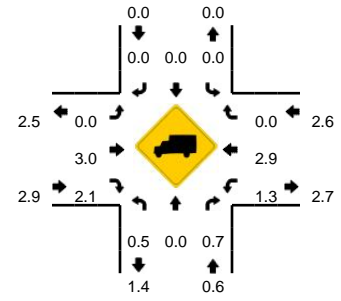
Method for determining peak hour: Total Entering Volume

LOCATION: Brutscher St -- Hwy 99W
CITY/STATE: Newberg, OR

QC JOB #: 13121418
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:30 PM -- 1:45 PM



15-Min Count Period Beginning At	Brutscher St (Northbound)				Brutscher St (Southbound)				Hwy 99W (Eastbound)				Hwy 99W (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:00 AM	34	3	23	0	2	1	19	0	8	251	14	2	25	342	7	0	731	
11:15 AM	46	4	22	0	10	2	12	0	11	283	12	3	32	309	12	0	758	
11:30 AM	44	3	39	0	5	3	12	0	8	254	12	5	36	314	5	0	740	
11:45 AM	43	3	25	0	5	1	10	0	6	300	12	1	35	364	8	0	813	3042
12:00 PM	41	4	49	0	8	4	16	0	9	269	5	0	36	326	11	0	778	3089
12:15 PM	42	3	24	0	3	2	6	0	9	255	9	2	40	388	5	0	788	3119
12:30 PM	53	6	38	0	4	2	12	0	5	261	14	3	46	346	15	0	805	3184
12:45 PM	46	3	26	0	2	4	17	0	3	272	3	3	33	359	10	0	781	3152
1:00 PM	44	3	38	0	2	2	12	0	6	256	10	1	42	336	7	0	759	3133
1:15 PM	49	0	32	0	5	0	10	0	6	295	10	2	34	313	15	0	771	3116
1:30 PM	57	1	36	0	5	2	10	0	3	292	11	4	40	339	16	0	816	3127
1:45 PM	48	4	31	0	1	5	16	0	9	296	16	0	38	322	12	0	798	3144
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
All Vehicles	228	4	144	0	20	8	40	0	12	1168	44	16	160	1356	64	0	3264	
Heavy Trucks	0	0	0	0	0	0	0	0	0	36	0	0	4	40	0	0	80	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

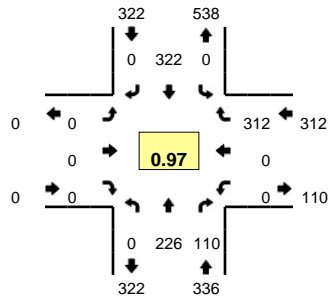
Comments:

Type of peak hour being reported: User-Defined

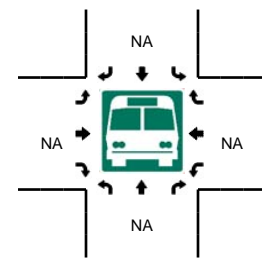
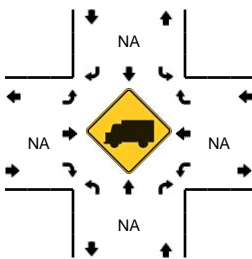
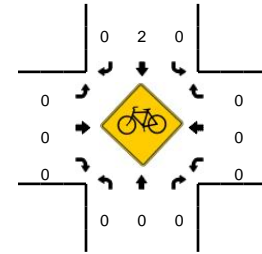
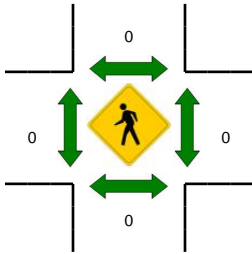
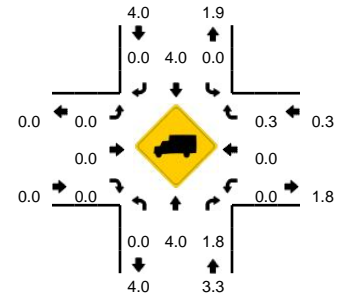
Method for determining peak hour: Total Entering Volume

LOCATION: N Springbrook Rd -- Fred Meyer Acc/Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121408
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:30 PM -- 1:45 PM



15-Min Count Period Beginning At	N Springbrook Rd (Northbound)				N Springbrook Rd (Southbound)				Fred Meyer Acc/Main Drive Aisle (Eastbound)				Fred Meyer Acc/Main Drive Aisle (Westbound)				Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
11:00 AM	0	63	24	0	0	63	0	0	0	0	0	0	0	0	68	0	218
11:15 AM	0	64	22	0	0	63	0	0	0	0	0	0	0	0	66	0	215
11:30 AM	0	58	19	0	0	60	0	0	0	0	0	0	0	0	75	0	212
11:45 AM	0	44	17	0	0	58	0	0	0	0	0	0	0	0	80	0	199
12:00 PM	0	69	21	0	0	53	0	0	0	0	0	0	0	0	74	0	217
12:15 PM	0	61	25	0	0	53	0	0	0	0	0	0	0	0	60	0	199
12:30 PM	0	50	24	0	0	68	0	0	0	0	0	0	0	0	67	0	209
12:45 PM	0	56	26	0	0	77	0	0	0	0	0	0	0	0	74	0	233
1:00 PM	0	59	30	0	0	81	0	0	0	0	0	0	0	0	76	0	246
1:15 PM	0	59	29	0	0	72	0	0	0	0	0	0	0	0	87	0	247
1:30 PM	0	51	29	0	0	97	0	0	0	0	0	0	0	0	74	0	251
1:45 PM	0	57	22	0	0	72	0	0	0	0	0	0	0	0	75	0	226
Peak 15-Min Flowrates																	
	Northbound				Southbound				Eastbound				Westbound				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
All Vehicles	0	204	116	0	0	388	0	0	0	0	0	0	0	0	296	0	1004
Heavy Trucks	0	4	0	0	0	28	0	0	0	0	0	0	0	0	0	0	32
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Railroad																	
Stopped Buses																	

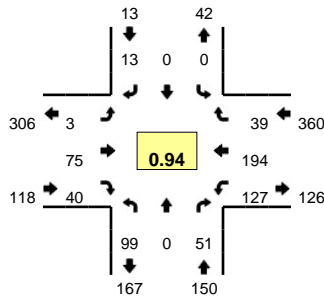
Comments:

Type of peak hour being reported: User-Defined

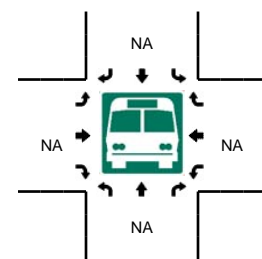
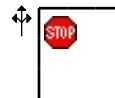
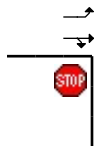
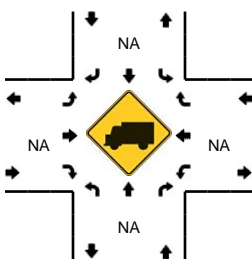
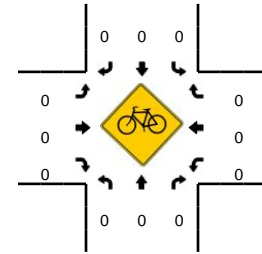
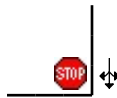
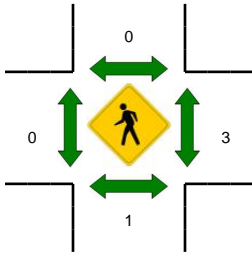
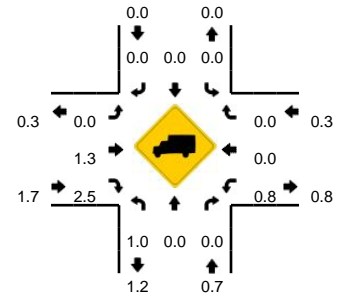
Method for determining peak hour: Total Entering Volume

LOCATION: US Bank/Fuel Drive Aisle -- Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121406
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:15 PM -- 1:30 PM



15-Min Count Period Beginning At	US Bank/Fuel Drive Aisle (Northbound)				US Bank/Fuel Drive Aisle (Southbound)				Main Drive Aisle (Eastbound)				Main Drive Aisle (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:00 AM	29	0	12	0	0	0	2	0	2	14	12	0	38	34	8	0	151	
11:15 AM	31	0	13	0	1	1	0	0	1	10	10	0	37	41	8	0	153	
11:30 AM	35	0	15	0	1	1	3	0	0	15	10	0	33	37	11	0	161	
11:45 AM	24	0	13	0	0	0	1	0	0	16	6	0	35	54	9	0	158	623
12:00 PM	29	0	17	0	0	0	2	0	3	12	8	0	37	39	3	0	150	622
12:15 PM	25	0	18	0	0	1	0	0	1	13	12	0	23	35	10	0	138	607
12:30 PM	20	0	14	0	1	0	2	0	0	18	6	0	38	43	8	0	150	596
12:45 PM	27	0	15	0	1	0	1	0	1	24	8	0	28	40	7	0	152	590
1:00 PM	31	0	12	0	0	0	1	0	1	19	11	0	36	45	11	0	167	607
1:15 PM	30	0	10	0	0	0	2	0	0	19	12	0	36	52	10	0	171	640
1:30 PM	19	0	18	0	0	0	6	0	2	16	12	0	25	46	9	0	153	643
1:45 PM	19	0	11	0	0	0	4	0	0	21	5	0	30	51	9	0	150	641
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	120	0	40	0	0	0	8	0	0	76	48	0	144	208	40	0	684	
Heavy Trucks	0	0	0		0	0	0		0	0	0		0	0	0		0	
Pedestrians		0				0				0				4			4	
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0	
Railroad																		
Stopped Buses																		

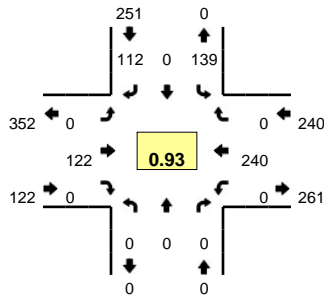
Comments:

Type of peak hour being reported: User-Defined

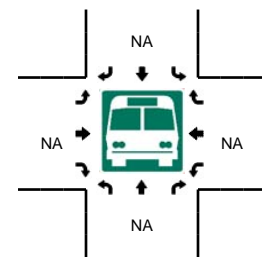
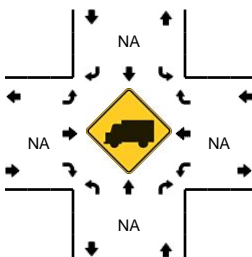
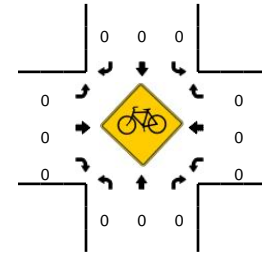
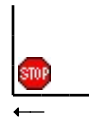
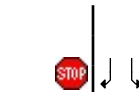
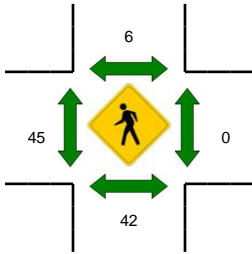
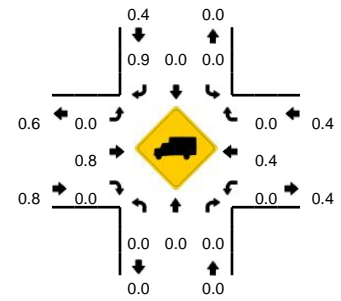
Method for determining peak hour: Total Entering Volume

LOCATION: West RI Acc -- Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121404
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:00 PM -- 1:15 PM



15-Min Count Period Beginning At	West RI Acc (Northbound)				West RI Acc (Southbound)				Main Drive Aisle (Eastbound)				Main Drive Aisle (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:00 AM	0	0	0	0	37	0	29	0	0	23	0	0	0	59	0	0	148	
11:15 AM	0	0	0	0	41	0	29	0	0	28	0	0	0	51	0	0	149	
11:30 AM	0	0	0	0	39	0	35	0	0	33	0	0	0	49	0	0	156	
11:45 AM	0	0	0	0	34	0	31	0	0	27	0	0	0	64	0	0	156	609
12:00 PM	0	0	0	0	45	0	22	0	0	33	0	0	0	59	0	0	159	620
12:15 PM	0	0	0	0	45	0	27	0	0	29	0	0	0	42	0	0	143	614
12:30 PM	0	0	0	0	40	0	27	0	0	34	0	0	0	62	0	0	163	621
12:45 PM	0	0	0	0	29	0	31	0	0	37	0	0	0	51	0	0	148	613
1:00 PM	0	0	0	0	45	0	27	0	0	32	0	0	0	61	0	0	165	619
1:15 PM	0	0	0	0	37	0	29	0	0	28	0	0	0	66	0	0	160	636
1:30 PM	0	0	0	0	29	0	32	0	0	34	0	0	0	41	0	0	136	609
1:45 PM	0	0	0	0	28	0	24	0	0	28	0	0	0	72	0	0	152	613
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	0	0	0	0	180	0	108	0	0	128	0	0	0	244	0	0	660	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Pedestrians		40				0				52				0			92	
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0	
Railroad																	0	
Stopped Buses																		

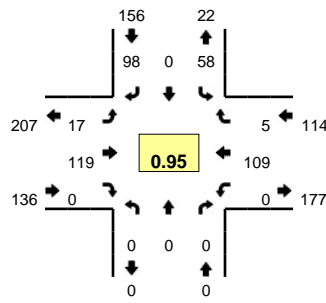
Comments:

Type of peak hour being reported: User-Defined

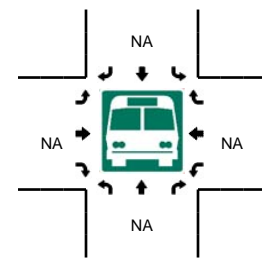
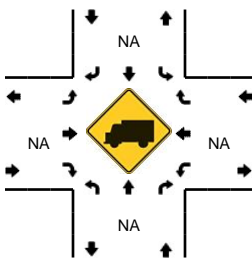
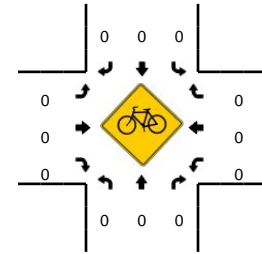
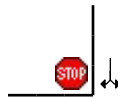
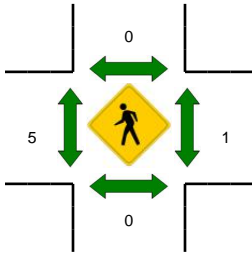
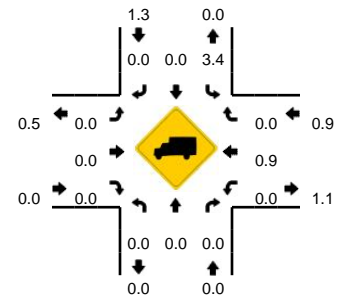
Method for determining peak hour: Total Entering Volume

LOCATION: East RIRO Acc -- Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121402
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:30 PM -- 1:45 PM



15-Min Count Period Beginning At	East RIRO Acc (Northbound)				East RIRO Acc (Southbound)				Main Drive Aisle (Eastbound)				Main Drive Aisle (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:00 AM	0	0	0	0	17	0	36	0	4	20	0	0	0	29	0	0	106	
11:15 AM	0	0	0	0	14	0	27	0	2	35	0	0	0	23	2	0	103	
11:30 AM	0	0	0	0	11	0	24	0	5	33	0	0	0	26	0	0	99	
11:45 AM	0	0	0	0	10	0	17	0	6	23	0	0	0	29	1	0	86	394
12:00 PM	0	0	0	0	23	0	29	0	1	37	0	0	0	29	2	0	121	409
12:15 PM	0	0	0	0	13	0	19	0	5	29	0	0	0	18	2	0	86	392
12:30 PM	0	0	0	0	11	0	15	0	5	24	0	0	0	38	1	0	94	387
12:45 PM	0	0	0	0	11	0	24	0	5	36	0	0	0	25	2	0	103	404
1:00 PM	0	0	0	0	11	0	20	0	4	32	0	0	0	26	0	0	93	376
1:15 PM	0	0	0	0	18	0	28	0	2	22	0	0	0	33	0	0	103	393
1:30 PM	0	0	0	0	17	0	23	0	7	34	0	0	0	22	4	0	107	406
1:45 PM	0	0	0	0	12	0	27	0	4	31	0	0	0	28	1	0	103	406
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	0	0	0	0	68	0	92	0	28	136	0	0	0	88	16	0	428	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

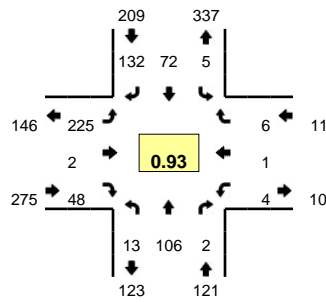
Comments:

Type of peak hour being reported: User-Defined

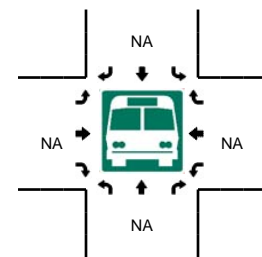
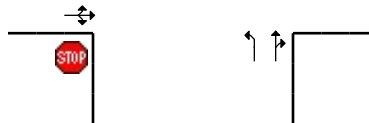
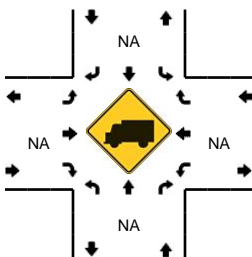
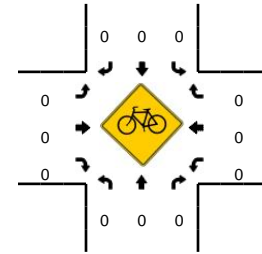
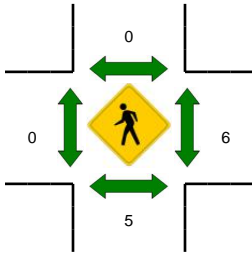
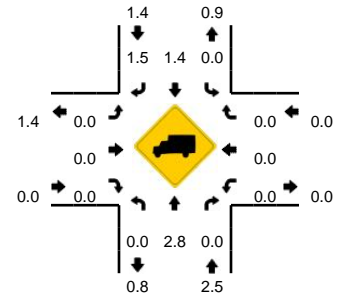
Method for determining peak hour: Total Entering Volume

LOCATION: Brutscher St -- Fred Meyer North Acc/Main Drive Aisle
CITY/STATE: Newberg, OR

QC JOB #: 13121416
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:45 PM -- 2:00 PM



15-Min Count Period Beginning At	Brutscher St (Northbound)				Brutscher St (Southbound)				Fred Meyer North Acc/Main Drive Aisle (Eastbound)				Fred Meyer North Acc/Main Drive Aisle (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:00 AM	4	23	1	0	1	10	31	0	38	0	7	0	0	0	1	0	116	
11:15 AM	1	18	0	0	3	16	27	0	50	1	16	0	0	2	2	0	136	
11:30 AM	2	21	1	0	0	19	30	0	61	0	8	0	0	0	2	0	144	
11:45 AM	8	18	0	0	0	22	31	0	54	0	6	0	0	0	0	0	139	535
12:00 PM	3	28	0	0	0	16	30	0	66	0	9	0	0	0	0	0	152	571
12:15 PM	0	17	1	0	2	10	32	1	49	2	10	0	0	1	4	0	129	564
12:30 PM	8	29	1	0	3	23	40	0	66	0	9	0	1	1	2	0	183	603
12:45 PM	3	21	2	0	0	17	24	0	56	1	7	0	2	0	0	0	133	597
1:00 PM	4	32	2	0	0	20	28	0	46	2	12	0	2	0	2	0	150	595
1:15 PM	3	24	0	0	1	17	32	0	59	0	6	0	0	0	2	1	145	611
1:30 PM	2	24	0	0	0	13	38	0	62	0	17	0	0	0	0	0	156	584
1:45 PM	4	26	0	0	4	22	34	0	58	0	13	0	1	1	2	0	165	616
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
All Vehicles	16	104	0	0	16	88	136	0	232	0	52	0	4	4	8	0	660	
Heavy Trucks	0	4	0	0	0	0	4	0	0	0	0	0	0	0	0	0	8	
Pedestrians		12				0				0				12			24	
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0	
Railroad																		
Stopped Buses																		

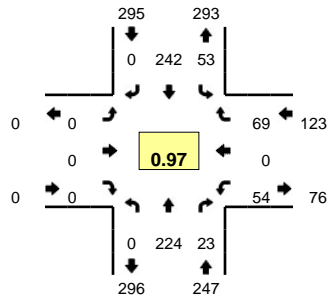
Comments:

Type of peak hour being reported: User-Defined

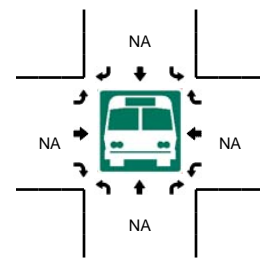
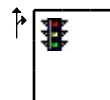
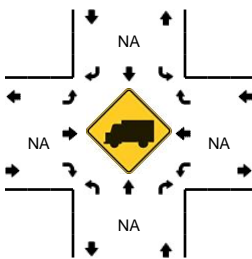
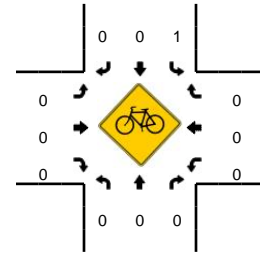
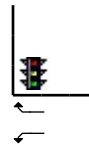
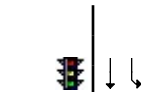
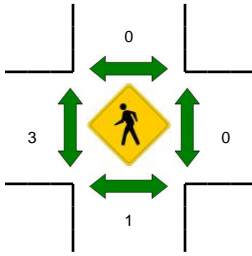
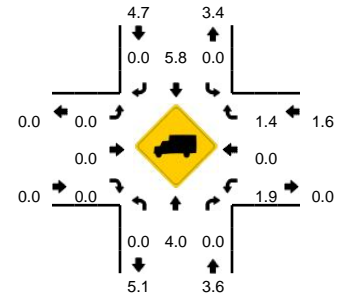
Method for determining peak hour: Total Entering Volume

LOCATION: N Springbrook Rd -- Hayes St
CITY/STATE: Newberg, OR

QC JOB #: 13121410
DATE: Sat, Oct 18 2014



Peak-Hour: 1:00 PM -- 2:00 PM
Peak 15-Min: 1:30 PM -- 1:45 PM



15-Min Count Period Beginning At	N Springbrook Rd (Northbound)				N Springbrook Rd (Southbound)				Hayes St (Eastbound)				Hayes St (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:00 AM	0	66	6	0	8	42	0	0	0	0	0	0	19	0	15	0	156	
11:15 AM	0	58	5	0	12	47	0	0	0	0	0	0	13	0	22	0	157	
11:30 AM	0	61	7	0	17	38	0	0	0	0	0	0	7	0	14	0	144	
11:45 AM	0	48	2	0	17	38	0	0	0	0	0	0	11	0	13	0	129	586
12:00 PM	0	74	6	0	14	37	0	0	0	0	0	0	10	0	12	0	153	583
12:15 PM	0	63	6	0	11	38	0	0	0	0	0	0	16	0	20	0	154	580
12:30 PM	0	55	5	0	12	54	0	0	0	0	0	0	16	0	18	0	160	596
12:45 PM	0	58	9	0	14	57	0	0	0	0	0	0	21	0	16	0	175	642
1:00 PM	0	59	5	0	13	61	0	0	0	0	0	0	15	0	18	0	171	660
1:15 PM	0	67	6	0	15	53	0	0	0	0	0	0	11	0	15	0	167	673
1:30 PM	0	51	5	0	13	69	0	0	0	0	0	0	15	0	19	0	172	685
1:45 PM	0	47	7	0	12	59	0	0	0	0	0	0	13	0	17	0	155	665
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
All Vehicles	0	204	20	0	52	276	0	0	0	0	0	0	60	0	76	0	688	
Heavy Trucks	0	4	0	0	0	28	0	0	0	0	0	0	0	0	0	0	32	
Pedestrians	0	0	0	0	0	0	0	0	8	0	0	0	0	0	0	0	8	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad																		
Stopped Buses																		

Comments:

APPENDIX D
**SEASONAL
ADJUSTMENT
FACTOR DATA AND
CALCULATIONS**

ATR 36-004	2008	2009	2011	2012	2013	MAX	MIN	AVERAGE	SAF
Peak Month (Month)	109% (Jul-Aug)	109% (Jul)	109% (Aug)	110% (Aug)	109% (Aug)	110%	109%	109%	106.5%
Count Month (Oct)	104%	101%	102%	102%	103%	104%	101%	102%	

Open ATR Data: <http://www.grpmackweb.com/gm/EFL.php?i=127516>

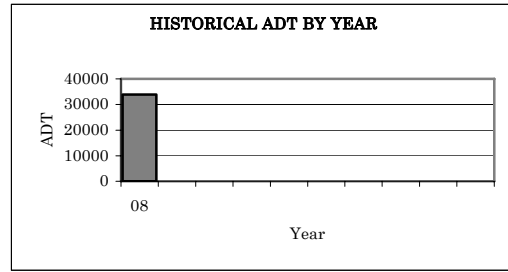
On-Site ATR Method: ODOT APM v2 Section 5.4.1

Location: OR8; MP 14.84; TUALATIN VALLEY HIGHWAY NO. 29; 0.28 mile west of N.W. 334th Avenue

Site Name: Cornelius (34-009)
Installed: September, 2007

HISTORICAL TRAFFIC DATA

Year	Average Daily Traffic	Percent of AADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2008	33838	120	10.2	9.7	9.6	9.5



2008 TRAFFIC DATA

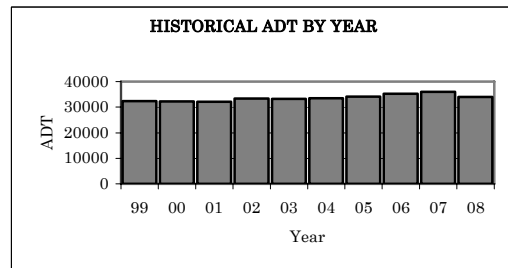
Month	Average Weekday Traffic	Percent of AADT	Average Daily Traffic	Percent of AADT	Classification Breakdown	Percent of AADT
February	35714	106	34447	102	Passenger cars	91.7
March	35431	105	34317	101	Light Trucks	2.8
April	35945	106	34957	103	Buses	0.8
May	35030	104	35051	104	Single unit trucks (2 axles)	2.3
June	34922	103	34056	101	Single unit trucks (3 axles)	0.6
July	35789	106	34073	101	Single unit trucks (4 or more axles)	0.1
August	36278	107	34957	103	Single trailer trucks (4 or less axles)	0.5
September	35707	106	34896	103	Single trailer trucks (5 axles)	0.4
October	35286	104	34125	101	Single trailer trucks (6 or more axles)	0.2
November	33669	100	32516	96	Multi trailer trucks (5 or less axles)	0.1
December	30918	91	29833	88	Multi trailer trucks (6 axles)	0.1
					Multi trailer trucks (7 or more axles)	0.0

Location: OR99W; MP 21.81; PACIFIC HIGHWAY WEST NO. 91; 0.01 mile west of Brutscher Street

Site Name: Newberg (36-004)
Installed: July, 1952

HISTORICAL TRAFFIC DATA

Year	Average Daily Traffic	Percent of AADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
1999	32417	***	***	***	***	***
2000	32292	***	***	***	***	***
2001	32158	***	***	***	***	***
2002	33361	120	9.4	9.0	8.9	8.8
2003	33269	121	9.3	9.0	8.9	8.8
2004	33463	122	9.3	9.0	8.9	8.8
2005	34128	121	9.3	8.9	8.8	8.8
2006	35302	122	8.9	8.8	8.8	8.7
2007	35985	120	8.9	8.7	8.7	8.6
2008	34049	122	9.2	8.9	8.9	8.8



2008 TRAFFIC DATA

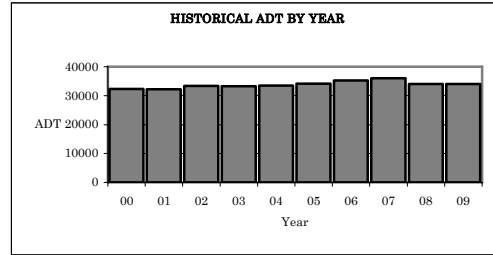
Month	Average Weekday Traffic	Percent of AADT	Average Daily Traffic	Percent of AADT	Classification Breakdown	Percent of AADT
February	36157	106	35002	103	Passenger cars	40.6
March	35830	105	34616	102	Light Trucks	52.8
April	36110	106	35294	104	Buses	0.2
May	35543	104	35049	103	Single unit trucks (2 axles)	3.1
June	36460	107	35428	104	Single unit trucks (3 axles)	0.7
July	37246	109	35667	105	Single unit trucks (4 or more axles)	0.2
August	36975	109	35965	106	Single trailer trucks (4 or less axles)	0.4
September	35606	105	35021	103	Single trailer trucks (5 axles)	0.9
October	35376	104	34352	101	Single trailer trucks (6 or more axles)	0.5
November	34178	100	33021	97	Multi trailer trucks (5 or less axles)	0.1
December	27809	82	26509	78	Multi trailer trucks (6 axles)	0.1
					Multi trailer trucks (7 or more axles)	0.6

Location: OR99W/OR219 MP 21.81; PACIFIC HIGHWAY WEST NO. 91; 0.01 mile west of Brutscher Street

Site Name: Newberg (36-004)
Installed: July, 1952

HISTORICAL TRAFFIC DATA

Year	Average Daily Traffic	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2000	32292	***	***	***	***	***
2001	32158	***	***	***	***	***
2002	33361	120	9.4	9.0	8.9	8.8
2003	33269	121	9.3	9.0	8.9	8.8
2004	33463	122	9.3	9.0	8.9	8.8
2005	34128	121	9.3	8.9	8.8	8.8
2006	35302	122	8.9	8.8	8.8	8.7
2007	35985	120	8.9	8.7	8.7	8.6
2008	34049	122	9.2	8.9	8.9	8.8
2009	34060	***	***	***	***	***



2009 TRAFFIC DATA

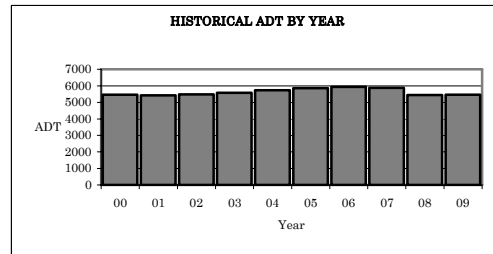
Month	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT	Classification Breakdown	
					Motorcycles	Percent of ADT
January	33027	97	31897	94	Motorcycles	0.12
February	34060	100	33391	98	Passenger cars	40.59
March	34556	101	33651	99	Light Trucks	52.76
April	35385	104	34778	102	Buses	0.18
May	34982	103	35003	103	Single unit trucks (2 axles)	3.07
June	36200	106	35300	104	Single unit trucks (3 axles)	0.71
July	37100	109	35900	105	Single unit trucks (4 or more axles)	0.18
August	36900	108	36100	106	Single trailer trucks (4 or less axles)	0.35
September	35217	103	34899	102	Single trailer trucks (5 axles)	0.90
October	34356	101	33468	98	Single trailer trucks (6 or more axles)	0.47
November	33658	99	32675	96	Multi trailer trucks (5 or less axles)	0.06
December	33477	98	31661	93	Multi trailer trucks (6 axles)	0.05
					Multi trailer trucks (7 or more axles)	0.56

Location: OR99W; MP 47.45; PACIFIC HIGHWAY WEST NO. 91; 0.07 mile north of Yamhill-Polk County Line

Site Name: Amity (36-005)
Installed: September, 1956

HISTORICAL TRAFFIC DATA

Year	Average Daily Traffic	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2000	5451	140	12.1	11.2	11.0	10.9
2001	5425	131	13.1	11.2	10.7	10.5
2002	5483	137	14.1	12.1	11.3	11.1
2003	5571	140	13.2	11.5	11.0	10.9
2004	5731	132	13.3	11.4	11.1	10.9
2005	5858	***	***	***	***	***
2006	5957	137	13.2	11.5	11.1	10.7
2007	5874	132	13.2	11.3	10.9	10.7
2008	5433	131	12.9	11.8	11.2	11.0
2009	5452	132	14.5	11.8	11.3	11.0



2009 TRAFFIC DATA

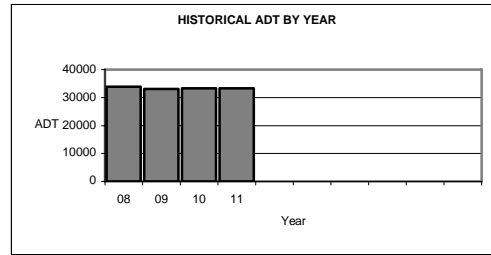
Month	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT	Classification Breakdown	
					Motorcycles	Percent of ADT
January	5138	94	4925	90	Motorcycles	1.04
February	5278	97	5188	95	Passenger cars	58.30
March	5335	98	5228	96	Light Trucks	29.49
April	5613	103	5571	102	Buses	0.28
May	5723	105	5770	106	Single unit trucks (2 axles)	2.56
June	5850	107	5700	105	Single unit trucks (3 axles)	2.18
July	5861	108	5678	104	Single unit trucks (4 or more axles)	0.02
August	5937	109	5743	105	Single trailer trucks (4 or less axles)	0.68
September	5796	106	5891	108	Single trailer trucks (5 axles)	4.78
October	5717	105	5628	103	Single trailer trucks (6 or more axles)	0.26
November	5566	102	5376	99	Multi trailer trucks (5 or less axles)	0.00
December	4987	91	4725	87	Multi trailer trucks (6 axles)	0.00
					Multi trailer trucks (7 or more axles)	0.41

Location: OR8; MP 14.84; TUALATIN VALLEY HIGHWAY NO. 29; 0.28 mile west of N.W. 334th Avenue

Site Name: Cornelius (34-009)
Installed: September, 2007

HISTORICAL TRAFFIC DATA

Year	ADT	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2008	33838	120	10.2	9.7	9.6	9.5
2009	33042	120	9.8	9.5	9.4	9.3
2010	33237	124	10.2	9.7	9.5	9.4
2011	33248	118	9.9	9.7	9.5	9.4



2011 TRAFFIC DATA

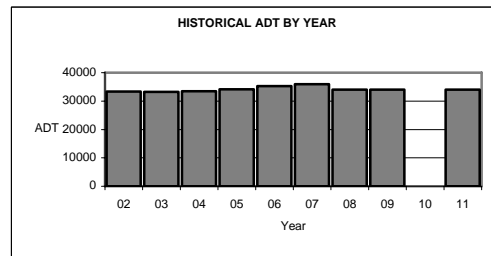
	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT	Classification Breakdown	
						Percent of ADT
January	32916	99	31555	95	Motorcycles	0.08
February	33372	100	32576	98	Passenger cars	73.27
March	33803	102	32713	98	Light Trucks	22.61
April	34568	104	33805	102	Buses	1.14
May	34036	102	33532	101	Single unit trucks (2 axles)	1.81
June	34972	105	34224	103	Single unit trucks (3 axles)	0.33
July	34129	103	33569	101	Single unit trucks (4 or more axles)	0.04
August	35092	106	34036	102	Single trailer trucks (4 or less axles)	0.21
September	34594	104	33683	101	Single trailer trucks (5 axles)	0.44
October	34558	104	33592	101	Single trailer trucks (6 or more axles)	0.03
November	33959	102	32726	98	Multi trailer trucks (5 or less axles)	0.00
December	34041	102	32967	99	Multi trailer trucks (6 axles)	0.00
					Multi trailer trucks (7 or more axles)	0.04

Location: OR99W/OR219; MP 21.81; PACIFIC HIGHWAY WEST NO. 91; 0.01 mile west of Brucher Street

Site Name: Newberg (36-004)
Installed: December, 1969

HISTORICAL TRAFFIC DATA

Year	ADT	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2002	33361	120	9.4	9.0	8.9	8.8
2003	33269	121	9.3	9.0	8.9	8.8
2004	33463	122	9.3	9.0	8.9	8.8
2005	34128	121	9.3	8.9	8.8	8.8
2006	35302	122	8.9	8.8	8.8	8.7
2007	35985	120	8.9	8.7	8.7	8.6
2008	34049	122	9.2	8.9	8.9	8.8
2009	34060	***	***	***	***	***
2010	***	***	***	***	***	***
2011	34083	120	9.6	9.0	8.9	8.8



2011 TRAFFIC DATA

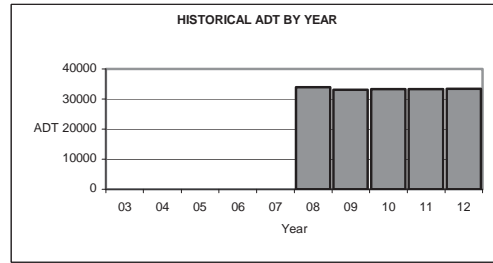
	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT	Classification Breakdown	
						Percent of ADT
January	32847	96	31532	93	Motorcycles	0.67
February	34044	100	33262	98	Passenger cars	74.57
March	34267	101	33055	97	Light Trucks	19.34
April	34939	103	34388	101	Buses	0.38
May	34640	102	34289	101	Single unit trucks (2 axles)	2.40
June	35834	105	35249	103	Single unit trucks (3 axles)	0.35
July	36605	107	36289	106	Single unit trucks (4 or more axles)	0.04
August	37129	109	36371	107	Single trailer trucks (4 or less axles)	0.35
September	35630	105	35253	103	Single trailer trucks (5 axles)	1.50
October	34830	102	34056	100	Single trailer trucks (6 or more axles)	0.33
November	33623	99	32733	96	Multi trailer trucks (5 or less axles)	0.01
December	33840	99	32518	95	Multi trailer trucks (6 axles)	0.00
					Multi trailer trucks (7 or more axles)	0.06

Location: OR8; MP 14.84; TUALATIN VALLEY HIGHWAY NO. 29; 0.28 mile west of N.W. 334th Avenue

Site Name: Cornelius (34-009)
Installed: September, 2007

HISTORICAL TRAFFIC DATA

Year	ADT	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2003	***	***	***	***	***	***
2004	***	***	***	***	***	***
2005	***	***	***	***	***	***
2006	***	***	***	***	***	***
2007	***	***	***	***	***	***
2008	33838	120	10.2	9.7	9.6	9.5
2009	33042	120	9.8	9.5	9.4	9.3
2010	33237	124	10.2	9.7	9.5	9.4
2011	33248	118	9.9	9.7	9.5	9.4
2012	33333	117	9.8	9.5	9.4	9.4



2012 TRAFFIC DATA

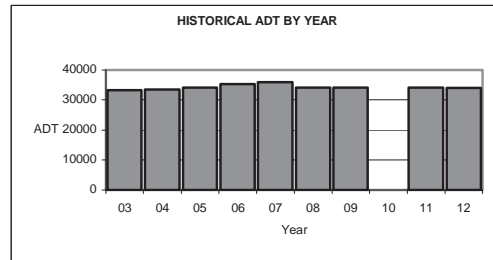
	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT	Classification Breakdown	
					Motorcycles	Percent of ADT
January	32848	99	31795	95	Motorcycles	0.14
February	34140	102	33156	99	Passenger cars	79.64
March	34127	102	33367	100	Light Trucks	16.69
April	35472	106	34789	104	Buses	1.10
May	34795	104	34405	103	Single unit trucks (2 axles)	1.27
June	35139	105	34360	103	Single unit trucks (3 axles)	0.40
July	34614	104	33916	102	Single unit trucks (4 or more axles)	0.04
August	34361	103	33371	100	Single trailer trucks (4 or less axles)	0.27
September	34262	103	33304	100	Single trailer trucks (5 axles)	0.35
October	33890	102	32785	98	Single trailer trucks (6 or more axles)	0.07
November	33297	100	32074	96	Multi trailer trucks (5 or less axles)	0.00
December	33313	100	32678	98	Multi trailer trucks (6 axles)	0.00
					Multi trailer trucks (7 or more axles)	0.03

Location: OR99W/OR219 MP 21.81; PACIFIC HIGHWAY WEST NO. 91; 0.01 mile west of Brutscher Street

Site Name: Newberg (36-004)
Installed: December, 1969

HISTORICAL TRAFFIC DATA

Year	ADT	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2003	33269	121	9.3	9.0	8.9	8.8
2004	33463	122	9.3	9.0	8.9	8.8
2005	34128	121	9.3	8.9	8.8	8.8
2006	35302	122	8.9	8.8	8.8	8.7
2007	35985	120	8.9	8.7	8.7	8.6
2008	34049	122	9.2	8.9	8.9	8.8
2009	34060	***	***	***	***	***
2010	***	***	***	***	***	***
2011	34083	120	9.6	9.0	8.9	8.8
2012	33969	122	9.0	8.8	8.7	8.7



2012 TRAFFIC DATA

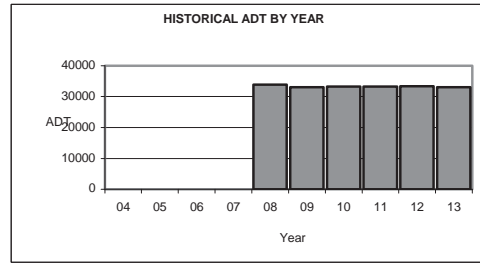
	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT	Classification Breakdown	
					Motorcycles	Percent of ADT
January	31633	93	30482	90	Motorcycles	0.67
February	34000	100	33300	98	Passenger cars	74.57
March	33700	99	33000	97	Light Trucks	19.34
April	35061	103	34568	102	Buses	0.38
May	34429	101	34533	102	Single unit trucks (2 axles)	2.40
June	35772	105	35146	103	Single unit trucks (3 axles)	0.35
July	36560	108	35996	106	Single unit trucks (4 or more axles)	0.04
August	37430	110	36833	108	Single trailer trucks (4 or less axles)	0.35
September	35510	105	35244	104	Single trailer trucks (5 axles)	1.50
October	34589	102	33793	99	Single trailer trucks (6 or more axles)	0.33
November	33753	99	32689	96	Multi trailer trucks (5 or less axles)	0.01
December	32850	97	32047	94	Multi trailer trucks (6 axles)	0.00
					Multi trailer trucks (7 or more axles)	0.06

Location: OR8; MP 14.84; TUALATIN VALLEY HIGHWAY NO. 29; 0.28 mile west of N.W. 334th Avenue

Site Name: Cornelius (34-009)
Installed: September, 2007

HISTORICAL TRAFFIC DATA

Year	ADT	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2004	***	***	***	***	***	***
2005	***	***	***	***	***	***
2006	***	***	***	***	***	***
2007	***	***	***	***	***	***
2008	33838	120	10.2	9.7	9.6	9.5
2009	33042	120	9.8	9.5	9.4	9.3
2010	33237	124	10.2	9.7	9.5	9.4
2011	33248	118	9.9	9.7	9.5	9.4
2012	33333	117	9.8	9.5	9.4	9.4
2013	33000	120	9.9	9.5	9.4	9.2



2013 TRAFFIC DATA

	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT
January	31830	96	30974	94
February	33718	102	32682	99
March	34232	104	33354	101
April	35075	106	33902	103
May	34549	105	34097	103
June	35192	107	34411	104
July	34613	105	33666	102
August	34126	103	33233	101
September	33630	102	32776	99
October	33609	102	32931	100
November	32775	99	32008	97
December	32890	100	31963	97

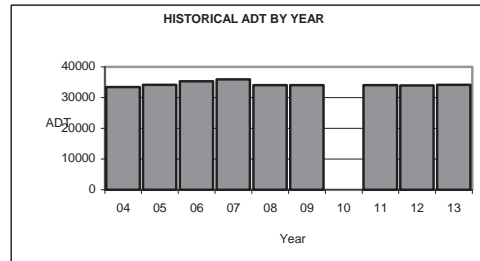
Classification Breakdown	Percent of ADT
Motorcycles	0.14
Passenger cars	79.64
Light Trucks	16.69
Buses	1.10
Single unit trucks (2 axles)	1.27
Single unit trucks (3 axles)	0.40
Single unit trucks (4 or more axles)	0.04
Single trailer trucks (4 or less axles)	0.27
Single trailer trucks (5 axles)	0.35
Single trailer trucks (6 or more axles)	0.07
Multi trailer trucks (5 or less axles)	0.00
Multi trailer trucks (6 axles)	0.00
Multi trailer trucks (7 or more axles)	0.03

Location: OR99W/OR219 MP 21.81; PACIFIC HIGHWAY WEST NO. 91; 0.01 mile west of Brutscher Street

Site Name: Newberg (36-004)
Installed: December, 1969

HISTORICAL TRAFFIC DATA

Year	ADT	Percent of ADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2004	33463	122	9.3	9.0	8.9	8.8
2005	34128	121	9.3	8.9	8.8	8.8
2006	35302	122	8.9	8.8	8.8	8.7
2007	35985	120	8.9	8.7	8.7	8.6
2008	34049	122	9.2	8.9	8.9	8.8
2009	34060	***	***	***	***	***
2010	***	***	***	***	***	***
2011	34083	120	9.6	9.0	8.9	8.8
2012	33969	122	9.0	8.8	8.7	8.7
2013	34174	120	9.0	8.8	8.7	8.6



2013 TRAFFIC DATA

	Average Weekday Traffic	Percent of ADT	Average Daily Traffic	Percent of ADT
January	32139	94	30666	90
February	34204	100	32996	97
March	34565	101	33957	99
April	35354	103	34467	101
May	34526	101	34535	101
June	35804	105	35433	104
July	36659	107	36125	106
August	37082	109	36294	106
September	35676	104	34868	102
October	35261	103	34752	102
November	34401	101	33268	97
December	33992	99	32728	96

Classification Breakdown	Percent of ADT
Motorcycles	0.13
Passenger cars	46.38
Light Trucks	48.71
Buses	0.29
Single unit trucks (2 axles)	2.35
Single unit trucks (3 axles)	0.39
Single unit trucks (4 or more axles)	0.09
Single trailer trucks (4 or less axles)	0.15
Single trailer trucks (5 axles)	1.07
Single trailer trucks (6 or more axles)	0.23
Multi trailer trucks (5 or less axles)	0.16
Multi trailer trucks (6 axles)	0.03
Multi trailer trucks (7 or more axles)	0.02

APPENDIX E
**IN-PROCESS
PROJECTS**

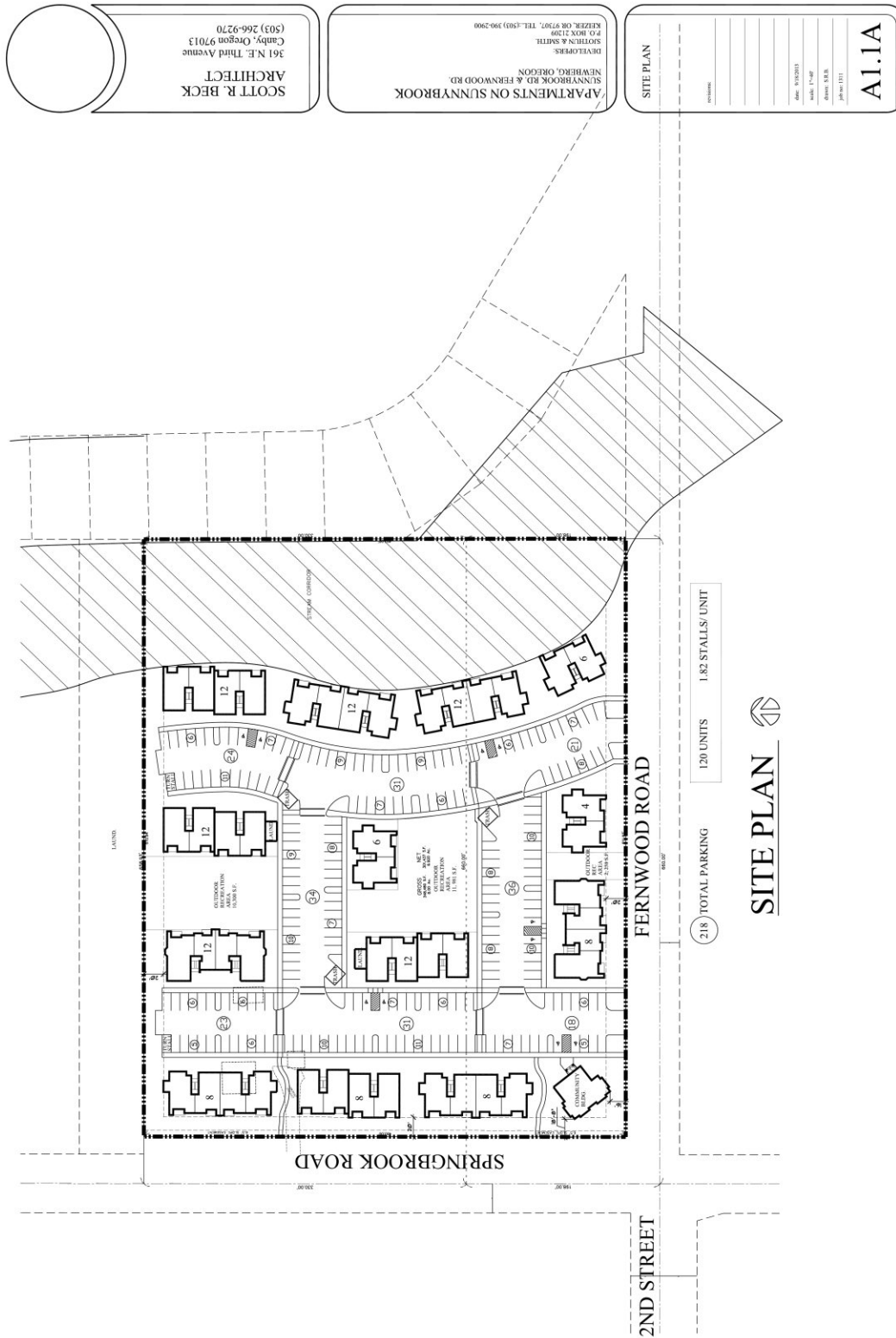


Figure 1 – Site Map

2008 and 2012.

SCENARIO 2 – PROPOSED DEVELOPMENT

Under this scenario, the entire site would be developed as R3 Multifamily Apartments which would be comprised of 120 units. For the purposes of this report the total development of the site was considered. This was done to show as near as possible the total impact of the development on the surrounding street system.

TRIP GENERATION

Estimates of total daily peak hour driveway volumes for the existing uses were developed from empirical observations at many similar-sized facilities located throughout the United States. These empirical observations are summarized in a standard reference manual published by the Institute of Transportation Engineers (ITE) *Trip Generation, 9th Edition* and the procedures and data provided in the *ITE Trip Generation Handbook*.

Estimating the number of vehicle trip ends that will be generated by the proposed development is of prime importance in order to accurately assess the impacts of development on the road network.

The figures shown in Table 4 below, represent the expected number of vehicle trip ends to be generated by the proposed development on a daily p.m. peak hour basis based on the mathematical regression equations found in Section 220 of the *Trip Generation Manual (9th Edition)*. This development is expected to generate approximately 851 trips per day

Land Use	Size	Peak Hour	
		Entry	Exit
220 - Apartments	120 Dwelling Units	55	29

For the purposes of this analysis, it is assumed that none of the home related person trips will be made by mass transit. This assumption ignores the recent statewide projections that 4-6 percent of all home based person trips for purposes unrelated to work will be made by transit. Therefore, this assumption tends to cause the analysis to reflect a worst-case condition, and over estimate the traffic impacts associated with this development.

TRIP DISTRIBUTION

The trip distribution for this report was determined from the traffic volumes shown in the field observation and review of the *Phase 1 Technical Report Addendum, Kittelson & Associates, Inc., September 2011*. See Figure 6.

Trip Distribution



Scenario 1: 1: Scenario 2 : w Newberg Apartments

Report File: F:\...\Scenario 2 with Newberg Apartments.pdf

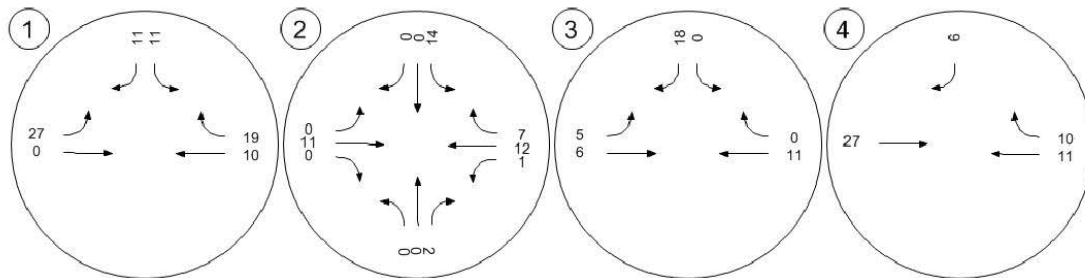
Figure 6 - Trip Distribution

Generated with **PTV VISTRO**
Version 2.00-01

Apartments on Sunnybrook Road

ATEP
9/27/2013

Traffic Volume - Net New Site Trips



Scenario 1: Scenario 2 w Newberg Apartments

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Figure 7 – Site Generated Peak Volumes

APPENDIX F
**CRASH DATA AND
SPIS LISTING**



2013 - On-State, Top 10% SPIS Groups - By Hwy, MP

2

Rte	Rdwy	BMP	EMP	Length	ADT	Crash	Fatal	A	B	C	PDO	City	County	Connection	Percent	SPIS
072 Salem																
OR-99EB	1	1.45	1.56	0.11	21,111	15	0	1	1	3	10	Salem	Marion	LEG (TO VERDA LN. NE)	90	44.94
OR-99EB	1	1.49	1.64	0.15	22,900	17	0	1	1	5	10	Salem	Marion	LEG (TO VERDA LN. NE)	90	49.24
OR-99EB	1	3.07	3.25	0.18	19,866	29	0	0	8	8	13	Salem	Marion	LEG (TO BRDWY ST. NE)	95	56.99
OR-99EB	1	4.28	4.37	0.09	17,077	22	0	0	5	6	11	Salem	Marion	GAINES ST. NE	90	45.14
OR-99EB	1	4.64	4.80	0.16	26,933	20	1	0	1	10	8	Salem	Marion		95	59.09
OR-22	1	5.35	5.53	0.18	14,511	53	0	0	2	18	33	Salem	Marion	COMMERCIAL ST.	95	73.13
OR-22	2	5.38	5.48	0.10	18,400	32	0	0	2	13	17	Salem	Marion	LIBERTY ST.	95	56.82
OR-22	1	6.68	6.87	0.19	35,100	41	0	1	3	13	24	Salem	Marion	17TH ST. SE	95	71.54
OR-22	1	7.14	7.24	0.10	33,000	18	0	1	2	6	9	Salem	Marion	22ND ST. SE	90	51.03
OR-22	1	7.43	7.50	0.07	33,000	22	0	0	2	12	8	Salem	Marion	24TH ST. SE (2ND LT.)	90	46.01
OR-22	1	7.44	7.62	0.18	47,400	57	0	0	3	23	31	Salem	Marion	25TH ST. SE	95	73.96
OR-22	1	7.83	8.02	0.19	47,400	52	1	0	5	17	29	Salem	Marion	LEG (FROM AIRPORT RD.)	95	76.11
OR-22	1	8.17	8.38	0.21	47,900	76	1	0	6	29	40	Salem	Marion	HAWTHORNE AVE.	95	80.40
081 Pacific Highway East																
OR-99E	1	31.80	31.96	0.16	19,200	6	1	1	0	3	1	Woodburn	Marion	MT. JEFFERSON AVE.	90	51.21
OR-99E	1	32.09	32.27	0.18	19,200	22	0	1	0	13	8	Woodburn	Marion	HARDCASTLE AVE.	95	58.85
OR-99E	1	32.78	32.94	0.16	18,500	32	0	0	5	9	18	Woodburn	Marion	TOMLIN AVE.	90	51.29
OR-99E	1	32.91	33.07	0.16	13,600	21	0	1	0	10	10	Woodburn	Marion	SILVERTON AVE.	95	60.24
OR-99E	1	44.36	44.47	0.11	10,099	13	0	1	1	4	7		Marion	NE HAZEL GREEN RD.	90	48.14
OR-99E	1	44.40	44.50	0.10	12,499	12	0	1	1	3	7		Marion	NE HAZEL GREEN RD.	90	45.23
091 Pacific Highway West																
OR-99W	1	21.71	21.87	0.16	34,100	19	0	1	2	8	8	Newberg	Yamhill	BRUTSCHER ST.	90	48.88
OR-99W	1	21.95	22.14	0.19	38,500	56	0	0	2	17	37	Newberg	Yamhill	LEG (TO N SPRINGBROOK RD.)	95	66.79

**Crash data shown in the SPIS group report results from the summation of crash data between the begin and end mile points of the Group.

**ADT, SPIS Score, and Percent data shown in the SPIS group report are the highest values from all sites within the Group.

10/23/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

COLLISION TYPE	FATAL CRASHES	NON- FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER- SECTION	INTER- SECTION RELATED	OFF- ROAD
YEAR: 2013														
FIXED / OTHER OBJECT	0	1	0	1	0	1	0	1	0	1	0	0	0	1
REAR-END	0	16	3	19	0	21	0	13	4	16	3	14	0	0
SIDESWIPE - OVERTAKING	0	0	2	2	0	0	0	2	0	2	0	0	0	0
TURNING MOVEMENTS	0	1	4	5	0	2	0	4	1	4	1	3	0	0
YEAR 2013 TOTAL	0	18	9	27	0	24	0	20	5	23	4	17	0	1
YEAR: 2012														
ANGLE	0	2	1	3	0	3	0	1	2	1	2	3	0	0
PEDESTRIAN	1	1	0	2	1	1	0	2	0	2	0	1	0	1
REAR-END	0	14	10	24	0	22	1	16	8	19	5	16	0	0
SIDESWIPE - OVERTAKING	0	1	0	1	0	2	0	0	1	1	0	0	0	0
TURNING MOVEMENTS	0	0	6	6	0	0	0	3	3	3	3	5	0	0
YEAR 2012 TOTAL	1	18	17	36	1	28	1	22	14	26	10	25	0	1
YEAR: 2011														
REAR-END	0	7	13	20	0	9	0	12	5	15	5	15	0	0
SIDESWIPE - OVERTAKING	0	0	1	1	0	0	0	1	0	1	0	1	0	0
TURNING MOVEMENTS	0	0	2	2	0	0	0	1	1	1	1	1	0	0
YEAR 2011 TOTAL	0	7	16	23	0	9	0	14	6	17	6	17	0	0
YEAR: 2010														
ANGLE	0	1	0	1	0	1	0	1	0	1	0	0	0	1
MISCELLANEOUS	0	1	0	1	0	1	0	1	0	1	0	0	0	1
PEDESTRIAN	0	1	0	1	0	1	0	1	0	1	0	1	0	0
REAR-END	0	6	11	17	0	9	0	10	7	12	5	8	0	0

10/23/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

COLLISION TYPE	FATAL CRASHES	NON-	PROPERTY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER-	INTER-	OFF- ROAD
		FATAL CRASHES	DAMAGE ONLY									SECTION	SECTION RELATED	
TURNING MOVEMENTS	0	1	3	4	0	1	0	4	0	3	1	2	0	0
YEAR 2010 TOTAL	0	10	14	24	0	13	0	17	7	18	6	11	0	2
YEAR: 2009														
ANGLE	0	2	0	2	0	2	0	2	0	1	1	2	0	0
REAR-END	0	9	9	18	0	14	0	13	4	16	2	14	2	0
TURNING MOVEMENTS	0	2	1	3	0	3	0	3	0	2	1	2	0	0
YEAR 2009 TOTAL	0	13	10	23	0	19	0	18	4	19	4	18	2	0
FINAL TOTAL	1	66	66	133	1	93	1	91	36	103	30	88	2	4

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OREGON DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
CONTINUOUS SYSTEM CRASH LISTING

091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	E A U C O DATE	COUNTY	RD# FC	COMPNT	CONN#	RD CHAR	INT-TYPE (MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	E L I C N S	P E D	E R R O R	ACT	E V E N T	C A U S E					
INVEST	D C S L K TIME	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	E V E N T	C A U S E	
CITY	TH	NEWBERG	MN 0	PACIFIC HY 99W	NE	(NONE)	UNKNOWN	N	DRY	REAR	01	PRVTE	NE-SW								000	000	00		
	4P	NEWBERG UA	21.79	BRUTSCHER ST	03	(04)		N	DAY	INJ		PSNGR CAR		01	DRVR	INJC	61	F	OR-Y OR<25		043	000	00	07	
												02	NONE 0 PRVTE PSNGR CAR		01	DRVR	INJC	47	F	OR-Y OR<25		000	000	00	00
												02	NONE 0 PRVTE PSNGR CAR		02	PSNG	INJC	49	M			000	000	00	00
00915	N N N	10/12/2012	YAMHILL	1 14				N	RAIN	S-1STOP	01	NONE 0	STRGHT											07	
NONE	FR	NEWBERG	MN 0	PACIFIC HY 99W	NE	(NONE)	UNKNOWN	N	WET	REAR		PRVTE	NE-SW									000	000	00	00
	4P	NEWBERG UA	21.79	BRUTSCHER ST	03	(04)		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	50	M	OR-Y OR>25		026	000	000	07	07
												02	NONE 0 PRVTE PSNGR CAR		01	DRVR	NONE	49	F	UNK N-RES		000	000	00	00
01007	N N N	11/14/2013	YAMHILL	1 14				N	CLR	S-1STOP	01	NONE 0	STRGHT											27	00
NO RPT	TH	NEWBERG	MN 0	PACIFIC HY 99W	NE	(NONE)	UNKNOWN	N	UNK	REAR		PRVTE	NE-SW									000	000	00	00
	5P	NEWBERG UA	21.79	BRUTSCHER ST	03	(04)		N	DUSK	INJ		PSNGR CAR		01	DRVR	NONE	56	F	OR-Y OR<25		016	038	000	27	27
												02	NONE 0 PRVTE PSNGR CAR		01	DRVR	INJC	24	M	OR-Y OR>25		000	000	00	00
00356	N N N	05/13/2009	YAMHILL	1 14				Y	RAIN	S-1STOP	01	NONE 0	STRGHT											07	00
NONE	WE	NEWBERG	MN 0	PACIFIC HY 99W	NE	(DIVMD)	UNKNOWN	N	WET	REAR		PRVTE	NE-SW									000	000	00	00
	4P	NEWBERG UA	21.79	BRUTSCHER ST	04	(04)		N	DAY	INJ		PSNGR CAR		01	DRVR	NONE	57	F	OR-Y OR<25		014	000	000	07	07
												02	NONE 0 PRVTE PSNGR CAR		01	DRVR	INJC	27	F	OR-Y OR>25		000	000	00	00
												02	NONE 0 PRVTE PSNGR CAR		02	PSNG	INJC	24	M			000	000	00	00
00594	N N N N N	08/07/2009	YAMHILL	1 14				N	CLD	S-1STOP	01	NONE 0	STRGHT											013	07
CITY	FR	NEWBERG	MN 0	BRUTSCHER ST	UN	0	TRF SIGNAL	N	DRY	REAR		PRVTE	N -S									000	000	00	00
	8A	NEWBERG UA	21.80	PACIFIC HY 99W	06			N	DAY	INJ		PSNGR CAR		01	DRVR	NONE	53	F	OR-Y OR<25		026	000	000	07	07
												02	NONE 0 PRVTE PSNGR CAR		01	DRVR	NONE	21	F	OR-Y OR<25		000	000	00	00
												03	NONE 0 PRVTE PSNGR CAR		01	DRVR	INJC	22	F	OR-Y		000	000	00	00

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091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	INVEST	D C S L K	TH	DATE	COUNTY	RD# FC	COMPNT	CONN#	RD CHAR	INT-TYPE	SPCL USE	MOVE	A S	INJ	G E LICNS	PED	ACT	EVENT	CAUSE		
ELGHRDAY	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC	ERROR			
NONE			5P		NEWBERG UA	21.99	0		03	(04)	NONE	0	01	DRVR	NONE	00	M	UNK	042	000	07
00932	N N N		SA	11/12/2011	YAMHILL	1	14		06	(04)	NONE	0	01	DRVR	NONE	66	M	OR-Y	000	000	00
			5P		NEWBERG UA	22.00	0		06	(04)	NONE	0	01	DRVR	NONE	66	M	OR-Y	000	000	00
00364	N N N		WE	05/01/2013	YAMHILL	1	14		06	(04)	NONE	0	01	DRVR	NONE	16	M	OR-Y	080	000	05
			3P		NEWBERG UA	22.01	0		06	(04)	NONE	0	01	DRVR	NONE	16	M	OR-Y	080	000	05
00029	N N N N N		TU	01/10/2012	YAMHILL	1	14		05	(04)	NONE	0	01	DRVR	NONE	71	F	OR-Y	029	026	02
			3P		NEWBERG UA	22.03	0		05	(04)	NONE	0	01	DRVR	NONE	71	F	OR-Y	029	026	02
00458	N N N		4P	06/01/2010	YAMHILL	1	14		06	(04)	NONE	0	01	DRVR	NONE	18	M	OR-Y	052,026	000	32,07
					NEWBERG UA	22.03	0		06	(04)	NONE	0	01	DRVR	NONE	18	M	OR-Y	052,026	000	32,07
00504	N N N N N		12P	07/05/2011	YAMHILL	1	14		08	(04)	NONE	0	01	DRVR	NONE	17	M	OR-Y	026	000	07
					NEWBERG UA	22.03	0		08	(04)	NONE	0	01	DRVR	NONE	17	M	OR-Y	026	000	07
00905	N N N		SU	10/13/2013	YAMHILL	1	14		NE	(RSDMD)	NONE	0	01	DRVR	NONE				000		07
					NEWBERG	MN	0		NE	(RSDMD)	NONE	0	01	DRVR	NONE				000		07

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091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	E L G H R DAY	CITY	MLG TYP	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E LICNS	PED	ACT	EVENT	CAUSE				
INVEST	D C S L K TIME	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC	ERROR	ACT	EVENT	CAUSE	
											02	NONE	0	STOP									
												PRVTE	NE-SW								011		00
												PSNGR	CAR	02	PSNG	INJC	15	F		000	000		00
											02	NONE	0	STOP									
												PRVTE	NE-SW								011		00
												PSNGR	CAR	03	PSNG	INJC	12	M		000	000		00
01058	N N N	11/17/2012	YAMHILL	1 14	STRGHT		N	N	RAIN	S-STRGHT	01	NONE	0	STRGHT									13,02
NO RPT	SA		NEWBERG	MN 0	PACIFIC HY 99W	NE	(NONE)	L-GRN-SIG	N	WET	SS-O	PRVTE	NE-SW								088		00
	3P		NEWBERG UA	22.04	SPRINGBROOK RD	04			N	DAY	INJ	PSNGR	CAR	01	DRVR	NONE	30	F	OR-Y	045,028	000		13,02
							(04)																
											02	NONE	0	STRGHT									
												PRVTE	NE-SW										00
												PSNGR	CAR	01	DRVR	INJC	35	F	OR-Y	000	000		00
																							00
											02	NONE	0	STRGHT									00
												PRVTE	NE-SW										00
												PSNGR	CAR	02	PSNG	INJC	14	F		000	000		00
00319	N N N N N	04/18/2013	YAMHILL	1 14	INTER	CROSS	N	N	CLR	S-OTHER	01	NONE	0	TURN-L									05
CITY	TH		NEWBERG	MN 0	PACIFIC HY 99W	UN		TRF SIGNAL	N	DRY	TURN	PRVTE	N -E								000		00
	8P		NEWBERG UA	22.05	SPRINGBROOK RD	05	0		N	DARK	INJ	PSNGR	CAR	01	DRVR	NONE	69	M	OR-Y	080	000		05
											02	NONE	0	TURN-L									00
												PRVTE	N -E										00
												PSNGR	CAR	01	DRVR	NONE	42	M	OR-Y	000	000		00
																							00
											02	NONE	0	TURN-L									00
												PRVTE	N -E										00
												PSNGR	CAR	02	PSNG	INJC	41	F		000	000		00
											02	NONE	0	TURN-L									00
												PRVTE	N -E										00
												PSNGR	CAR	03	PSNG	INJC	12	F		000	000		00
00021	N N N	01/12/2009	YAMHILL	1 14	INTER	CROSS	N	N	CLR	S-STRGHT	01	NONE	0	STRGHT									07
NO RPT	MO		NEWBERG	MN 0	PACIFIC HY 99W	UN		TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW								000		00
	8P		NEWBERG UA	22.05	SPRINGBROOK RD	06	2		N	DARK	INJ	PSNGR	CAR	01	DRVR	INJB	61	M	OR-Y	042	000		07
											02	NONE	0	STRGHT									00
												PRVTE	NE-SW										00
												PSNGR	CAR	01	DRVR	NONE	26	F	OR-Y	000	000		00
																							00
00732	N N N	09/11/2011	YAMHILL	1 14	INTER	CROSS	N	N	CLR	S-STRGHT	01	NONE	0	STRGHT									13
NONE	SU		NEWBERG	MN 0	PACIFIC HY 99W	NE		TRF SIGNAL	N	DRY	SS-O	PRVTE	SW-NE								000		00
	7P		NEWBERG UA	22.05	SPRINGBROOK RD	05	0		N	DAY	PDO	PSNGR	CAR	01	DRVR	NONE	00	F	OR-Y	045	000		13
											02	NONE	0	STRGHT									00
												PRVTE	SW-NE										00

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091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	DATE	COUNTY	RD# FC	COMPNT	CONN#	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A	S	INJ	G	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE
INVEST	D C S L K TIME	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE
CITY	SA 2P	NEWBERG NEWBERG UA	MN 0 22.05	PACIFIC HY 99W SPRINGBROOK RD	E 05	2	TRF SIGNAL	N	DRY	TURN	01	PRVTE	E -E	01	DRVR	NONE	32	M	OR-Y		008	000		00 08,01
											02	NONE PRVTE PSNGR CAR	TURN-R S -E										016 000	00 00
00711 CITY	NNNNN FR 4P	YAMHILL NEWBERG NEWBERG UA	1 14 MN 0 22.05	PACIFIC HY 99W SPRINGBROOK RD	INTER E 06	CROSS 0	N TRF SIGNAL	N	CLR DRY DAY	S-1STOP REAR PDO	01	POLCE PRVTE PSNGR CAR	STRGHT E -W	01	DRVR	NONE	39	M	OR-Y		026	000		07 00 07
											02	NONE PRVTE PSNGR CAR	STOP E -W										011 000	00 00
00125 CITY	NNN TU 7A	YAMHILL NEWBERG NEWBERG UA	1 14 MN 0 22.05	PACIFIC HY 99W SPRINGBROOK RD	INTER E 06	CROSS 0	N TRF SIGNAL	N	RAIN WET DAY	S-1STOP REAR INJ	01	NONE PRVTE PSNGR CAR	STRGHT E -W	01	DRVR	NONE	25	F	OR-Y		026	000		07 00 07
											02	NONE PRVTE PSNGR CAR	STOP E -W										011 000	00 00
00667 CITY	NNN WE 3P	YAMHILL NEWBERG NEWBERG UA	1 14 MN 0 22.05	PACIFIC HY 99W SPRINGBROOK RD	INTER E 06	CROSS 0	N NONE	N	CLR DRY DAY	S-1STOP REAR INJ	01	NONE PRVTE PSNGR CAR	STRGHT E -W	01	DRVR	NONE	59	M	OR-Y		052,026	000		013 00 32
											02	NONE PRVTE PSNGR CAR	STOP E -W										011 000	013 00
											03	NONE PRVTE PSNGR CAR	STOP E -W										011 000	00 00
00845 NO RPT	NNN TH 5P	YAMHILL NEWBERG NEWBERG UA	1 14 MN 0 22.05	PACIFIC HY 99W SPRINGBROOK RD	INTER E 06	CROSS 0	N TRF SIGNAL	N	RAIN WET DAY	S-1STOP REAR INJ	01	NONE PRVTE PSNGR CAR	STRGHT E -W	01	DRVR	NONE	23	F	OR-Y		026	000		07 00 07
											02	NONE PRVTE MTRCYCLE	STOP E -W										011 000	00 00
00745 CITY	NNNNN FR 2P	YAMHILL NEWBERG NEWBERG UA	1 14 MN 0 22.05	PACIFIC HY 99W SPRINGBROOK RD	INTER S 06	CROSS 0	N TRF SIGNAL	N	CLR DRY DAY	BIKE TURN INJ	01	NONE PRVTE PSNGR CAR	TURN-R S -NE	01	DRVR	NONE	55	M	OR-Y		027	000		02 00 02

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216/690

091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	E L G H R DAY	CITY	MLG TYP	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E LICNS	PED	ACT	EVENT	CAUSE				
INVEST	D C S L K TIME	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC	ERROR	ACT	EVENT	CAUSE	
												PRVTE	SW-NE							011		00	
												PSNGR CAR		01	DRVR	NONE	31 F	OR-Y		000	000		OR<25
01023	N N N	12/06/2011	YAMHILL	1 14	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT								07	
NONE	TU		NEWBERG	MN 0	PACIFIC HY 99W	SW			TRF SIGNAL	N	DRY	REAR		PRVTE	SW-NE						000	00	
	2P		NEWBERG UA	22.05	SPRINGBROOK RD	06	0	N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	00 M	UNK		026	000	07	
												PSNGR CAR											
												PRVTE	SW-NE								011	00	
												PSNGR CAR		01	DRVR	NONE	72 M	OR-Y		000	000	00	
																						OR<25	
01058	N N N	12/15/2011	YAMHILL	1 14	INTER	CROSS	N	N	CLD	S-1STOP	01	NONE	0	STRGHT								07	
NONE	TH		NEWBERG	MN 0	PACIFIC HY 99W	SW			TRF SIGNAL	N	WET	REAR		PRVTE	SW-NE						000	00	
	8A		NEWBERG UA	22.05	SPRINGBROOK RD	06	0	N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	25 M	OR-Y		026	000	07	
												PSNGR CAR											
												PRVTE	SW-NE									011	00
												PSNGR CAR		01	DRVR	NONE	40 M	OR-Y		000	000	00	
																						OR<25	
00066	N N N	01/21/2012	YAMHILL	1 14	INTER	CROSS	N	N	CLR	ANGL-STP	01	NONE	0	TURN-L								13	
NO RPT	SA		NEWBERG	MN 0	PACIFIC HY 99W	SW			UNKNOWN	N	DRY	TURN		PRVTE	SW-N						000	00	
	9P		NEWBERG UA	22.05	SPRINGBROOK RD	06	0	N	DARK	PDO		PSNGR CAR		01	DRVR	NONE	17 F	OR-Y		045	000	13	
												PSNGR CAR											
												PRVTE	SW-NE									012	00
												PSNGR CAR		01	DRVR	NONE	57 M	OR-Y		000	000	00	
																						OR<25	
00562	N N N	07/06/2012	YAMHILL	1 14	STRGHT		N	N	CLR	S-1STOP	01	NONE	0	STRGHT								07	
NO RPT	FR		NEWBERG	MN 0	PACIFIC HY 99W	SW	(NONE)		UNKNOWN	N	DRY	REAR		PRVTE	SW-NE						000	00	
	6P		NEWBERG UA	22.05	SPRINGBROOK RD	06		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	19 M	OR-Y		026	000	07	
							(04)					PSNGR CAR											
												PRVTE	SW-NE									011	00
												PSNGR CAR		01	DRVR	NONE	00 M	UNK		000	000	00	
																						UNK	
00818	N N N	09/21/2012	YAMHILL	1 14	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT								124	07
NONE	FR		NEWBERG	MN 0	PACIFIC HY 99W	SW			TRF SIGNAL	N	WET	REAR		PRVTE	SW-NE						000	124	00
	3P		NEWBERG UA	22.05	SPRINGBROOK RD	06	2	N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	19 M	OR-Y		026	000	07	
												PSNGR CAR											
												PRVTE	SW-NE									011	00
												PSNGR CAR		01	DRVR	NONE	26 F	OR-Y		000	000	00	
																						OR<25	
00868	N N N N N	10/02/2012	YAMHILL	1 14	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT								07	
CITY	TU		NEWBERG	MN 0	PACIFIC HY 99W	SW			TRF SIGNAL	N	DRY	REAR		PRVTE	SW-NE						000	00	
	5P		NEWBERG UA	22.05	SPRINGBROOK RD	06	2	N	DAY	INJ		PSNGR CAR		01	DRVR	NONE	44 M	OR-Y		043	000	07	
												PSNGR CAR											
												PRVTE	SW-NE									011	00

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091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	INVEST	D C S L K TIME	E A U C O DATE	COUNTY	RD# FC	COMPNT	CONN#	RD CHAR	INT-TYPE	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	TRLR	QTY	MOVE	A	S	E	LICNS	PED	OR<25	ERROR	ACT	EVENT	CAUSE
			ELGHR DAY	CITY	MLG TYP	FIRST STREET	DIRECT	LEGS	TRAF-		RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E									
				URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL		DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC					
00545	N N N	06/26/2013	WE	YAMHILL	1 14	PACIFIC HY 99W	SW	0	N		N	CLR	S-1STOP	01	NONE	0	01	DRVR	NONE	00	F	UNK			026	000		07
NONE		9A		NEWBERG	MN 0	PACIFIC HY 99W	SW	0	TRF SIGNAL		N	DRY	REAR		PRVTE													00
				NEWBERG UA	22.05	SPRINGBROOK RD	06	0			N	DAY	PDO	02	NONE	0	01	DRVR	NONE	46	F	OR-Y		000	000		00	
															PSNGR	CAR							OR<25					00
00688	N N N	08/06/2013	TU	YAMHILL	1 14	PACIFIC HY 99W	SW	0	N		N	UNK	S-1STOP	01	NONE	0	01	DRVR	NONE	18	M	OR-Y			026	000		07
NONE		3P		NEWBERG	MN 0	PACIFIC HY 99W	SW	0	TRF SIGNAL		N	UNK	REAR		PRVTE													00
				NEWBERG UA	22.05	SPRINGBROOK RD	06	0			N	DAY	PDO	02	NONE	0	01	DRVR	NONE	51	M	OR-Y		000	000		00	
															PSNGR	CAR							OR<25					00
01203	N N N	12/31/2013	5P	YAMHILL	1 14	PACIFIC HY 99W	SW	2	N		N	CLR	S-1STOP	01	NONE	0	01	DRVR	NONE	41	F	OR-Y			026	000		07
NO RPT				NEWBERG	MN 0	PACIFIC HY 99W	SW	0	TRF SIGNAL		N	DRY	REAR		PRVTE													00
				NEWBERG UA	22.05	SPRINGBROOK RD	06	2			N	DUSK	INJ	02	NONE	0	01	DRVR	NONE	61	F	OR-Y		000	000		00	
															PSNGR	CAR							OR<25					00
															PSNGR	CAR							OR<25					00
00038	Y N N	01/12/2011	5P	YAMHILL	1 14	PACIFIC HY 99W	W	0	N		N	RAIN	S-STRGHT	01	NONE	0	01	DRVR	NONE	58	F	OR-Y		042	000		01	
NONE				NEWBERG	MN 0	PACIFIC HY 99W	W	0	TRF SIGNAL		N	WET	REAR		PRVTE													00
				NEWBERG UA	22.05	SPRINGBROOK RD	06	0			N	DARK	INJ	02	NONE	0	01	DRVR	NONE	47	F	OR-Y		000	000		00	
															PSNGR	CAR							OR<25					00
00168	N N N	03/02/2011	4P	YAMHILL	1 14	PACIFIC HY 99W	W	0	N		N	UNK	S-1STOP	01	NONE	0	01	DRVR	NONE	34	M	OR-Y		026	000		07, 27	
NONE				NEWBERG	MN 0	PACIFIC HY 99W	W	0	UNKNOWN		N	UNK	REAR		PRVTE													00
				NEWBERG UA	22.05	SPRINGBROOK RD	06	0			N	DAY	PDO	02	NONE	0	01	DRVR	NONE	35	M	OR-Y		000	000		00	
															PSNGR	CAR							OR<25					00
00493	Y N N	07/01/2011	12P	YAMHILL	1 14	PACIFIC HY 99W	W	0	N		N	CLR	S-1STOP	01	NONE	0	01	DRVR	NONE	00	Unk	UNK			026	000		01
CITY				NEWBERG	MN 0	PACIFIC HY 99W	W	0	TRF SIGNAL		N	DRY	REAR		PRVTE													00
				NEWBERG UA	22.05	SPRINGBROOK RD	06	0			N	DAY	PDO		PSNGR	CAR							UNK					01
																							UNK					01

091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	INVEST	D C S L K	TIME	URBAN AREA	RD# FC	COMPNT	CONN#	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	E LICNS	PED	ACT	EVENT	CAUSE
EAUCO DATE	COUNTY	MLG TYP	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E	RES	LOC	ERROR				
ELGHR DAY	CITY	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X					
														02	NONE	0					
															PRVTE						011
															PSNGR	CAR					000
																					000
														02	NONE	0					
															PRVTE						011
															PSNGR	CAR					000
00686	N N N		08/24/2011	YAMHILL	1 14			INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0					
NONE				NEWBERG	MN 0	PACIFIC HY 99W		W		UNKNOWN	N	DRY	REAR		PRVTE						000
		4P		NEWBERG UA	22.05	SPRINGBROOK RD		06	0		N	DAY	PDO		PSNGR	CAR					026
																					000
														02	NONE	0					
															PRVTE						011
															PSNGR	CAR					000
00138	N N N		02/21/2013	YAMHILL	1 14			INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0					
NONE				NEWBERG	MN 0	PACIFIC HY 99W		W		YIELD	N	DRY	REAR		PRVTE						000
		3P		NEWBERG UA	22.05	SPRINGBROOK RD		06	2		N	DAY	INJ		PSNGR	CAR					026
																					000
														02	NONE	0					
															PRVTE						011
															PSNGR	CAR					000
00895	N N N		10/19/2010	YAMHILL	1 14			INTER	CROSS	N	N	CLR	O-1TURN	01	NONE	0					
NO RPT				NEWBERG	MN 0	PACIFIC HY 99W		CN		TRF SIGNAL	N	DRY	TURN		PRVTE						000
		8A		NEWBERG UA	22.05	SPRINGBROOK RD		02	0		N	DAY	INJ		PSNGR	CAR					020
																					000
														02	NONE	0					
															PRVTE						000
															PSNGR	CAR					000
00821	N N N		09/21/2012	YAMHILL	1 14			INTER	CROSS	N	N	RAIN	O-1TURN	01	NONE	0					
NO RPT				NEWBERG	MN 0	PACIFIC HY 99W		CN		L-GRN-SIG	N	WET	TURN		PRVTE						000
		6A		NEWBERG UA	22.05	SPRINGBROOK RD		02	2		N	DAY	PDO		PSNGR	CAR					020
																					000
														02	NONE	0					
															PRVTE						000
															PSNGR	CAR					000
01045	N N N		11/15/2012	YAMHILL	1 14			INTER	CROSS	N	N	CLD	S-1STOP	01	NONE	0					092
NONE				NEWBERG	MN 0	PACIFIC HY 99W		CN		TRF SIGNAL	N	DRY	REAR		PRVTE						000
		3P		NEWBERG UA	22.05	SPRINGBROOK RD		02	2		N	DAY	INJ		PSNGR	CAR					026
																					000
														02	NONE	0					
															PRVTE						011
															PSNGR	CAR					000
00146	N N N		02/19/2009	YAMHILL	1 14			INTER	CROSS	N	N	CLR	ANGL-OTH	01	NONE	0					
																					04

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091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	INVEST	D C S L K	TIME	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE
NONE			TH	NEWBERG	MN 0	PACIFIC HY 99W	CN		TRF SIGNAL	N	DRY	TURN		PRVTE	SW-S								000	000	00	
			6P	NEWBERG UA	22.05	SPRINGBROOK RD	03	2		Y	DARK	PDO		PSNGR CAR		01	DRVR	NONE	50	F	OR-Y		020	000	04	OR<25
														02	NONE	0	STRGHT							000	000	00
														PRVTE	N -S									000	000	00
														PSNGR CAR		01	DRVR	NONE	46	F	OR-Y		000	000	00	OR<25
00430		N N N N N	06/08/2009	YAMHILL	1 14		INTER	CROSS	N	N	CLR	ANGL-OTH	01	NONE	0	TURN-L									32,04	
CITY			MO	NEWBERG	MN 0	PACIFIC HY 99W	CN		TRF SIGNAL	N	DRY	ANGL		PRVTE	N -NE								000	000	00	
			5P	NEWBERG UA	22.05	SPRINGBROOK RD	03	0		N	DAY	INJ		PSNGR CAR		01	DRVR	NONE	56	F	OR-Y		052,020	000	00	32,04
														02	NONE	0	STRGHT							000	000	00
														PRVTE	SW-NE									000	000	00
														PSNGR CAR		01	DRVR	INJB	30	F	OR-Y		000	000	00	OR<25
00400		N N N	05/28/2011	YAMHILL	1 14		INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT									07	
NO RPT			SA	NEWBERG	MN 0	PACIFIC HY 99W	CN		TRF SIGNAL	N	DRY	REAR		PRVTE	W -E								000	000	00	
			4P	NEWBERG UA	22.05	SPRINGBROOK RD	03	2		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	68	M	OR-Y		026	000	07	OR>25
														02	NONE	0	STOP							011	000	00
														PRVTE	W -E									000	000	00
														PSNGR CAR		01	DRVR	NONE	33	F	OR-Y		000	000	00	OR<25
00272		N N N N N	03/27/2012	YAMHILL	1 14		INTER	CROSS	N	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT									03	
CITY			TU	NEWBERG	MN 0	PACIFIC HY 99W	CN		TRF SIGNAL	N	WET	ANGL		PRVTE	W -E								000	000	00	
			9P	NEWBERG UA	22.05	SPRINGBROOK RD	03	0		N	DARK	INJ		PSNGR CAR		01	DRVR	NONE	17	F	OR-Y		021	000	03	OR<25
														02	NONE	0	STRGHT							000	000	00
														PRVTE	N -S									000	000	00
														PSNGR CAR		01	DRVR	INJC	63	M	OR-Y		000	000	00	OR<25
00922		N N N N N	10/15/2012	YAMHILL	1 14		INTER	CROSS	N	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT									02	
CITY			MO	NEWBERG	MN 0	PACIFIC HY 99W	CN		TRF SIGNAL	N	WET	ANGL		PRVTE	W -E								000	000	00	
			7P	NEWBERG UA	22.05	SPRINGBROOK RD	03	2		N	DARK	PDO		PSNGR CAR		01	DRVR	NONE	65	M	OR-Y		028	000	02	OR<25
														02	NONE	0	STRGHT							015	000	00
														PRVTE	N -S									000	000	00
														PSNGR CAR		01	DRVR	NONE	56	M	OR-Y		000	000	00	OR<25
01182		N N N	12/03/2010	YAMHILL	1 14		INTER	CROSS	N	N	CLR	S-OTHER	01	NONE		TURN-L									05	
NO RPT			FR	NEWBERG	MN 0	PACIFIC HY 99W	CN		TRF SIGNAL	N	DRY	TURN		PRVTE	N -E								000	000	00	
			2P	NEWBERG UA	22.05	SPRINGBROOK RD	04	2		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	64	M	OR-Y		080	000	05	OR<25
														02	NONE	0	TURN-L							000	000	00
														PRVTE	N -E									000	000	00
														PSNGR CAR		01	DRVR	NONE	44	F	OR-Y		000	000	00	OR<25
00894		Y N N N N	11/02/2011	YAMHILL	1 14		STRGHT		N	N	FOG	S-1STOP	01	NONE	0	STRGHT									32,01	
CITY			WE	NEWBERG	MN 0	PACIFIC HY 99W	S	(RSDMD)	UNKNOWN	N	WET	REAR		PRVTE	SW-NE								000	000	00	

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224/690

091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	E A U C O DATE	COUNTY	RD# FC	COMPNT	CONN#	RD CHAR	INT-TYPE	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	INJ	G E	LICNS	PED	ERROR	ACT	EVENT	CAUSE	
INVEST	D C S L K TIME	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E X	RES	LOC	052,047,026	000		32,01	
	8A	NEWBERG UA	22.07	SPRINGBROOK RD	05		(04)	N	DAWN	INJ	PSNGR CAR		01 DRVR	NONE	50	F	OR-Y					
											02 NONE 0	STOP SW-NE								011	00	
											PSNGR CAR		01 DRVR	INJC	40	F	OR-Y	000	000		00	
																					00	
80153	01/12/2010	YAMHILL	1 14			STRGHT	N	N	RAIN	S-1STOP	01 NONE 0	STRGHT									07	
NONE	TU	NEWBERG	MN 0	PACIFIC HY 99W	NE	(RSDMD)	UNKNOWN	N	WET	REAR	PRVTE	NE-SW								000	00	
	8P	NEWBERG UA	22.09	SPRINGBROOK RD	03		(04)	N	DLIT	PDO	PSNGR CAR		01 DRVR	NONE	00	M	OR-Y	026	000		07	
											02 NONE 0	STOP NE-SW									011	00
											PSNGR CAR		01 DRVR	NONE	74	F	OR-Y	000	000		00	
																					00	
																					00	
00037	01/06/2009	YAMHILL	1 14			STRGHT	N	N	CLR	S-1STOP	01 NONE 0	STRGHT									27,07	
NONE	TU	NEWBERG	MN 0	PACIFIC HY 99W	SW	(NONE)	UNKNOWN	N	DRY	REAR	PRVTE	SW-NE								000	00	
	4P	NEWBERG UA	22.10	SPRINGBROOK RD	06		(04)	N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	19	M	OR-Y	016,026	000		27,07	
											02 NONE 0	STOP SW-NE									011	00
											PSNGR CAR		01 DRVR	NONE	52	M	OR-Y	000	000		00	
																					00	
00818	09/29/2010	YAMHILL	1 14			ALLEY	N	Y	CLR	BIKE	01 NONE	TURN-R									02	
NO RPT	WE	NEWBERG	MN 0	PACIFIC HY 99W	SW	(NONE)	NONE	N	DRY	ANGL	PRVTE	NW-SW								000	00	
	2P	NEWBERG UA	22.11	SPRINGBROOK RD	01		(04)	N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	52	F	OR-Y	027	000		02	
																					00	
												-	01 BIKE	INJB	18	M		SIDEWK	000	000	00	
												STRGHT NE SW									00	
00662	08/05/2012	YAMHILL	1 14			STRGHT	N	N	CLR	S-1STOP	01 NONE 0	STRGHT									07	
CITY	SU	NEWBERG	MN 0	PACIFIC HY 99W	UN	(NONE)	NONE	N	DRY	REAR	PRVTE	NE-SW								000	00	
	3P	NEWBERG UA	22.12	SPRINGBROOK RD	07		(04)	Y	DAY	INJ	PSNGR CAR		01 DRVR	NONE	58	M	OR-Y	026	000		07	
											02 NONE 0	STOP NE-SW									011	00
											PSNGR CAR		01 DRVR	INJC	45	M	OR-Y	000	000		00	
																					00	
																					00	
00362	04/20/2012	YAMHILL	1 14			ALLEY	N	N	CLD	ANGL-OTH	01 NONE 0	TURN-L									02	
NO RPT	FR	NEWBERG	MN 0	PACIFIC HY 99W	SW	(NONE)	NONE	N	WET	TURN	PRVTE	N-NE								000	00	
	1P	NEWBERG UA	22.12	SPRINGBROOK RD	05		(04)	N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	34	F	OR-Y	028	000		02	
											02 NONE 0	STRGHT SW-NE									000	00
											PSNGR CAR		01 DRVR	NONE	62	M	OR-Y	000	000		00	
																					00	
00727	09/17/2009	YAMHILL	1 14			ALLEY	N	N	CLR	ANGL-OTH	01 NONE 0	STRGHT									02	
CITY	TH	NEWBERG	MN 0	PACIFIC HY 99W	SW	(NONE)	NONE	N	DRY	TURN	PRVTE	SW-NE								000	00	
	11A	NEWBERG UA	22.12	DEBORAH ST	06		(04)	N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	19	F	OR-Y	000	000		00	

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225/690

091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	DATE	COUNTY	RD#	FC	INT-TYPE	SPCL USE	ACT	EVENT	CAUSE																
INVEST	D C S L K TIME	URBAN AREA	MILEPNT	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE	
							(04)					01	NONE	0		STRGHT									OR<25
												PRVTE	SW-NE										000		00
												PSNGR	CAR	02	PSNG	INJB	40	F				000	000		00
												02	NONE	0		TURN-L									
												PRVTE	S -SW										018		00
												PSNGR	CAR	01	DRVR	INJC	24	F	OR-Y			028	000		02
																									OR<25
90179	03/09/2013	YAMHILL	1	14	STRGHT		N	Y	CLR	FIX OBJ	01	NONE	0		TURN-L									058	16
CITY	SA	NEWBERG	MN	0	PACIFIC HY 99W	W	(NONE)	NONE	N	DRY	FIX	PRVTE	NE-S										000	058	00
	1P	NEWBERG UA	22.12	SPRINGBROOK RD	08			N	DAY	INJ		PSNGR	CAR	01	DRVR	INJC	23	F	OR-Y			081	000		16
							(04)																		
01033	11/24/2010	YAMHILL	1	14	STRGHT		N	N	CLR	S-1STOP	01	NONE	0		STRGHT										07
NONE	WE	NEWBERG	MN	0	PACIFIC HY 99W	NE	(NONE)	UNKNOWN	N	DRY	REAR	PRVTE	SW-NE										000		00
	1P	NEWBERG UA	22.14	DEBORAH ST	06			N	DAY	PDO		PSNGR	CAR	01	DRVR	NONE	52	F	OR-Y			026	000		07
							(04)																		
												02	NONE	0		STOP									
												PRVTE	SW-NE										011		00
												PSNGR	CAR	01	DRVR	NONE	00	M	OR-Y			000	000		00
																									UNK
00510	06/27/2010	YAMHILL	1	14	STRGHT		N	N	CLR	S-1STOP	01	NONE	0		STRGHT										07
NO RPT	SU	NEWBERG	MN	0	DEBORAH ST	E	(NONE)	TRF SIGNAL	N	DRY	REAR	PRVTE	S -N										000		00
	7P	NEWBERG UA	22.14	PACIFIC HY 99W	07			N	DAY	PDO		PSNGR	CAR	01	DRVR	NONE	60	M	OR-Y			026	000		07
							(04)																		
												02	NONE	0		STOP									
												PRVTE	S -N										011		00
												PSNGR	CAR	01	DRVR	NONE	39	F	OR-Y			000	000		00
																									OR<25
00341	05/04/2010	YAMHILL	1	14	STRGHT		N	N	CLR	S-1STOP	01	NONE	0		STRGHT										07
NONE	TU	NEWBERG	MN	0	PACIFIC HY 99W	W	(NONE)	UNKNOWN	N	DRY	REAR	PRVTE	W -E										000		00
	1P	NEWBERG UA	22.15	SPRINGBROOK RD	08			N	DAY	INJ		PSNGR	CAR	01	DRVR	NONE	36	F	OR-Y			026	000		07
							(04)																		
												01	NONE	0		STRGHT									
												PRVTE	W -E										000		00
												PSNGR	CAR	02	PSNG	INJC	05	M				000	000		00
												02	NONE	0		STOP									
												PRVTE	W -E										011		00
												PSNGR	CAR	01	DRVR	NONE	28	F	OTH-Y			000	000		00
																									OR<25
00697	08/08/2012	YAMHILL	1	14	STRGHT		N	N	CLR	S-1STOP	01	NONE	0		STRGHT										07
NONE	WE	NEWBERG	MN	0	PACIFIC HY 99W	W	(NONE)	UNKNOWN	N	DRY	REAR	PRVTE	W -E										000		00
	7A	NEWBERG UA	22.15	SPRINGBROOK RD	08			N	DAY	INJ		PSNGR	CAR	01	DRVR	NONE	00	M	UNK			026	000		07
							(04)																		
												02	NONE	0		STOP									
												PRVTE	W -E										011		00
												PSNGR	CAR	01	DRVR	INJC	48	F	OR-Y			000	000		00
																									OR<25

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091: PACIFIC HIGHWAY WEST

Highway 091 ALL ROAD TYPES, MP 21.70 to 22.20 01/01/2009 to 12/31/2013, Both Add and Non-Add mileage

Total crash records: 133

SER#	DATE	COUNTY	RD# FC	INT-TYPE	SPCL USE	TRLR QTY	MOVE	A S	INJ	G E	LICNS	PED	ACT	EVENT	CAUSE
INVEST	D C S L K TIME	URBAN AREA	MILEPNT SECOND STREET	LOCTN	(#LANES) CONTL	DRVWY LIGHT SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E X RES	LOC	ERROR		
							PRVTE	W -E					000	000	00
							PSNGR CAR		01 DRVR	NONE	70 M	OR-Y	000	000	00
												OR<25			
							04 NONE 0	STRGHT						000	000
							PRVTE	S -N							00
							PSNGR CAR		01 DRVR	INJC	49 F	OR-Y	000	000	00
												OR<25			
00986	11/15/2010	YAMHILL	1 14	STRGHT	N	RAIN	01 NONE 0	STRGHT							07
NO RPT	MO	NEWBERG	MN 0 PACIFIC HY 99W	NE	(NONE)	WET	PRVTE	NE-SW						000	00
	5P	NEWBERG UA	22.20 DEBORAH ST	04		DLIT	PSNGR CAR		01 DRVR	NONE	36 M	OR-Y	026	000	07
												OR<25			
							02 NONE 0	STOP						011	00
							PRVTE	NE-SW						000	00
							PSNGR CAR		01 DRVR	NONE	40 F	OR-Y	000	000	00
												OR<25			
01032	11/24/2010	YAMHILL	1 14	STRGHT	N	CLR	01 NONE 0	U-TURN							08
NO RPT	WE	NEWBERG	MN 0 PACIFIC HY 99W	NE	(NONE)	DRY	PRVTE	NE-NE						000	00
	5P	NEWBERG UA	22.20 DEBORAH ST	04		DLIT	PSNGR CAR		01 DRVR	NONE	19 F	OR-Y	008	000	08
												OR<25			
							02 NONE 0	STRGHT						000	00
							PRVTE	NE-SW						000	00
							PSNGR CAR		01 DRVR	NONE	39 M	OR-Y	000	000	00
												OR<25			
01121	12/13/2013	YAMHILL	1 14	STRGHT	N	CLD	01 NONE 0	STRGHT						013	01,27
STATE	FR	NEWBERG	MN 0 PACIFIC HY 99W	E	(NONE)	WET	PRVTE	SW-NE						022	00
	11A	NEWBERG UA	22.20 DEBORAH ST	06		DAY	PSNGR CAR		01 DRVR	NONE	20 F	NONE	016	038	01,27
												OR<25			
							02 NONE 0	STRGHT						022	013
							PRVTE	SW-NE						000	00
							PSNGR CAR		01 DRVR	INJC	78 F	OR-Y	000	000	00
												OR>25			
							03 NONE 0	STOP						011	00
							PRVTE	SW-NE						000	00
							PSNGR CAR		01 DRVR	NONE	48 F	OR-Y	000	000	00
												OR<25			

10/30/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL CRASHES	NON- FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER- SECTION	INTER- SECTION RELATED	OFF- ROAD
YEAR: 2013														
ANGLE	0	0	1	1	0	0	0	1	0	1	0	1	0	0
FIXED / OTHER OBJECT	1	0	2	3	1	0	0	2	1	2	1	1	0	3
REAR-END	0	10	5	15	0	14	0	10	4	13	2	15	0	0
TURNING MOVEMENTS	0	2	1	3	0	3	0	3	0	2	1	3	0	0
YEAR 2013 TOTAL	1	12	9	22	1	17	0	16	5	18	4	20	0	3
YEAR: 2012														
ANGLE	0	1	1	2	0	1	0	0	2	0	2	2	0	0
BACKING	0	0	1	1	0	0	0	0	1	1	0	0	0	0
PEDESTRIAN	0	2	0	2	0	2	0	2	0	2	0	2	0	0
REAR-END	0	8	7	15	0	12	0	10	5	12	3	14	0	0
TURNING MOVEMENTS	0	3	6	9	0	3	0	3	5	5	4	8	0	0
YEAR 2012 TOTAL	0	14	15	29	0	18	0	15	13	20	9	26	0	0
YEAR: 2011														
REAR-END	0	2	14	16	0	2	0	11	3	14	2	16	0	0
SIDESWIPE - OVERTAKING	0	0	1	1	0	0	0	1	0	1	0	1	0	0
TURNING MOVEMENTS	0	0	1	1	0	0	0	1	0	1	0	1	0	0
YEAR 2011 TOTAL	0	2	16	18	0	2	0	13	3	16	2	18	0	0
YEAR: 2010														
ANGLE	0	0	1	1	0	0	0	1	0	1	0	1	0	0
REAR-END	0	5	5	10	0	8	0	5	5	7	3	10	0	0

10/30/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL	NON-	PROPERTY	TOTAL	PEOPLE	PEOPLE	TRUCKS	DRY	WET	DAY	DARK	INTER-	INTER-	OFF-
	CRASHES	FATAL	DAMAGE	CRASHES	KILLED	INJURED		SURF	SURF			SECTION	SECTION	ROAD
TURNING MOVEMENTS	0	3	3	6	0	3	1	4	2	5	1	6	0	0
YEAR 2010 TOTAL	0	8	9	17	0	11	1	10	7	13	4	17	0	0
YEAR: 2009														
ANGLE	0	2	1	3	0	2	0	3	0	3	0	3	0	0
FIXED / OTHER OBJECT	0	0	2	2	0	0	0	1	1	2	0	0	0	2
REAR-END	0	7	5	12	0	9	0	9	2	11	1	12	0	0
TURNING MOVEMENTS	0	1	2	3	0	1	0	3	0	1	2	3	0	0
YEAR 2009 TOTAL	0	10	10	20	0	12	0	16	3	17	3	18	0	2
FINAL TOTAL	1	46	59	106	1	60	1	70	31	84	22	99	0	5

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CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 106

SER#	E L G H R DAY	CLASS	CITY STREET	RD CHAR	INT-TYPE			OFFRD	WTHR	CRASH	SPCL USE		MOVE	A S	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE			
					(MEDIAN)	INT-REL	TRAF-				TRLR QTY	TYPE											
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC						
	3P			06	0		N	DAY	INJ		PSNGR CAR		01	DRVR	NONE	62 M	OR-Y		026	000	07		
											02 NONE 0	STOP											
											PRVTE	E -W									011	00	
											PSNGR CAR		01	DRVR	INJC	30 M	OR-Y		000	000	000	00	
																						OR<25	
00706	N N N	08/30/2010	14	HILLSBORO-SILV HY	INTER	CROSS	N	CLR	S-1STOP	01	NONE 0	STRGHT									006	07	
NONE	MO			SPRINGBROOK RD	S		TRF SIGNAL	N	DRY REAR		PRVTE	S -N									000	00	
	5P				06	0		N	DAY	INJ	PSNGR CAR		01	DRVR	NONE	48 F	OR-Y		026	000	000	07	
											02 NONE	STOP											OR<25
											PRVTE	S -N									011	006	00
											PSNGR CAR		01	DRVR	INJC	44 F	OR-Y		000	000	000	000	00
																							OR<25
00242	N N N	04/20/2009	16	HAWORTH AVE	INTER	3-LEG	N	CLR	S-1STOP	01	NONE 0	STRGHT										07	
NONE	MO			SPRINGBROOK RD	N		STOP SIGN	N	DRY REAR		PRVTE	N -S									000	00	
	1P				06	0		N	DAY	INJ	PSNGR CAR		01	DRVR	NONE	20 M	OR-Y		026	000	000	07	
											02 NONE	STOP											OR<25
											PRVTE	N -S									011		00
											PSNGR CAR		01	DRVR	INJC	51 F	OR-Y		000	000	000	000	00
																							OR<25
00274	N N N N N	03/27/2012	17	HAWORTH AVE	INTER	CROSS	N	CLR	PED	01	NONE 0	TURN-L										02	
CITY	TU			SPRINGBROOK RD	N		STOP SIGN	N	DRY		PRVTE	E -S									000	00	
	2P				06	0		N	DAY	INJ	PSNGR CAR		01	DRVR	NONE	33 M	OR-Y		029	000	000	02	
																							OR<25
												-											
												STRGHT	01	CONV	INJB	53 M					I XWLK	000	000
												N S											00
00286	N N N	03/31/2012	17	HAWORTH AVE	INTER	CROSS	N	CLR	S-1STOP	01	NONE 0	STRGHT										07	
NONE	SA			SPRINGBROOK RD	S		UNKNOWN	N	DRY REAR		PRVTE	S -N									000	00	
	4P				06	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	32 F	OR-Y		026	000	000	07	
											02 NONE	STOP											OR<25
											PRVTE	S -N									011		00
											PSNGR CAR		01	DRVR	NONE	17 F	OR-Y		000	000	000	000	00
																							OR<25
00018	N N N N N	01/05/2009	16	HAWORTH AVE	INTER	3-LEG	N	CLD	ANGL-OTH	01	NONE 0	STRGHT										03	
STATE	MO			SPRINGBROOK RD	CN		STOP SIGN	N	DRY	ANGL	PRVTE	N -S									000	00	
	11A				01	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	54 M	OR-Y		021	000	000	03	
											02 NONE	STRGHT											OR<25
											PRVTE	W -E									000	00	
											PSNGR CAR		01	DRVR	NONE	59 M	OR-Y		000	000	000	000	00
																							OR<25
00910	N N N	11/21/2009	16	HAWORTH AVE	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE 0	STRGHT										02	
NONE	SA			SPRINGBROOK RD	CN		STOP SIGN	N	DRY	TURN	PRVTE	W -E									000	00	
	5P				04	0		N	DUSK	PDO	PSNGR CAR		01	DRVR	NONE	58 F	OR-Y		028	000	000	02	
																							OR<25
											02 NONE	TURN-L											OR<25

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CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 106

SER#	INVEST	D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	TRLR QTY	MOVE	A S	PRTC	INJ	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE
E A U C O DATE	E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	TO	P#	TYPE	SVRTY	E	X	RES	LOC				
		FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE												
										02	NONE	0	STOP										
											PRVTE	NE-SW										011	00
											PSNGR	CAR	02	PSNG	NO<5	02	F			000	000		00
										02	NONE	0	STOP										
											PRVTE	NE-SW										011	00
											PSNGR	CAR	03	PSNG	NO<5	01	M			000	000		00
00732	N N N	09/11/2011	14	PACIFIC HY 99W	INTER	CROSS	N		N	CLR	S-STRGHT	01	NONE	0	STRGHT								13
NONE		SU		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	SS-O	PRVTE	SW-NE										000	00
		7P			05	0		N	DAY	PDO	PSNGR	CAR	01	DRVR	NONE	00	F	OR-Y		045	000		13
										02	NONE	0	STRGHT										
											PRVTE	SW-NE										000	00
											PSNGR	CAR	01	DRVR	NONE	31	F	OR-Y		000	000		00
01087	N N N	12/23/2011	14	PACIFIC HY 99W	INTER	CROSS	N		N	CLR	S-1STOP	01	NONE	0	BACK								10
NONE		FR		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	REAR	UNKN	SW-NE										000	00
		5P			06	0		N	DLIT	PDO	PSNGR	CAR	01	DRVR	NONE	00	F	OR-Y		011	000		10
										02	NONE	0	STOP										
											PRVTE	NE-SW										011	00
											PSNGR	CAR	01	DRVR	NONE	25	F	OR-Y		000	000		00
00690	N N N	08/11/2012	14	PACIFIC HY 99W	INTER	CROSS	N		N	CLR	S-1TURN	01	NONE	0	STRGHT								13
NO RPT		SA		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	TURN	PRVTE	NE-SW										000	00
		11A			06	0		N	DAY	PDO	PSNGR	CAR	01	DRVR	NONE	24	F	OR-Y		045	000		13
										02	NONE	0	TURN-L										
											PRVTE	NE-S										000	00
											PSNGR	CAR	01	DRVR	NONE	74	M	OR-Y		000	000		00
00851	N N N	09/28/2012	14	PACIFIC HY 99W	INTER	CROSS	N		N	CLR	S-1STOP	01	NONE	0	STRGHT								07
NO RPT		FR		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW										000	00
		2P			06	2		N	DAY	PDO	PSNGR	CAR	01	DRVR	NONE	19	M	OR-Y		026	000		07
										02	NONE	0	STOP										
											PRVTE	NE-SW										011	00
											PSNGR	CAR	01	DRVR	NONE	86	M	OR-Y		000	000		00
01210	N N N	12/27/2012	14	PACIFIC HY 99W	INTER	CROSS	N		N	CLR	S-1STOP	01	NONE	0	STRGHT								27
NO RPT		TH		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW										000	00
		9P			06	2		N	DLIT	PDO	PSNGR	CAR	01	DRVR	NONE	30	F	OR-Y		016,026	038		27
										01	NONE	0	STRGHT										
											PRVTE	NE-SW										000	00
											PSNGR	CAR	02	PSNG	NO<5	04	M			000	000		00
										02	NONE	0	STOP										
											PRVTE	NE-SW										011	00
											PSNGR	CAR	01	DRVR	NONE	47	M	OR-Y		000	000		00

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CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 106

SER#	INVEST	D C S L K	DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	E LICNS	PED	ERROR	ACT	EVENT	CAUSE
ELGHRDAY	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	TRAF-	RNDDBT	SURF	COLL	OWNER	FROM	TO	P#	TYPE	SVRTY	E X RES	LOC			
			FR	0	SPRINGBROOK RD	S		TRF SIGNAL	N	DRY	REAR	PRVTE	S -N					000		00
			3P			06	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	32 F	026	000	07
												02	NONE	0	STOP					
												PRVTE	S -N						011	00
												PSNGR CAR		01	DRVR	NONE	31 F	000	000	00
												02	NONE	0	STOP					
												PRVTE	S -N						011	00
												PSNGR CAR		02	PSNG	NO<5	02 F	000	000	00
00604	N N N		07/17/2012	16	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	S-1STOP	01	NONE	0	STRGHT					07
NONE			TU	0	SPRINGBROOK RD	S		TRF SIGNAL	N	WET	REAR	PRVTE	S -N						000	00
			11A			06	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	00 M	026	000	07
												02	NONE	0	STOP					
												PRVTE	S -N						011	00
												PSNGR CAR		01	DRVR	NONE	62 F	000	000	00
00066	N N N N N		01/20/2009	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT					32,07
CITY			TU		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000	00
			10A			06	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	32 M	052,026	000	32,07
												02	NONE	0	STOP					
												PRVTE	SW-NE						011	00
												PSNGR CAR		01	DRVR	NONE	43 M	000	000	00
00548	N N N		07/01/2009	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT					07
NO RPT			WE		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000	00
			11A			06	0		N	DAY	INJ	PSNGR CAR		01	DRVR	NONE	22 M	026	000	07
												02	NONE	0	STOP					
												PRVTE	SW-NE						011	00
												PSNGR CAR		01	DRVR	INJC	28 M	000	000	00
												02	NONE	0	STOP					
												PRVTE	SW-NE						011	00
												PSNGR CAR		02	PSNG	INJC	22 M	000	000	00
												02	NONE	0	STOP					
												PRVTE	SW-NE						011	00
												PSNGR CAR		03	PSNG	NO<5	02 F	000	000	00
00743	N N N		09/25/2009	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT					07
CITY			FR		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000	00
			1P			06	0		N	DAY	INJ	PSNGR CAR		01	DRVR	INJA	29 F	026	000	07
												02	NONE	0	STOP					
												PRVTE	SW-NE						011	00
												PSNGR CAR		01	DRVR	INJC	42 M	000	000	00

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CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 106

SER#	INVEST	D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	PED	ERROR	ACT	EVENT	CAUSE			
E A U C O DATE	E L G H R DAY	FROM	DIST	FIRST STREET	DIRECT	(MEDIAN)	TRAF-	RNDBT	SURF	COLL	TRLR QTY	FROM	INJ	G E LICNS							
9P				SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E X RES	LOC					
					06	0		N	DARK	PDO	PSNGR CAR		01 DRVR	NONE	17 F	OR-Y		045	000	13	
											02 NONE 0	STOP									
											PRVTE	SW-NE							012	00	
											PSNGR CAR		01 DRVR	NONE	57 M	OR-Y		000	000	00	
00818	N N N	09/21/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT							124	07	
NONE		FR		SPRINGBROOK RD	SW		TRF SIGNAL	N	WET	REAR	PRVTE	SW-NE							000	124	00
		3P			06	2		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	19 M	OR-Y		026	000	07	
											02 NONE 0	STOP									
											PRVTE	SW-NE							011	00	
											PSNGR CAR		01 DRVR	NONE	26 F	OR-Y		000	000	00	
00868	N N N N N	10/02/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT							000	00	
CITY		TU		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE							000	00	
		5P			06	2		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	44 M	OR-Y		043	000	07	
											02 NONE 0	STOP									
											PRVTE	SW-NE							011	00	
											PSNGR CAR		01 DRVR	INJC	41 M	OR-Y		000	000	00	
											02 NONE 0	STOP							011	00	
											PRVTE	SW-NE							000	000	00
											PSNGR CAR		02 PSNG	INJC	39 F			000	000	00	
00873	N N N	10/03/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT							000	00	
NONE		WE		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE							000	00	
		12P			06	2		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	20 F	OTH-Y		026	000	07	
											02 NONE 0	STOP									
											PRVTE	SW-NE							011	00	
											PSNGR CAR		01 DRVR	INJC	41 M	OR-Y		000	000	00	
											02 NONE 0	STOP							011	00	
											PRVTE	SW-NE							000	000	00
											PSNGR CAR		01 DRVR	INJC	41 M	OR-Y		000	000	00	
											02 NONE 0	STOP							011	00	
											PRVTE	SW-NE							000	000	00
											PSNGR CAR		02 PSNG	INJC	65 F			000	000	00	
00987	N N N	11/01/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	ANGL-OTH	01 NONE 0	TURN-L							000	00	
NO RPT		TH		SPRINGBROOK RD	SW		L-GRN-SIG	N	WET	TURN	PRVTE	S -SW							000	00	
		7A			05	0		N	DAWN	PDO	SCHL BUS		01 DRVR	NONE	58 M	OR-Y		007,028	000	08,02	
											02 NONE 0	TURN-L									

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OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
URBAN NON-SYSTEM CRASH LISTING

CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 106

SER#	INVEST	E A U C O DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	PRTC	INJ	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE
E L G H R DAY	D C S L K TIME	DIST	FROM	FIRST STREET	DIRECT	(MEDIAN)	TRAF-	RNDBT	SURF	COLL	TRLR QTY	FROM									
				SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO		P# TYPE	SVRTY	E X RES	LOC				
											02 NONE 0	STOP									
											PRVTE	SW-NE									011 00
											PSNGR CAR			02 PSNG	INJC	59 M		000	000		00
00038	Y N N	01/12/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	S-STRGHT	01 NONE 0	STRGHT									01
NONE		WE		SPRINGBROOK RD	W		TRF SIGNAL	N	WET	REAR	PRVTE	W -E							000		00
		5P			06	0		N	DARK	INJ	PSNGR CAR			01 DRVR	NONE	58 F	OR-Y	042	000		01
											02 NONE 0	STRGHT									
											PRVTE	W -E									000 00
											PSNGR CAR			01 DRVR	INJC	47 F	OR-Y	000	000		00
00168	N N N	03/02/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	UNK	S-1STOP	01 NONE 0	STRGHT									07,27
NONE		WE		SPRINGBROOK RD	W		UNKNOWN	N	UNK	REAR	PRVTE	W -E							000		00
		4P			06	0		N	DAY	PDO	PSNGR CAR			01 DRVR	NONE	34 M	OR-Y	026	000		07,27
											02 NONE 0	STOP									
											PRVTE	W -E									011 00
											PSNGR CAR			01 DRVR	NONE	35 M	OR-Y	000	000		00
00493	Y N N	07/01/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT									01
CITY		FR		SPRINGBROOK RD	W		TRF SIGNAL	N	DRY	REAR	PRVTE	E -W							000		00
		12P			06	0		N	DAY	PDO	PSNGR CAR			01 DRVR	NONE	00 Unk	UNK	026	000		01
											02 NONE 0	STOP									
											PRVTE	E -W									011 00
											PSNGR CAR			01 DRVR	NONE	37 F	OR-Y	000	000		00
											02 NONE 0	STOP									
											PRVTE	E -W									011 00
											PSNGR CAR			02 PSNG	NO<5	01 M		000	000		00
00686	N N N	08/24/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT									07
NONE		WE		SPRINGBROOK RD	W		UNKNOWN	N	DRY	REAR	PRVTE	W -E							000		00
		4P			06	0		N	DAY	PDO	PSNGR CAR			01 DRVR	NONE	52 F	OR-Y	026	000		07
											02 NONE 0	STOP									
											PRVTE	W -E									011 00
											PSNGR CAR			01 DRVR	NONE	34 F	OR-Y	000	000		00
00248	N N N	03/19/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	S-1STOP	01 NONE 0	STRGHT									013 27
NO RPT		MO		SPRINGBROOK RD	W		TRF SIGNAL	N	WET	REAR	PRVTE	W -E							000		00
		7A			06	0		N	DAY	PDO	PSNGR CAR			01 DRVR	NONE	29 M	OR-Y	026	000		07
											02 NONE 0	STOP									
											PRVTE	W -E									011 013 00
											PSNGR CAR			01 DRVR	NONE	73 M	OR-Y	000	000		00
											03 NONE 0	STOP									
											PRVTE	W -E									011 00
											PSNGR CAR			01 DRVR	NONE	61 M	OR-Y	000	000		00

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245/690

CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 106

SER#	INVEST	S D P R S W E A U C O DATE E L G H R DAY D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE		OFFRD	WTHR	CRASH	SPCL USE		MOVE	A S					ACT	EVENT	CAUSE			
						(MEDIAN)	INT-REL				TRLR QTY	OWNER		FROM	PRTC	INJ	G E LICNS	PED				ERROR		
			DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE
			FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY														
00138	NONE	N N N 02/21/2013 TH 3P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER W	CROSS	N	N	CLR	S-1STOP REAR INJ	01	NONE	0	STRGHT W -E	01	DRVR	NONE	00	M	UNK	026	000	000	07
						2	YIELD	N	DRY		02	NONE	0	STOP W -E	01	DRVR	INJC	32	M	OR-Y	000	011	000	00
00146	NONE	N N N 02/19/2009 TH 6P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER CN	CROSS	N	N	CLR	ANGL-OTH TURN PDO	01	NONE	0	TURN-R SW-S	01	DRVR	NONE	50	F	OR-Y	020	000	000	04
						2	TRF SIGNAL	N	DRY		02	NONE	0	STRGHT N -S	01	DRVR	NONE	46	F	OR-Y	000	000	000	00
00430	CITY	N N N N N 06/08/2009 MO 5P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER CN	CROSS	N	N	CLR	ANGL-OTH ANGL INJ	01	NONE	0	TURN-L N -NE	01	DRVR	NONE	56	F	OR-Y	052,020	000	000	32,04
						0	TRF SIGNAL	N	DRY		02	NONE	0	STRGHT SW-NE	01	DRVR	INJB	30	F	OR-Y	000	000	000	00
01182	NO RPT	N N N 12/03/2010 FR 2P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER CN	CROSS	N	N	CLR	S-OTHER TURN PDO	01	NONE	0	TURN-L N -E	01	DRVR	NONE	64	M	OR-Y	080	000	000	05
						2	TRF SIGNAL	N	DRY		02	NONE	0	TURN-L N -E	01	DRVR	NONE	44	F	OR-Y	000	000	000	00
00895	NO RPT	N N N 10/19/2010 TU 8A	14	PACIFIC HY 99W SPRINGBROOK RD	INTER CN	CROSS	N	N	CLR	O-1TURN TURN INJ	01	NONE	0	TURN-L N -E	01	DRVR	NONE	42	F	OR-Y	020	000	000	04
						0	TRF SIGNAL	N	DRY		02	NONE	0	STRGHT S -N	01	DRVR	INJC	34	F	OR-Y	000	000	000	00
00400	NO RPT	N N N 05/28/2011 SA 4P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER CN	CROSS	N	N	CLR	S-1STOP REAR PDO	01	NONE	0	STRGHT W -E	01	DRVR	NONE	68	M	OR-Y	026	000	000	07
						2	TRF SIGNAL	N	DRY		02	NONE	0	STOP W -E	01	DRVR	NONE	33	F	OR-Y	000	011	000	00
00272		N N N N N 03/27/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT										03

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246/690

CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD and Intersectional Crashes at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 106

SER#	INVEST	D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	TRLR QTY	MOVE	A S	PRTC	INJ	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE	
E A U C O DATE	E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	TO	P#	TYPE	SVRTY	E X RES	LOC							
		FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE													
												01 NONE 0	STRGHT											
												PRVTE	N -S								000	000	00	
												PSNGR CAR			02 PSNG	INJC	63 M				000	000	00	
												02 NONE 0	STRGHT											
												PRVTE	E -W									015	00	
												PSNGR CAR			01 DRVR	NONE	44 F	OR-Y			000	000	00	
00894	N N N N N	10/18/2010	16	SPRINGBROOK RD	INTER	CROSS	N	N	CLR	ANGL-OTH	01 NONE 2	STRGHT											03	
CITY		MO	0	2ND ST	CN		STOP SIGN	N	DRY	TURN	PRVTE	S -N											000	00
		12P			04	0		N	DAY	PDO	SEMI TOW				01 DRVR	NONE	46 M	OR-Y			021	000	03	
												02 NONE 0	TURN-L											
												PRVTE	W -N										000	00
												PSNGR CAR			01 DRVR	NONE	30 M	OR-Y			000	000	00	
00814	N N N	09/21/2012	16	SPRINGBROOK RD	INTER	CROSS	N	N	RAIN	ANGL-OTH	01 NONE 0	STRGHT											03	
NO RPT		FR	0	2ND ST	CN		STOP SIGN	N	WET	TURN	PRVTE	N -S											015	00
		9A			03	0		N	DAY	INJ	PSNGR CAR				01 DRVR	INJB	22 M	OR-Y			000	000	00	
												02 NONE 0	TURN-L											
												PRVTE	W -N										000	00
												PSNGR CAR			01 DRVR	NONE	30 M	OR-Y			021	000	03	
01190	N N N N N	12/20/2012	16	SPRINGBROOK RD	INTER	CROSS	N	N	RAIN	O-1TURN	01 NONE 0	STRGHT											03,27	
CITY		TH	0	2ND ST	CN		STOP SIGN	N	WET	TURN	PRVTE	N -S											000	00
		6A			01	0		N	DARK	INJ	PSNGR CAR				01 DRVR	INJB	24 F	OR-Y			021,016	038	03,27	
												02 NONE 0	TURN-L											
												PRVTE	S -W										015	00
												PSNGR CAR			01 DRVR	NONE	56 F	OR-Y			000	000	00	

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10/30/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

BRUTSCHER ST and Intersectional Crashes at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL CRASHES	NON- FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER- SECTION	INTER- SECTION RELATED	OFF- ROAD
YEAR: 2013														
NON-COLLISION	0	1	0	1	0	1	0	1	0	0	1	1	0	0
REAR-END	0	2	0	2	0	2	0	1	1	2	0	2	0	0
TURNING MOVEMENTS	0	0	2	2	0	0	0	2	0	2	0	2	0	0
YEAR 2013 TOTAL	0	3	2	5	0	3	0	4	1	4	1	5	0	0
YEAR: 2012														
REAR-END	0	4	1	5	0	5	0	4	1	5	0	4	0	0
TURNING MOVEMENTS	0	0	1	1	0	0	0	1	0	0	1	1	0	0
YEAR 2012 TOTAL	0	4	2	6	0	5	0	5	1	5	1	5	0	0
YEAR: 2011														
ANGLE	0	0	1	1	0	0	0	1	0	0	1	1	0	0
REAR-END	0	4	1	5	0	6	0	3	2	3	2	3	0	0
TURNING MOVEMENTS	0	0	1	1	0	0	0	1	0	1	0	0	0	0
YEAR 2011 TOTAL	0	4	3	7	0	6	0	5	2	4	3	4	0	0
YEAR: 2010														
PEDESTRIAN	0	1	0	1	0	1	0	1	0	1	0	1	0	0
REAR-END	0	0	1	1	0	0	0	0	1	1	0	0	0	0
TURNING MOVEMENTS	0	1	1	2	0	1	0	2	0	2	0	0	0	0
YEAR 2010 TOTAL	0	2	2	4	0	2	0	3	1	4	0	1	0	0
YEAR: 2009														
ANGLE	0	1	0	1	0	1	0	1	0	0	1	1	0	0

10/30/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

BRUTSCHER ST and Intersectional Crashes at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL	NON-	PROPERTY	TOTAL	PEOPLE	PEOPLE	TRUCKS	DRY	WET	DAY	DARK	INTER-	INTER-	OFF-
	CRASHES	FATAL	DAMAGE	CRASHES	KILLED	INJURED		SURF	SURF			SECTION	SECTION	
REAR-END	0	1	2	3	0	1	0	2	1	3	0	3	0	0
YEAR 2009 TOTAL	0	2	2	4	0	2	0	3	1	3	1	4	0	0
FINAL TOTAL	0	15	11	26	0	18	0	20	6	20	6	19	0	0

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CITY OF NEWBERG, YAMHILL COUNTY

BRUTSCHER ST and Intersectional Crashes at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 26

SER#	INVEST	S D P R S W E A U C O DATE E L G H R DAY D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN) INT-REL LEGS TRAF- (#LANES) CONTL	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY MOVE	A S G E LICNS E X RES	PED	ERROR	ACT	EVENT	CAUSE
			DIST	FIRST STREET	DIRECT		RNDBT	SURF	COLL	OWNER	FROM					
			FROM	SECOND STREET	LOCTN		DRVWY	LIGHT	SVRTY	V# TYPE	TO					
00152	NONE	N N N 02/26/2011 SA 6P	17	LITTLE OAK BRUTSCHER ST	INTER CN 03	3-LEG N 0	N	CLR	ANGL-OTH ANGL PDO	01 NONE 0 PRVTE PSNGR CAR	STRGHT W -E		028	000	000	02
										02 NONE PRVTE PSNGR CAR	STRGHT N -S			000	000	00
																02
00913	NONE	Y N N 10/16/2013 WE 7A	17	BRUTSCHER ST HAYES ST	INTER CN 01	CROSS N 0	N	CLR	OVERTURN NCOL INJ	01 NONE 0 PRVTE MTRCYCLE	TURN-L E -S		047	000	124	01
																00
																01
00594	CITY	N N N N N 08/07/2009 FR 8A	14	BRUTSCHER ST PACIFIC HY 99W	INTER UN 06	CROSS N 0	N	CLD	S-1STOP REAR INJ	01 NONE 0 PRVTE PSNGR CAR	STRGHT N -S		026	000	013	07
										02 NONE 0 PRVTE PSNGR CAR	STOP N -S			011	013	00
										03 NONE 0 PRVTE PSNGR CAR	STOP N -S			011		00
00920	NONE	N N N 11/25/2009 WE 3P	14	BRUTSCHER ST PACIFIC HY 99W	INTER NE 06	3-LEG N 0	N	CLR	S-1STOP REAR PDO	01 NONE 0 PRVTE PSNGR CAR	STRGHT NE-SW		026	000	000	07
										02 NONE 0 PRVTE PSNGR CAR	STOP NE-SW			011		00
																00
00415	NO RPT	N N N 05/19/2012 SA 10A	14	BRUTSCHER ST PACIFIC HY 99W	INTER NE 06	3-LEG N 0	N	CLR	S-1STOP REAR INJ	01 NONE 0 PRVTE PSNGR CAR	STRGHT NE-SW		026	000	013	07
										02 NONE 0 PRVTE PSNGR CAR	STOP NE-SW			011	013	00
										03 NONE 0 PRVTE PSNGR CAR	STOP NE-SW			011		00
00518	NO RPT	N N N 06/22/2012 FR 4P	14	BRUTSCHER ST PACIFIC HY 99W	INTER NE 06	3-LEG N 0	N	RAIN	S-1STOP REAR INJ	01 NONE 0 PRVTE PSNGR CAR	STRGHT NE-SW		026	000	000	27
										02 NONE 0 PRVTE PSNGR CAR	STOP NE-SW			011		00
																00

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OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
URBAN NON-SYSTEM CRASH LISTING

CITY OF NEWBERG, YAMHILL COUNTY

BRUTSCHER ST and Intersectional Crashes at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 26

SER#	INVEST	S D P R S W E A U C O DATE E L G H R DAY D C S L K TIME	CLASS	CITY STREET DIST FROM	RD CHAR FIRST STREET SECOND STREET LOCTN	INT-TYPE (MEDIAN) INT-REL LEGS TRAF- CONTL (#LANES)	OFFRD	WTHR	CRASH COLL SVRTY	SPCL USE TRLR QTY OWNER V# TYPE	MOVE FROM TO	PRTC	INJ SVRTY	A S E X RES		PED LOC	ERROR	ACT	EVENT	CAUSE	
																					OR<25
00907 NONE		N N N 10/04/2012 TH 5P	14	BRUTSCHER ST PACIFIC HY 99W	INTER NE 06	CROSS N 0	N	CLR DRY DAY	S-1STOP REAR PDO	01 NONE PRVTE PSNGR CAR	0	STRGHT NE-SW	01 DRVR	NONE	26 M	OR-Y OR<25	026	000 000	000 000	07 07	
										02 NONE PRVTE PSNGR CAR	0	STOP NE-SW	01 DRVR	NONE	40 M	OR-Y OR<25	000	011 000	000 000	00 00	
00829 NONE		N N N 09/23/2013 MO 2P	14	BRUTSCHER ST PACIFIC HY 99W	INTER NE 06	3-LEG N 0	N	RAIN WET DAY	S-1STOP REAR INJ	01 NONE PRVTE PSNGR CAR	0	STRGHT NE-SW	01 DRVR	NONE	40 F	OR-Y OR<25	026	000 000	000 000	07 07	
										02 NONE PRVTE PSNGR CAR	0	STOP NE-SW	01 DRVR	INJC	71 M	OR-Y OR<25	000	011 000	000 000	00 00	
00661 NO RPT		N N N 08/18/2011 TH 5P	14	BRUTSCHER ST PACIFIC HY 99W	INTER E 06	3-LEG N 0	N	CLR DRY DAY	S-1STOP REAR INJ	01 NONE PRVTE PSNGR CAR	0	STRGHT E -W	01 DRVR	INJB	35 F	OR-Y OR<25	052,026	000 000	000 000	013 32 32	
										02 NONE PRVTE PSNGR CAR	0	STOP E -W	01 DRVR	INJC	43 M	OR-Y OR<25	000	011 000	013 000	00 00	
										03 NONE PRVTE PSNGR CAR	0	STOP E -W	01 DRVR	NONE	59 M	OR-Y OR<25	000	011 000	000 000	00 00	
01136 CITY		N N N N N 12/21/2010 TU 9A	14	BRUTSCHER ST PACIFIC HY 99W	INTER SW 06	3-LEG N 0	N	CLD DRY DAY	PED PED INJ	01 NONE PRVTE PSNGR CAR	0	TURN-L SE-SW	01 DRVR	NONE	33 M	OTH-Y N-RES	029	000 000	000 000	02 02	
																					00
																					00
00469 CITY		N N N N N 06/11/2012 MO 10A	14	BRUTSCHER ST PACIFIC HY 99W	INTER SW 06	3-LEG N 0	N	CLR DRY DAY	S-1STOP REAR INJ	01 NONE PRVTE PSNGR CAR	0	STRGHT SW-NE	01 DRVR	NONE	22 F	OR-Y OR<25	026	000 000	000 000	27 07	
										02 NONE PRVTE PSNGR CAR	0	STOP SW-NE	01 DRVR	INJB	46 F	OR-Y OR<25	000	011 000	000 000	00 00	
										02 NONE PRVTE PSNGR CAR	0	STOP SW-NE	02 PSNG	INJB	25 F		000	011 000	000 000	00 00	
01018 CITY		N N N 11/09/2012 FR	14	BRUTSCHER ST PACIFIC HY 99W	INTER SW	CROSS N	N	CLD DRY	ANGL-STP TURN	01 NONE PRVTE	0	TURN-L SE-SW							092 000	32, 27 00	

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CITY OF NEWBERG, YAMHILL COUNTY

BRUTSCHER ST and Intersectional Crashes at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 26

INVEST	D C S L K TIME	CLASS	CITY STREET	RD CHAR	(#LANES)	INT-TYPE	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR QTY	MOVE	PRTC	INJ	G	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE	
SER#	E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	TO	P# TYPE	SVRTY	E	X	RES	LOC						
	5P			05	0		N	DUSK	PDO	PSNGR	CAR	01	DRVR	NONE	44	M	OR-Y	OR<25	052,016	038		32,27		
										02	NONE	0	STOP											
										PRVTE	NE-SW											011	092	00
										PSNGR	CAR	01	DRVR	NONE	71	M	OR-Y	OR<25	000	000		00	00	
00585	N N N N N 07/09/2013	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	S-1STOP	01	NONE	0	STRGHT										07	
CITY	TU		PACIFIC HY 99W	SW		TRF	SIGNAL	N	DRY	REAR	PRVTE	SW-NE										000	00	
	6A			06	0			N	DAY	INJ	PSNGR	CAR	01	DRVR	NONE	50	M	OR-Y	OR<25	026	000		07	
										02	NONE	0	STOP											
										PRVTE	SW-NE											011	00	
										PSNGR	CAR	01	DRVR	INJC	36	M	OR-Y	OR<25	000	000		00	00	
00800	N N N 10/30/2009	14	BRUTSCHER ST	INTER	CROSS	N	N	CLD	S-1STOP	01	NONE	0	STRGHT										07,27	
NONE	FR		PACIFIC HY 99W	W		TRF	SIGNAL	N	SNO	REAR	PRVTE	W -E										000	00	
	9A			06	0			N	DAY	PDO	PSNGR	CAR	01	DRVR	NONE	43	M	OR-Y	OR<25	026	000		07,27	
										01	NONE	0	STRGHT											
										PRVTE	W -E											000	00	
										PSNGR	CAR	02	PSNG	NO<5	03	F						000	00	
										02	NONE	0	STOP											
										PRVTE	W -E											011	00	
										PSNGR	CAR	01	DRVR	NONE	00	M	OR-Y	OR<25	000	000		00	00	
00045	N N N N N 01/14/2011	14	BRUTSCHER ST	INTER	3-LEG	N	N	RAIN	S-1STOP	01	NONE	0	STRGHT										07	
CITY	FR		PACIFIC HY 99W	W		TRF	SIGNAL	N	WET	REAR	PRVTE	W -E										000	00	
	5P			06	0			N	DARK	INJ	PSNGR	CAR	01	DRVR	NONE	23	M	OR-Y	OR<25	026	000		07	
										02	NONE	0	STOP											
										PRVTE	W -E											011	00	
										PSNGR	CAR	01	DRVR	INJC	22	F	OR-Y	OR<25	000	000		00	00	
01005	N N N 11/07/2011	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	S-1STOP	01	NONE	0	STRGHT										07	
NONE	MO		PACIFIC HY 99W	W		TRF	SIGNAL	N	DRY	REAR	PRVTE	W -E										000	00	
	5P			06	0			N	DLIT	INJ	PSNGR	CAR	01	DRVR	NONE	42	F	OR-Y	OR>25	026	000		07	
										02	NONE	0	STOP											
										PRVTE	W -E											011	00	
										PSNGR	CAR	01	DRVR	INJC	31	M	OR-Y	OR>25	000	000		00	00	
00063	N N N N N 01/18/2009	14	BRUTSCHER ST	INTER	CROSS	N	N	CLR	ANGL-OTH	01	NONE	0	STRGHT										04	
CITY	SU		PACIFIC HY 99W	CN		TRF	SIGNAL	N	DRY	ANGL	PRVTE	W -E										000	00	
	11P			04	0			N	DARK	INJ	PSNGR	CAR	01	DRVR	NONE	18	F	OR-Y	OR<25	020	000		04	
										02	NONE	0	TURN-L											
										PRVTE	S -W											000	00	
										PSNGR	CAR	01	DRVR	INJB	16	M	OR-Y	OR<25	000	000		00	00	
00375	N N N 05/05/2013	14	BRUTSCHER ST	INTER	CROSS	N	N	CLR	ANGL-OTH	01	NONE	0	U-TURN										08	

CITY OF NEWBERG, YAMHILL COUNTY

BRUTSCHER ST and Intersectional Crashes at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 26

SER#	INVEST	D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE		OFFRD	WTHR	CRASH	SPCL USE		MOVE	PRTC	INJ	A S	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE
						(MEDIAN)	INT-REL				TRLR QTY	TYPE										
NO RPT	E L G H R DAY	FROM	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	TO	#	TYPE	SVRTY	E X RES	LOC				
NO RPT		SU 12P		PACIFIC HY 99W	CN	0	TRF SIGNAL	N	DRY	TURN	PRVTE	S -S								000		00
								N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	63 M	OR-Y OR<25	008	000		08
											02 NONE 0 PRVTE PSNGR CAR	STRGHT E -W								000		00
														01	DRVR	NONE	46 F	OR-Y OR<25	000	000		00
00740	N N N	08/23/2013	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	O-1TURN	01 NONE 0	STRGHT								002	04, 27	
NO RPT		FR 1P		PACIFIC HY 99W	CN	0	TRF SIGNAL	N	DRY	TURN	PRVTE	W -E								000	00	
								N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	31 F	OR-Y OR<25	020	000	002	04, 27
											01 NONE 0 PRVTE PSNGR CAR	STRGHT W -E								000	00	
														02	PSNG	NO<5	01 M		000	000		00
											02 NONE 0 PRVTE PSNGR CAR	TURN-L S -W								000	00	
														01	DRVR	NONE	23 F	OR-Y OR<25	000	000		00
00153	N N N	02/12/2010	17	BRUTSCHER ST	ALLEY	(NONE)	N	N	CLR	ANGL-OTH	01 NONE 0	TURN-L									02	
NONE		FR 4P	150	PACIFIC HY 99W	SE	(NONE)	UNKNOWN	N	DRY	TURN	PRVTE	SW-NW								018	00	
						(02)		N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	69 F	OR-Y OR<25	028	000		02
											02 NONE 0 PRVTE PSNGR CAR	TURN-L SE-SW								019	00	
														01	DRVR	NONE	59 M	OR-Y OR<25	000	000		00
01050	N N N	11/28/2010	19	BRUTSCHER ST	ALLEY	(NONE)	N	N	CLR	ANGL-OTH	01 NONE 0	TURN-L									02	
NO RPT		SU 12P	200	PACIFIC HY 99W	S	(NONE)	STOP SIGN	N	DRY	TURN	PRVTE	SW-NW								018	00	
						(02)		N	DAY	INJ	PSNGR CAR			01	DRVR	NONE	21 M	OR-Y OR<25	028	000		02
											02 NONE 0 PRVTE PSNGR CAR	STRGHT NW-SE								000	00	
														01	DRVR	INJC	62 M	OR-Y OR<25	000	000		00
00010	N N N	01/03/2011	17	BRUTSCHER ST	ALLEY	(NONE)	N	N	CLR	ANGL-OTH	01 NONE 0	TURN-R									02	
NONE		MO 1P	200	PACIFIC HY 99W	S	(NONE)	UNKNOWN	N	DRY	TURN	PRVTE	E -N								000	00	
						(02)		N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	00 M	UNK UNK	028	000		02
											02 NONE 0 PRVTE PSNGR CAR	STRGHT S -N								000	00	
														01	DRVR	NONE	47 F	OR-Y OR<25	000	000		00
00624	N N N	07/21/2012	14	BRUTSCHER ST	STRGHT	(NONE)	N	N	CLR	S-1STOP	01 NONE 0	STRGHT									07	
NONE		SA 3P		PACIFIC HY 99W	E	(NONE)	UNKNOWN	N	DRY	REAR	PRVTE	W -E								000	00	
						(04)		Y	DAY	INJ	PSNGR CAR			01	DRVR	NONE	33 F	OR-Y OR<25	026	000		07
											02 NONE 0 PRVTE PSNGR CAR	STOP W -E								011	00	
														01	DRVR	INJC	27 M	OR-Y OR<25	000	000		00

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255/690

OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
URBAN NON-SYSTEM CRASH LISTING

CITY OF NEWBERG, YAMHILL COUNTY

BRUTSCHER ST and Intersectional Crashes at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 26

SER#	E A U C O DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	PRTC	INJ	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	E X RES	P# TYPE	SVRTY	E X RES	LOC	ERROR	ACT	EVENT	CAUSE
00538	N N N 07/02/2010	14	BRUTSCHER ST	STRGHT	N	N	N	CLD	S-1STOP	01 NONE 0	STRGHT								013	07
NONE	FR		PACIFIC HY 99W	SW	(RSDMD)	UNKNOWN	N	WET	REAR	PRVTE	NE-SW							000		00
	6P			08			N	DAY	PDO	PSNGR CAR			01 DRVR	NONE	17 M	OR-Y	026	000		07
					(04)											OR<25				
										02 NONE 0	STOP								011	013
										PRVTE	NE-SW							000	000	00
										PSNGR CAR			01 DRVR	NONE	00 M	OR-Y				00
																OR<25				
										03 NONE 0	STOP								011	
										PRVTE	NE-SW							000	000	00
										PSNGR CAR			01 DRVR	NONE	00 M	OR-Y	000			00
																OR<25				
00321	N N N 04/30/2011	14	BRUTSCHER ST	STRGHT	N	N	N	CLR	S-1STOP	01 NONE 0	STRGHT									07
NO RPT	SA		PACIFIC HY 99W	SW	(RSDMD)	UNKNOWN	N	DRY	REAR	PRVTE	SW-NE							000		00
	11A			08			N	DAY	PDO	PSNGR CAR			01 DRVR	NONE	56 M	OR-Y	026	000		07
					(04)											OR<25				
										02 NONE 0	STOP								011	
										PRVTE	SW-NE							000	000	00
										PSNGR CAR			01 DRVR	NONE	37 M	UNK				00
																N-RES				
00437	N N N N N 06/12/2011	14	BRUTSCHER ST	STRGHT	N	N	N	CLD	S-1STOP	01 NONE	STRGHT									07
CITY	SU		PACIFIC HY 99W	W	(RSDMD)	UNKNOWN	N	WET	REAR	PRVTE	W -E							000		00
	3P			08			N	DAY	INJ	PSNGR CAR			01 DRVR	NONE	43 M	OTH-Y	026	000		07
					(04)											N-RES				
										02 NONE 0	STOP								011	
										PRVTE	W -E							000	000	00
										PSNGR CAR			01 DRVR	INJC	47 M	OR-Y				00
																OR<25				
										02 NONE 0	STOP								011	
										PRVTE	W -E							000	000	00
										PSNGR CAR			02 PSNG	INJC	11 F					00

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10/30/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

HAYES ST and Intersectional Crashes at HAYES ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL CRASHES	NON- FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER- SECTION	INTER- SECTION RELATED	OFF- ROAD
YEAR: 2013														
NON-COLLISION	0	1	0	1	0	1	0	1	0	0	1	1	0	0
REAR-END	0	1	0	1	0	1	0	0	1	1	0	1	0	0
YEAR 2013 TOTAL	0	2	0	2	0	2	0	1	1	1	1	2	0	0
YEAR: 2012														
FIXED / OTHER OBJECT	0	0	1	1	0	0	0	0	1	1	0	1	0	1
YEAR 2012 TOTAL	0	0	1	1	0	0	0	0	1	1	0	1	0	1
FINAL TOTAL	0	2	1	3	0	2	0	1	2	2	1	3	0	1

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CITY OF NEWBERG, YAMHILL COUNTY

HAYES ST and Intersectional Crashes at HAYES ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 3

SER#	INVEST	S D	P R S W	E A U C O DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	TRLR QTY	MOVE	A S	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE	
E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E LICNS	PED										
D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC	ERROR	ACT	EVENT	CAUSE				
01080	N N N N N	11/23/2012	19	BURL ST	INTER	CROSS	N	Y	RAIN	FIX OBJ	01	NONE	0	PARKNG							040,088	27		
CITY	FR	0	HAYES ST	E	STOP SIGN	N	WET	FIX	PRVTE	S -N											008	040,088	00	
	10A			06	0	N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	41	M	OR-Y			016,080,081	038		27	
																							OR<25	
00913	Y N N	10/16/2013	17	BRUTSCHER ST	INTER	CROSS	N	N	CLR	OVERTURN	01	NONE	0	TURN-L								124	01	
NONE	WE	0	HAYES ST	CN	CURVE	Y	DRY	NCOL	PRVTE	E -S											000	124	00	
	7A			01	0	N	DAWN	INJ	MTRCYCLE			01	DRVR	INJB	20	M	OR-Y			047	017		01	
																							OR<25	
00830	N N N N N	09/23/2013	16	HAYES ST	INTER	3-LEG	N	N	CLD	S-1STOP	01	NONE	0	STRGHT									07	
CITY	MO	0	SPRINGBROOK RD	N	TRF SIGNAL	N	SNO	REAR	PRVTE	N -S											000		00	
	8A			06	0	N	DAY	INJ	PSNGR CAR			01	DRVR	NONE	48	F	OR-Y			026	000		07	
																							OR<25	
												02	NONE	0	STOP							011		00
																								00
												01	DRVR	INJC	37	F	OR-Y			000	000			00
																								OR<25

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10/23/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL CRASHES	NON- FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER- SECTION	INTER- SECTION RELATED	OFF- ROAD
YEAR: 2013														
REAR-END	0	9	3	12	0	13	0	9	2	11	1	12	0	0
TURNING MOVEMENTS	0	1	0	1	0	2	0	1	0	0	1	1	0	0
YEAR 2013 TOTAL	0	10	3	13	0	15	0	10	2	11	2	13	0	0
YEAR: 2012														
ANGLE	0	1	1	2	0	1	0	0	2	0	2	2	0	0
PEDESTRIAN	0	1	0	1	0	1	0	1	0	1	0	1	0	0
REAR-END	0	5	6	11	0	7	0	7	4	9	2	11	0	0
TURNING MOVEMENTS	0	0	4	4	0	0	0	2	2	2	2	4	0	0
YEAR 2012 TOTAL	0	7	11	18	0	9	0	10	8	12	6	18	0	0
YEAR: 2011														
REAR-END	0	1	14	15	0	1	0	10	3	13	2	15	0	0
SIDESWIPE - OVERTAKING	0	0	1	1	0	0	0	1	0	1	0	1	0	0
TURNING MOVEMENTS	0	0	1	1	0	0	0	1	0	1	0	1	0	0
YEAR 2011 TOTAL	0	1	16	17	0	1	0	12	3	15	2	17	0	0
YEAR: 2010														
REAR-END	0	3	4	7	0	4	0	4	3	5	2	7	0	0
TURNING MOVEMENTS	0	1	1	2	0	1	0	2	0	2	0	2	0	0
YEAR 2010 TOTAL	0	4	5	9	0	5	0	6	3	7	2	9	0	0
YEAR: 2009														
ANGLE	0	1	0	1	0	1	0	1	0	1	0	1	0	0

10/23/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL	NON-	PROPERTY	TOTAL	PEOPLE	PEOPLE	TRUCKS	DRY	WET	DAY	DARK	INTER-	INTER-	OFF-
	CRASHES	FATAL	DAMAGE	CRASHES	KILLED	INJURED		SURF	SURF			SECTION	SECTION	
REAR-END	0	5	5	10	0	7	0	7	2	9	1	10	0	0
TURNING MOVEMENTS	0	1	1	2	0	1	0	2	0	1	1	2	0	0
YEAR 2009 TOTAL	0	7	6	13	0	9	0	10	2	11	2	13	0	0
FINAL TOTAL	0	29	41	70	0	39	0	48	18	56	14	70	0	0

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CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	INVEST	S D P R S W E A U C O DATE E L G H R DAY D C S L K TIME	CLASS	CITY STREET DIST FIRST STREET SECOND STREET	RD CHAR DIRECT LOCTN	INT-TYPE (MEDIAN) LEGS (#LANES)	INT-REL TRAF- CONTL	OFFRD RDNBT DRVWY	WTHR SURF LIGHT	CRASH COLL SVRTY	SPCL USE TRLR QTY MOVE OWNER FROM V# TYPE	A S G E LICNS E X RES LOC	PRTC INJ SVRTY	ERROR	ACT	EVENT	CAUSE
00021	NO RPT	N N N 01/12/2009 MO 8P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER UN 06	CROSS 2	N TRF SIGNAL	N N	CLR DRY DARK	S-STRGHT REAR INJ	01 NONE 0 PRVTE PSNGR CAR	STRGHT NE-SW	01 DRVR INJB	61 M OR-Y OR<25	042	000 000	07 00 07
00029	CITY	N N N N N 01/10/2012 TU 3P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER UN 05	CROSS 0	N TRF SIGNAL	N N	CLR DRY DAY	PED PED INJ	01 NONE 0 PRVTE PSNGR CAR	TURN-R W -S	01 DRVR NONE	71 F OR-Y OR<25	029	026	02 00 02
00319	CITY	N N N N N 04/18/2013 TH 8P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER UN 05	CROSS 0	N TRF SIGNAL	N N	CLR DRY DARK	S-OTHER TURN INJ	01 NONE 0 PRVTE PSNGR CAR	TURN-L N -E	01 DRVR NONE	69 M OR-Y OR<25	080	000	05 00 05
01068	NO RPT	N N N 11/07/2011 MO 7A	16	PACIFIC HY 99W SPRINGBROOK RD	INTER N 06	CROSS 0	N TRF SIGNAL	N N	CLR WET DAY	S-1STOP REAR PDO	01 NONE 0 PUBLIC SCHL BUS	STRGHT N -S	01 DRVR NONE	00 Unk UNK	026	000	07 00 07
00322	NONE	N N N 04/15/2009 WE 5P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER NE 06	CROSS 0	N TRF SIGNAL	N N	CLR DRY DAY	S-1STOP REAR PDO	01 NONE 0 PRVTE PSNGR CAR	STRGHT NE-SW	01 DRVR NONE	00 M OR-Y OR<25	026	000	07 00 07
00489	NO RPT	N N N 06/30/2009 TU 3P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER NE 06	CROSS 0	N TRF SIGNAL	N N	UNK UNK DAY	S-1STOP REAR PDO	01 NONE 0 PRVTE PSNGR CAR	STRGHT NE-SW	01 DRVR NONE	30 F UNK	026	000	07 00 07

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OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
URBAN NON-SYSTEM CRASH LISTING

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	E A U C O DATE	CLASS	CITY STREET	RD CHAR	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	PRTC	INJ	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E X RES	LOC				
										02 NONE 0	STOP								OR<25
										PRVTE	NE-SW						011		00
										PSNGR CAR		01 DRVR	NONE	34 M	OTH-Y	000	000		00
																			N-RES
00578	N N N	07/13/2009	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT						013	07
NO RPT		MO		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW					000		00
		11A			06	0		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	33 F	OR-Y	026	000	07
																			OR<25
										02 NONE 0	STOP								011
										PRVTE	NE-SW						011	013	00
										PSNGR CAR		01 DRVR	NONE	35 F	OR-Y	000	000		00
																			OR<25
										03 NONE 0	STOP								011
										PRVTE	NE-SW						011		00
										PSNGR CAR		01 DRVR	NONE	00 Unk	UNK	000	000		00
																			UNK
00065	N N N	01/05/2010	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	S-1STOP	01 NONE 0	TURN-R							27,07
NONE		TU		SPRINGBROOK RD	NE		YIELD	N	WET	REAR	PRVTE	E -N					000		00
		7P			09	2		N	DARK	PDO	PSNGR CAR		01 DRVR	NONE	55 M	OR-Y	026,016	000	27,07
																			OR<25
										02 NONE 0	STOP								011
										PRVTE	E -N						011		00
										PSNGR CAR		01 DRVR	NONE	28 F	OR-Y	000	000		00
																			OR<25
00870	N N N	10/13/2010	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE	STRGHT							013
NO RPT		WE		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW					000		00
		3P			06	0		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	37 F	OR-Y	026	000	07
																			OR<25
										02 NONE 0	STOP								011
										PRVTE	NE-SW						011	013	00
										PSNGR CAR		01 DRVR	NONE	68 M	OR-Y	000	000		00
																			OR<25
										03 NONE 0	STOP								011
										PRVTE	NE-SW						011		00
										PSNGR CAR		01 DRVR	NONE	67 F	OR-Y	000	000		00
																			OR<25
00516	N N N	07/07/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	UNK	S-1STOP	01 NONE 0	STRGHT							07
NONE		TH		SPRINGBROOK RD	NE		TRF SIGNAL	N	UNK	REAR	PRVTE	NE-SW					000		00
		8A			06	0		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	00 M	OR-Y	026	000	07
																			UNK
																			OR<25
										02 NONE 0	STOP								011
										PRVTE	NE-SW						011		00
										PSNGR CAR		01 DRVR	NONE	60 M	OR-Y	000	000		00
																			OR<25
00692	N N N	08/26/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT							07
NONE		FR		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW					000		00
		3P			06	0		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	39 M	OR-Y	026	000	07
																			OR<25
										02 NONE 0	STOP								011
										PRVTE	NE-SW						011		00

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262/690

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	E A U C O DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	INT-REL INT-REL	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	A S	PRTC	INJ	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE
INVEST	E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM									
	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO		P# TYPE	SVRTY	E X RES	LOC				
										01 PSNGR CAR			01 DRVR	NONE	25 F	OR-Y	000	000		OR<25
										02 NONE 0 PRVTE PSNGR CAR	STOP NE-SW		02 PSNG	NO<5	02 F		000	000		00
										02 NONE 0 PRVTE PSNGR CAR	STOP NE-SW		03 PSNG	NO<5	01 M		000	000		00
00732	N N N	09/11/2011	14	PACIFIC HY 99W	INTER	CROSS	N	CLR	S-STRGHT	01 NONE 0	STRGHT									13
NONE	SU			SPRINGBROOK RD	NE		TRF SIGNAL	DRY	SS-O	PRVTE	SW-NE									00
	7P				05	0		DAY	PDO	PSNGR CAR			01 DRVR	NONE	00 F	OR-Y	045	000		OR<25
										02 NONE 0 PRVTE PSNGR CAR	STRGHT SW-NE		01 DRVR	NONE	31 F	OR-Y	000	000		00
																				00
01087	N N N	12/23/2011	14	PACIFIC HY 99W	INTER	CROSS	N	CLR	S-1STOP	01 NONE 0	BACK									10
NONE	FR			SPRINGBROOK RD	NE		TRF SIGNAL	DRY	REAR	UNKN	SW-NE									00
	5P				06	0		DLIT	PDO	PSNGR CAR			01 DRVR	NONE	00 F	OR-Y	011	000		UNK
										02 NONE 0 PRVTE PSNGR CAR	STOP NE-SW		01 DRVR	NONE	25 F	OR-Y	000	000		00
																				00
00690	N N N	08/11/2012	14	PACIFIC HY 99W	INTER	CROSS	N	CLR	S-1TURN	01 NONE 0	STRGHT									13
NO RPT	SA			SPRINGBROOK RD	NE		TRF SIGNAL	DRY	TURN	PRVTE	NE-SW									00
	11A				06	0		DAY	PDO	PSNGR CAR			01 DRVR	NONE	24 F	OR-Y	045	000		OR<25
										02 NONE 0 PRVTE PSNGR CAR	TURN-L NE-S		01 DRVR	NONE	74 M	OR-Y	000	000		OR>25
																				00
00851	N N N	09/28/2012	14	PACIFIC HY 99W	INTER	CROSS	N	CLR	S-1STOP	01 NONE 0	STRGHT									07
NO RPT	FR			SPRINGBROOK RD	NE		TRF SIGNAL	DRY	REAR	PRVTE	NE-SW									00
	2P				06	2		DAY	PDO	PSNGR CAR			01 DRVR	NONE	19 M	OR-Y	026	000		OR<25
										02 NONE 0 PRVTE PSNGR CAR	STOP NE-SW		01 DRVR	NONE	86 M	OR-Y	000	000		00
																				00
01210	N N N	12/27/2012	14	PACIFIC HY 99W	INTER	CROSS	N	CLR	S-1STOP	01 NONE 0	STRGHT									27
NO RPT	TH			SPRINGBROOK RD	NE		TRF SIGNAL	DRY	REAR	PRVTE	NE-SW									00
	9P				06	2		DLIT	PDO	PSNGR CAR			01 DRVR	NONE	30 F	OR-Y	016,026	038		OR<25
										01 NONE 0 PRVTE PSNGR CAR	STRGHT NE-SW									00
													02 PSNG	NO<5	04 M		000	000		00
										02 NONE 0	STOP									00

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CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	E A U C O DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S	P E LICNS	PED	ERROR	ACT	EVENT	CAUSE
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E X RES	LOC			
										PRVTE	NE-SW				011			00
										PSNGR CAR		01 DRVR	NONE	47 M	OR-Y	000	000	00
																		OR<25
00766	N N N 09/02/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE	0	STRGHT						07
NO RPT	MO		SPRINGBROOK RD	NE		TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW					000		00
	5P			06	1		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	22 F	OR-Y	026	000	07
																		OR<25
										02 NONE	0	STOP						00
										PRVTE	NE-SW							00
										PSNGR CAR		01 DRVR	INJC	52 F	OR-Y	000	000	00
																		OR<25
										02 NONE	0	STOP						00
										PRVTE	NE-SW							00
										PSNGR CAR		02 PSNG	INJC	17 F		000	000	00
00947	N N N 10/26/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE	0	TURN-L						07
NONE	SA		SPRINGBROOK RD	NE		L-GRN-SIG	N	DRY	REAR	PRVTE	NE-S					000		00
	10A			06	0		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	48 M	OR-Y	026	000	07
																		OR>25
										02 NONE	0	STOP						00
										PRVTE	NE-SW							00
										PSNGR CAR		01 DRVR	INJC	24 M	OR-Y	000	000	00
																		OR<25
										02 NONE	0	STOP						00
										PRVTE	NE-SW							00
										PSNGR CAR		02 PSNG	INJC	23 F		000	000	00
00124	Y N N N N 02/19/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLD	ANGL-OTH	01 NONE	0	U-TURN						08,01
CITY	SA		SPRINGBROOK RD	E		TRF SIGNAL	N	DRY	TURN	PRVTE	E -E					000		00
	2P			05	2		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	32 M	OR-Y	008	000	08,01
																		OR<25
										02 NONE	0	TURN-R						00
										PRVTE	S -E							00
										PSNGR CAR		01 DRVR	NONE	17 M	OR-Y	000	000	00
																		OR<25
00711	N N N N N 09/02/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 POLCE		STRGHT						07
CITY	FR		SPRINGBROOK RD	E		TRF SIGNAL	N	DRY	REAR	PRVTE	E -W					000		00
	4P			06	0		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	39 M	OR-Y	026	000	07
																		OR<25
										02 NONE	1	STOP						00
										PRVTE	E -W							00
										PSNGR CAR		01 DRVR	NONE	41 F	OR-Y	000	000	00
																		OR<25
00178	N N N 02/24/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE	0	STRGHT						07
NONE	FR		SPRINGBROOK RD	E		TRF SIGNAL	N	DRY	REAR	PRVTE	E -W					000		00
	9P			06	2		N	DARK	PDO	PSNGR CAR		01 DRVR	NONE	50 M	OR-Y	026	000	07
																		OR<25
										02 NONE	0	STOP						00
										PRVTE	E -W							00
										PSNGR CAR		01 DRVR	NONE	65 F	OR-Y	000	000	00
																		OR<25

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264/690

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	E A U C O DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	A S G E LICNS	PED	ERROR	ACT	EVENT	CAUSE			
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E X RES	LOC	ERROR	ACT	EVENT	CAUSE	
00790	N N N N N 09/30/2011	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								07	
CITY	FR	0	SPRINGBROOK RD	S		TRF SIGNAL	N	DRY	REAR	PRVTE	S -N							000	00	
	3P			06	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	32 F	OR-Y		026	000	07
																OR<25				
										02 NONE 0	STOP								011	00
										PRVTE	S -N								000	000
										PSNGR CAR		01	DRVR	NONE	31 F	OR-Y		000	000	00
																OR<25				
										02 NONE 0	STOP								011	00
										PRVTE	S -N								000	000
										PSNGR CAR		02	PSNG	NO<5	02 F			000	000	00
00604	N N N 07/17/2012	16	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	S-1STOP	01 NONE 0	STRGHT								07	
NONE	TU	0	SPRINGBROOK RD	S		TRF SIGNAL	N	WET	REAR	PRVTE	S -N								000	00
	11A			06	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	00 M	UNK		026	000	07
																UNK				
										02 NONE 0	STOP								011	00
										PRVTE	S -N								000	000
										PSNGR CAR		01	DRVR	NONE	62 F	OR-Y		000	000	00
																OR<25				
00066	N N N N N 01/20/2009	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								32,07	
CITY	TU		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE								000	00
	10A			06	0		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	32 M	OR-Y		052,026	000	32,07
																OR<25				
										02 NONE 0	STOP								011	00
										PRVTE	SW-NE								000	000
										PSNGR CAR		01	DRVR	NONE	43 M	OTH-Y		000	000	00
																N-RES				
00548	N N N 07/01/2009	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								07	
NO RPT	WE		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE								000	00
	11A			06	0		N	DAY	INJ	PSNGR CAR		01	DRVR	NONE	22 M	OR-Y		026	000	07
																OR<25				
										02 NONE 0	STOP								011	00
										PRVTE	SW-NE								000	000
										PSNGR CAR		01	DRVR	INJC	28 M	OR-Y		000	000	00
																OR<25				
										02 NONE 0	STOP								011	00
										PRVTE	SW-NE								000	000
										PSNGR CAR		02	PSNG	INJC	22 M			000	000	00
										02 NONE 0	STOP								011	00
										PRVTE	SW-NE								000	000
										PSNGR CAR		03	PSNG	NO<5	02 F			000	000	00
00743	N N N 09/25/2009	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								07	
CITY	FR		SPRINGBROOK RD	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE								000	00
	1P			06	0		N	DAY	INJ	PSNGR CAR		01	DRVR	INJA	29 F	OR-Y		026	000	07
																OR<25				
										02 NONE 0	STOP								011	00
										PRVTE	SW-NE								000	000

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CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	S D		CLASS	CITY STREET	RD CHAR	INT-TYPE			CRASH	SPCL USE		MOVE	A S											
	P R S W					(MEDIAN)	INT-REL	OFFRD		WTHR	TRLR		QTY	PRTC	INJ	G	E	LICNS	PED					
	E A U C O DATE	E L G H R DAY				DIST	FIRST STREET	DIRECT		LEGS	TRAF-		RNDBT							SURF	COLL	OWNER	FROM	
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE	
00776	N N N N N	10/21/2009	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW	CROSS	N	TRF SIGNAL	N	CLD	S-1STOP	01	NONE	0	STRGHT								00	
CITY	WE				06	0			N	DAY	INJ	PSNGR CAR			01	DRVR	NONE	49	M	OR-Y	051,026	000	00	33,07
	9A								N	DAY	INJ	PSNGR CAR										00		33,07
												02	NONE	0	STOP								011	00
												PSNGR CAR			01	DRVR	INJC	49	F	OR-Y	000	000	00	00
00826	N N N	10/08/2009	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW	CROSS	N	TRF SIGNAL	N	CLR	S-1STOP	01	NONE	0	STRGHT								00	27,07
NONE	TH				06	2			N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	76	F	OR-Y	016,026	000	00	00
	3P								N	DAY	PDO	PSNGR CAR											00	27,07
												02	NONE	0	STOP								011	00
												PSNGR CAR			01	DRVR	NONE	31	M	OR-Y	000	000	00	00
86584	N N N	12/16/2009	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW	CROSS	N	TRF SIGNAL	N	RAIN	S-1STOP	01	NONE	0	STRGHT								00	07
NO RPT	WE				06	2			N	WET	REAR	PRVTE			SW-NE									00
	11A								N	DAY	INJ	PSNGR CAR			01	DRVR	NONE	71	M	OR-Y	026	000	00	07
												02	NONE	0	STOP								011	00
												PSNGR CAR			01	DRVR	INJC	54	F	OR-Y	000	000	00	00
00236	N N N	03/26/2010	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW	CROSS	N	TRF SIGNAL	N	CLR	S-1STOP	01	NONE	0	STRGHT								00	07
NONE	FR				06	2			N	WET	REAR	PRVTE			SW-NE									00
	12P								N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	59	M	OR-Y	014	000	00	07
												02	NONE	0	STOP								011	00
												PSNGR CAR			01	DRVR	NONE	21	F	OR-Y	000	000	00	00
00830	N N N	09/24/2010	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW	CROSS	N	UNKNOWN	N	CLR	S-1STOP	01	NONE	0	STRGHT								00	07
NONE	FR				06	0			N	DRY	REAR	PRVTE			SW-NE									00
	12P								N	DAY	PDO	PSNGR CAR			01	DRVR	NONE	00	F	UNK	026	000	00	07
												02	NONE	0	STOP								011	00
												PSNGR CAR			01	DRVR	NONE	29	F	OR-Y	000	000	00	00
00961	N N N	11/05/2010	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW	CROSS	N	TRF SIGNAL	N	CLR	S-1STOP	01	NONE	0	STRGHT								00	07
NO RPT	FR				06	0			N	DRY	REAR	PRVTE			SW-NE									00
	2P								N	DAY	INJ	PSNGR CAR			01	DRVR	NONE	35	F	OR-Y	014	000	00	07
												02	NONE	0	STOP								011	00
												PSNGR CAR			01	DRVR	INJC	38	M	OTH-Y	000	000	00	00
												02	NONE	0	STOP									

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267/690

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	INVEST	S D P R S W E A U C O DATE E L G H R DAY D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN) LEGS (#LANES)	INT-REL TRAF- CONTL	OFFRD RDNBT DRVWY	WTHR SURF LIGHT	CRASH COLL SVRTY	SPCL USE TRLR QTY MOVE OWNER FROM TO	A S G E LICNS E X RES	PED LOC	ERROR	ACT	EVENT	CAUSE
00066	NO RPT	01/21/2012 SA 9P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW 06	CROSS 0	N UNKNOWN	N N N	CLR DRY DARK	ANGL-STP TURN PDO	01 NONE 0 PRVTE PSNGR CAR	TURN-L SW-N		045	000	000	13 00 13
											02 NONE 0 PRVTE PSNGR CAR	STOP SW-NE		000	012	000	00 00
00818	NONE	09/21/2012 FR 3P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW 06	CROSS 2	N TRF SIGNAL	N N N	CLR WET DAY	S-1STOP REAR PDO	01 NONE 0 PRVTE PSNGR CAR	STRGHT SW-NE		026	000	124 124	07 00 07
											02 NONE 0 PRVTE PSNGR CAR	STOP SW-NE		000	011	000	00 00
00868	CITY	10/02/2012 TU 5P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW 06	CROSS 2	N TRF SIGNAL	N N N	CLR DRY DAY	S-1STOP REAR INJ	01 NONE 0 PRVTE PSNGR CAR	STRGHT SW-NE		043	000	000	07 00 07
											02 NONE 0 PRVTE PSNGR CAR	STOP SW-NE		000	011	000	00 00
											02 NONE 0 PRVTE PSNGR CAR	STOP SW-NE		000	011	000	00 00
00873	NONE	10/03/2012 WE 12P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW 06	CROSS 2	N TRF SIGNAL	N N N	CLR DRY DAY	S-1STOP REAR INJ	01 NONE 0 PRVTE PSNGR CAR	STRGHT SW-NE		026	000	000	07 00 07
											02 NONE 0 PRVTE PSNGR CAR	STOP SW-NE		000	011	000	00 00
00888	NO RPT	10/07/2012 SU 12P	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW 06	CROSS 2	N L-GRN-SIG	N N N	CLR DRY DAY	S-1STOP REAR INJ	01 NONE 0 PRVTE PSNGR CAR	STRGHT SW-NE		047	088	093	01 00 01
											02 NONE 0 PRVTE PSNGR CAR	STOP SW-NE		000	012	000	00 00
											02 NONE 0 PRVTE PSNGR CAR	STOP SW-NE		000	012	000	00 00
00987	NO RPT	11/01/2012 TH 7A	14	PACIFIC HY 99W SPRINGBROOK RD	INTER SW 05	CROSS 0	N L-GRN-SIG	N N N	RAIN WET DAWN	ANGL-OTH TURN PDO	01 NONE 0 PRVTE SCHL BUS	TURN-L S -SW		007,028	000	000	08,02 00 08,02

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OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
URBAN NON-SYSTEM CRASH LISTING

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	S	D	E A U C O DATE	CLASS	CITY STREET	RD CHAR	(MEDIAN)	INT-TYPE	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A S							
INVEST	D	C	S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E X RES	LOC	ERROR	ACT	EVENT	CAUSE
E L G H R DAY	DIRCT	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E LICNS	PED							
												02 NONE 0	TURN-L								
												PRVTE	S -SW							000	00
												PSNGR CAR		01 DRVR	NONE	48 M	OR-Y	000	000	000	00
00278	N	N	N	N	04/04/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLD	S-1STOP	01 NONE 0	STRGHT						
CITY							SPRINGBROOK RD	SW	0	TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000
								06			N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	47 M	OR-Y	026	000
																					00
												02 NONE 0	STOP								
												PRVTE	SW-NE								011
												PSNGR CAR		01 DRVR	INJC	39 M	OR-Y	000	000	000	00
00294	N	N	N	N	04/09/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT						
NO RPT							SPRINGBROOK RD	SW	0	TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000
								06			N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	25 F	OR-Y	026	000
												02 NONE 0	STOP								
												PRVTE	SW-NE								011
												PSNGR CAR		01 DRVR	NONE	55 M	OR-Y	000	000	000	00
00460	N	N	N	N	06/04/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT						
CITY							SPRINGBROOK RD	SW	0	TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000
								06			N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	62 M	OR-Y	026	000
												02 NONE 0	STOP								
												PRVTE	SW-NE								011
												PSNGR CAR		01 DRVR	INJC	44 M	OR-Y	000	000	000	00
00545	N	N	N	N	06/26/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT						
NONE							SPRINGBROOK RD	SW	0	TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000
								06			N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	00 F	UNK	026	000
												02 NONE 0	STOP								
												PRVTE	SW-NE								011
												PSNGR CAR		01 DRVR	NONE	46 F	OR-Y	000	000	000	00
00688	N	N	N	N	08/06/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	UNK	S-1STOP	01 NONE 0	STRGHT						
NONE							SPRINGBROOK RD	SW	0	TRF SIGNAL	N	UNK	REAR	PRVTE	SW-NE						000
								06			N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	18 M	OR-Y	026	000
												02 NONE 0	STOP								
												PRVTE	SW-NE								011
												PSNGR CAR		01 DRVR	NONE	51 M	OR-Y	000	000	000	00
01203	N	N	N	N	12/31/2013	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01 NONE 0	STRGHT						
NO RPT							SPRINGBROOK RD	SW	2	TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000
								06			N	DUSK	INJ	PSNGR CAR		01 DRVR	NONE	41 F	OR-Y	026	000
												02 NONE 0	STOP								
												PRVTE	SW-NE								011
																					00

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CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	INVEST	S D P R S W E A U C O DATE E L G H R DAY D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	INT-REL INT-REL	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	A S G E LICNS	PED	ERROR	ACT	EVENT	CAUSE					
			DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ									
			FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC					
											01	PSNGR CAR		01	DRVR	INJC	61	F	OR-Y OR<25	000	000	00	
											02	NONE 0 PRVTE PSNGR CAR	STOP SW-NE									011 000	00 00
00038		Y N N	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	S-STRGHT	01	NONE 0	STRGHT									01	
NONE		WE		SPRINGBROOK RD	W		TRF SIGNAL	N	WET	REAR		PRVTE	W -E									000	00
		5P			06	0		N	DARK	INJ		PSNGR CAR		01	DRVR	NONE	58	F	OR-Y OR<25	042	000	01	
											02	NONE 0 PRVTE PSNGR CAR	STRGHT W -E									000 000	00 00
00168		N N N	14	PACIFIC HY 99W	INTER	CROSS	N	N	UNK	S-1STOP	01	NONE	STRGHT									07,27	
NONE		WE		SPRINGBROOK RD	W		UNKNOWN	N	UNK	REAR		PRVTE	W -E									000	00
		4P			06	0		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	34	M	OR-Y OR<25	026	000	07,27	
											02	NONE 0 PRVTE PSNGR CAR	STOP W -E									011 000	00 00
00493		Y N N	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE 0	STRGHT									01	
CITY		FR		SPRINGBROOK RD	W		TRF SIGNAL	N	DRY	REAR		PRVTE	E -W									000	00
		12P			06	0		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	00	Unk	UNK UNK	026	000	01	
											02	NONE 0 PRVTE PSNGR CAR	STOP E -W									011 000	00 00
											02	NONE 0 PRVTE PSNGR CAR	STOP E -W									011 000	00 00
00686		N N N	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	STRGHT									07	
NONE		WE		SPRINGBROOK RD	W		UNKNOWN	N	DRY	REAR		PRVTE	W -E									000	00
		4P			06	0		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	52	F	OR-Y OR<25	026	000	07	
											02	NONE PRVTE PSNGR CAR	STOP W -E									011 000	00 00
00248		N N N	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	S-1STOP	01	NONE 0	STRGHT									013	27
NO RPT		MO		SPRINGBROOK RD	W		TRF SIGNAL	N	WET	REAR		PRVTE	W -E									000	00
		7A			06	0		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	29	M	OR-Y OR<25	026	000	07	
											02	NONE PRVTE PSNGR CAR	STOP W -E									011 000	013 00
											03	NONE 0	STOP									000	00

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OREGON.. DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
URBAN NON-SYSTEM CRASH LISTING

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at SPRINGBROOK RD, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 70

SER#	E L G H R DAY	CLASS	CITY STREET	RD CHAR	INT-TYPE		OFFRD	WTHR	CRASH	SPCL USE		MOVE	PRTC	INJ	A S			ERROR	ACT	EVENT	CAUSE		
					(MEDIAN)	INT-REL				TRLR	QTY				E	X	RES					LOC	
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE
00272	N N N N N 03/27/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT										03
CITY	TU		SPRINGBROOK RD	CN		TRF SIGNAL	N	WET	ANGL		PRVTE		W -E										00
	9P			03	0		N	DARK	INJ		PSNGR CAR		01	DRVR	NONE	17	F	OR-Y		021	000		03
																		OR<25					
										02	NONE	0	STRGHT									000	00
											PRVTE		N -S									000	00
											PSNGR CAR		01	DRVR	INJC	63	M	OR-Y		000	000		00
																		OR<25					00
00821	N N N 09/21/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	O-1TURN	01	NONE	0	STRGHT										04
NO RPT	FR		SPRINGBROOK RD	CN		L-GRN-SIG	N	WET	TURN		PRVTE		NE-SW									000	00
	6A			02	2		N	DAY	PDO		PSNGR CAR		01	DRVR	NONE	00	F	UNK		020	000		04
																		UNK					
										02	NONE	0	TURN-L									000	00
											PRVTE		SW-N									000	00
											PSNGR CAR		01	DRVR	NONE	22	M	OR-Y		000	000		00
																		OR<25					00
00922	N N N N N 10/15/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT										02
CITY	MO		SPRINGBROOK RD	CN		TRF SIGNAL	N	WET	ANGL		PRVTE		W -E									000	00
	7P			03	2		N	DARK	PDO		PSNGR CAR		01	DRVR	NONE	65	M	OR-Y		028	000		02
																		OR<25					
										02	NONE	0	STRGHT									015	00
											PRVTE		N -S									000	00
											PSNGR CAR		01	DRVR	NONE	56	M	OR-Y		000	000		00
																		OR<25					00
01045	N N N 11/15/2012	14	PACIFIC HY 99W	INTER	CROSS	N	N	CLD	S-1STOP	01	NONE	0	STRGHT									092	07, 14
NONE	TH		SPRINGBROOK RD	CN		TRF SIGNAL	N	DRY	REAR		PRVTE		NE-SW									000	00
	3P			02	2		N	DAY	INJ		PSNGR CAR		01	DRVR	NONE	25	F	OR-Y		026	000		07, 14
																		OR<25					
										02	NONE	0	STOP									011	00
											PRVTE		NE-SW									000	00
											PSNGR CAR		01	DRVR	INJC	30	M	OR-Y		000	000		00
																		OR<25					00

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10/23/2014

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CRASH SUMMARIES BY YEAR BY COLLISION TYPE

PACIFIC HY 99W at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

COLLISION TYPE	FATAL CRASHES	NON- FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER- SECTION	INTER- SECTION RELATED	OFF- ROAD
YEAR: 2013														
REAR-END	0	2	0	2	0	2	0	1	1	2	0	2	0	0
TURNING MOVEMENTS	0	0	2	2	0	0	0	2	0	2	0	2	0	0
YEAR 2013 TOTAL	0	2	2	4	0	2	0	3	1	4	0	4	0	0
YEAR: 2012														
REAR-END	0	3	1	4	0	4	0	3	1	4	0	4	0	0
TURNING MOVEMENTS	0	0	1	1	0	0	0	1	0	0	1	1	0	0
YEAR 2012 TOTAL	0	3	2	5	0	4	0	4	1	4	1	5	0	0
YEAR: 2011														
REAR-END	0	3	0	3	0	4	0	2	1	1	2	3	0	0
YEAR 2011 TOTAL	0	3	0	3	0	4	0	2	1	1	2	3	0	0
YEAR: 2010														
PEDESTRIAN	0	1	0	1	0	1	0	1	0	1	0	1	0	0
YEAR 2010 TOTAL	0	1	0	1	0	1	0	1	0	1	0	1	0	0
YEAR: 2009														
ANGLE	0	1	0	1	0	1	0	1	0	0	1	1	0	0
REAR-END	0	1	2	3	0	1	0	2	1	3	0	3	0	0
YEAR 2009 TOTAL	0	2	2	4	0	2	0	3	1	3	1	4	0	0
FINAL TOTAL	0	11	6	17	0	13	0	13	4	13	4	17	0	0

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CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 17

SER#	INVEST	D C S L K	TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	TRLR QTY	MOVE	A S	E X RES	LOC	ERROR	ACT	EVENT	CAUSE															
																							E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RDNBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E LICNS	PED
00594	N N N N N		08/07/2009	14	BRUTSCHER ST	INTER	CROSS	N	N	N	CLD	S-1STOP	01 NONE 0	STRGHT							013	07															
	CITY		FR		PACIFIC HY 99W	UN					DRY	REAR	PRVTE	N -S							000	00															
			8A			06	0				DAY	INJ	PSNGR CAR			01 DRVR	NONE	53 F	OR-Y	026	000	07															
													02 NONE 0	STOP																							
													PRVTE	N -S							011	013															
													PSNGR CAR			01 DRVR	NONE	21 F	OR-Y	000	000	00															
													03 NONE 0	STOP																							
													PRVTE	N -S							011	00															
													PSNGR CAR			01 DRVR	INJC	22 F	OR-Y	000	000	00															
00920	N N N		11/25/2009	14	BRUTSCHER ST	INTER	3-LEG	N	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								07															
	NONE		WE		PACIFIC HY 99W	NE					DRY	REAR	PRVTE	NE-SW							000	00															
			3P			06	0				DAY	PDO	PSNGR CAR			01 DRVR	NONE	40 M	OR-Y	026	000	07															
													02 NONE 0	STOP																							
													PRVTE	NE-SW							011	00															
													PSNGR CAR			01 DRVR	NONE	00 Unk	UNK	000	000	00															
00415	N N N		05/19/2012	14	BRUTSCHER ST	INTER	3-LEG	N	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								013	07														
	NO RPT		SA		PACIFIC HY 99W	NE					DRY	REAR	PRVTE	NE-SW							000	00															
			10A			06	0				DAY	INJ	PSNGR CAR			01 DRVR	NONE	31 M	OR-Y	026	000	07															
													02 NONE 0	STOP																							
													PRVTE	NE-SW							011	013															
													PSNGR CAR			01 DRVR	NONE	36 F	OR-Y	000	000	00															
													03 NONE 0	STOP																							
													PRVTE	NE-SW							011	00															
													PSNGR CAR			01 DRVR	INJC	38 M	OR-Y	000	000	00															
00518	N N N		06/22/2012	14	BRUTSCHER ST	INTER	3-LEG	N	N	N	RAIN	S-1STOP	01 NONE 0	STRGHT									27														
	NO RPT		FR		PACIFIC HY 99W	NE					WET	REAR	PRVTE	NE-SW							000	00															
			4P			06	0				DAY	INJ	PSNGR CAR			01 DRVR	NONE	20 F	OR-Y	026	000	27															
													02 NONE 0	STOP																							
													PRVTE	NE-SW							011	00															
													PSNGR CAR			01 DRVR	INJC	42 F	OR-Y	000	000	00															
00907	N N N		10/04/2012	14	BRUTSCHER ST	INTER	CROSS	N	N	N	CLR	S-1STOP	01 NONE 0	STRGHT									07														
	NONE		TH		PACIFIC HY 99W	NE					DRY	REAR	PRVTE	NE-SW							000	00															
			5P			06	0				DAY	PDO	PSNGR CAR			01 DRVR	NONE	26 M	OR-Y	026	000	07															
													02 NONE 0	STOP																							
													PRVTE	NE-SW							011	00															
													PSNGR CAR			01 DRVR	NONE	40 M	OR-Y	000	000	00															
00829	N N N		09/23/2013	14	BRUTSCHER ST	INTER	3-LEG	N	N	N	RAIN	S-1STOP	01 NONE 0	STRGHT									07														
	NONE		MO		PACIFIC HY 99W	NE					WET	REAR	PRVTE	NE-SW							000	00															
			2P			06	0				DAY	INJ	PSNGR CAR			01 DRVR	NONE	40 F	OR-Y	026	000	07															

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275/690

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 17

SER#	INVEST	D C S L K TIME	CLASS	CITY STREET	RD CHAR	INT-TYPE	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	MOVE	PRTC	INJ	G E LICNS	PED	ERROR	ACT EVENT	CAUSE
NO RPT		E L G H R DAY	DIST	FIRST STREET	DIRECT	(#LANES)	TRAF-	RNDBT	SURF	COLL	TRLR QTY	FROM	P# TYPE	SVRITY	E X RES	LOC			
											02 NONE 0	STOP							
											PRVTE	NE-SW						011	00
											PSNGR CAR		01 DRVR	INJC	71 M	OR-Y	000	000	00
00661	N N N	08/18/2011	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	S-1STOP	01 NONE 0	STRGHT						013	32
		TH		PACIFIC HY 99W	E		UNKNOWN	N	DRY	REAR	PRVTE	E -W						000	00
		5P			06	0		N	DAY	INJ	PSNGR CAR		01 DRVR	INJB	35 F	OR-Y	052,026	000	32
											02 NONE 0	STOP							
											PRVTE	E -W						011 013	00
											PSNGR CAR		01 DRVR	INJC	43 M	OR-Y	000	000	00
											03 NONE 0	STOP							
											PRVTE	E -W						011	00
											PSNGR CAR		01 DRVR	NONE	59 M	OR-Y	000	000	00
01136	N N N N N	12/21/2010	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLD	PED	01 NONE 0	TURN-L							02
		TU		PACIFIC HY 99W	SW		TRF SIGNAL	Y	DRY	PED	PRVTE	SE-SW						000	00
		9A			06	0		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	33 M	OTH-Y	029	000	02
00469	N N N N N	06/11/2012	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	S-1STOP	01 NONE 0	STRGHT							27
		MO		PACIFIC HY 99W	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000	00
		10A			06	0		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	22 F	OR-Y	026	000	27
											02 NONE 0	STOP							
											PRVTE	SW-NE						011	00
											PSNGR CAR		01 DRVR	INJB	46 F	OR-Y	000	000	00
											02 NONE 0	STOP							
											PRVTE	SW-NE						011	00
											PSNGR CAR		02 PSNG	INJB	25 F		000	000	00
01018	N N N	11/09/2012	14	BRUTSCHER ST	INTER	CROSS	N	N	CLD	ANGL-STP	01 NONE 0	TURN-L						092	32,27
		FR		PACIFIC HY 99W	SW		TRF SIGNAL	N	DRY	TURN	PRVTE	SE-SW						000	00
		5P			05	0		N	DUSK	PDO	PSNGR CAR		01 DRVR	NONE	44 M	OR-Y	052,016	038	32,27
											02 NONE 0	STOP							
											PRVTE	NE-SW						011 092	00
											PSNGR CAR		01 DRVR	NONE	71 M	OR-Y	000	000	00
00585	N N N N N	07/09/2013	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	S-1STOP	01 NONE 0	STRGHT							07
		TU		PACIFIC HY 99W	SW		TRF SIGNAL	N	DRY	REAR	PRVTE	SW-NE						000	00
		6A			06	0		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	50 M	OR-Y	026	000	07
											02 NONE 0	STOP							
											PRVTE	SW-NE						011	00

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CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 17

SER#	S D		CLASS	CITY STREET	RD CHAR	INT-TYPE		OFFRD	WTHR	CRASH	SPCL USE		MOVE	A S				ERROR	ACT	EVENT	CAUSE	
	E A U C O DATE	P R S W				(MEDIAN)	INT-REL				TRLR QTY	OWNER		INJ	G E LICNS	PED						
INVEST	E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE	
D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE
												01	DRVR	INJC	36	M	OR-Y	OR<25	000	000		00
00800	N N N	10/30/2009	14	BRUTSCHER ST	INTER	CROSS	N	N	CLD	S-1STOP	01	NONE	0	STRGHT								07, 27
NONE	FR			PACIFIC HY 99W	W			TRF SIGNAL	N	SNO	REAR	01	DRVR	NONE	43	M	OR-Y	OR<25	026	000		00
	9A				06	0			N	DAY	PDO											07, 27
												01	NONE	0	STRGHT							00
												02	PSNG	NO<5	03	F			000	000		00
												02	NONE	0	STOP							00
																						00
												01	DRVR	NONE	00	M	OR-Y	OR<25	000	000		00
00045	N N N N N	01/14/2011	14	BRUTSCHER ST	INTER	3-LEG	N	N	RAIN	S-1STOP	01	NONE	0	STRGHT								07
CITY	FR			PACIFIC HY 99W	W			TRF SIGNAL	N	WET	REAR	01	DRVR	NONE	23	M	OR-Y	OR<25	026	000		00
	5P				06	0			N	DARK	INJ											07
												02	NONE	0	STOP							00
																						00
												01	DRVR	INJC	22	F	OR-Y	OR<25	000	000		00
01005	N N N	11/07/2011	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	S-1STOP	01	NONE	0	STRGHT								07
NONE	MO			PACIFIC HY 99W	W			TRF SIGNAL	N	DRY	REAR	01	DRVR	NONE	42	F	OR-Y	OR>25	026	000		00
	5P				06	0			N	DLIT	INJ											07
												02	NONE	0	STOP							00
																						00
												01	DRVR	INJC	31	M	OR-Y	OR>25	000	000		00
00063	N N N N N	01/18/2009	14	BRUTSCHER ST	INTER	CROSS	N	N	CLR	ANGL-OTH	01	NONE	0	STRGHT								04
CITY	SU			PACIFIC HY 99W	CN			TRF SIGNAL	N	DRY	ANGL	01	DRVR	NONE	18	F	OR-Y	OR<25	020	000		00
	11P				04	0			N	DARK	INJ											04
												02	NONE	0	TURN-L							00
																						00
												01	DRVR	INJB	16	M	OR-Y	OR<25	000	000		00
00375	N N N	05/05/2013	14	BRUTSCHER ST	INTER	CROSS	N	N	CLR	ANGL-OTH	01	NONE	0	U-TURN								08
NO RPT	SU			PACIFIC HY 99W	CN			TRF SIGNAL	N	DRY	TURN	01	DRVR	NONE	63	M	OR-Y	OR<25	008	000		00
	12P				01	0			N	DAY	PDO											08
												02	NONE	0	STRGHT							00
																						00
												01	DRVR	NONE	46	F	OR-Y	OR<25	000	000		00
00740	N N N	08/23/2013	14	BRUTSCHER ST	INTER	3-LEG	N	N	CLR	O-1TURN	01	NONE	0	STRGHT							002	04, 27
NO RPT	FR			PACIFIC HY 99W	CN			TRF SIGNAL	N	DRY	TURN	01	DRVR	NONE	31	F	OR-Y	OR<25	020	000	002	00
	1P				01	0			N	DAY	PDO											04, 27
																						00
												01	NONE	0	STRGHT							00

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277/690

CITY OF NEWBERG, YAMHILL COUNTY

PACIFIC HY 99W at BRUTSCHER ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 17

SER#	E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E LICNS	PED	ERROR	ACT	EVENT	CAUSE		
INVEST	D C S L K TIME	FROM	SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC				
										PRVTE	W -E						000		00		
										PSNGR CAR			02	PSNG	NO<5	01 M		000	000	00	
										02	NONE	0		TURN-L							
										PRVTE	S -W							000		00	
										PSNGR CAR			01	DRVR	NONE	23 F	OR-Y		000	000	00
																OR<25					

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COLLISION TYPE	FATAL CRASHES	NON- FATAL CRASHES	PROPERTY DAMAGE ONLY	TOTAL CRASHES	PEOPLE KILLED	PEOPLE INJURED	TRUCKS	DRY SURF	WET SURF	DAY	DARK	INTER- SECTION	INTER- SECTION RELATED	OFF- ROAD
YEAR: 2013														
REAR-END	0	1	0	1	0	1	0	0	1	1	0	1	0	0
YEAR 2013 TOTAL	0	1	0	1	0	1	0	0	1	1	0	1	0	0
FINAL TOTAL	0	1	0	1	0	1	0	0	1	1	0	1	0	0

CITY OF NEWBERG, YAMHILL COUNTY

SPRINGBROOK RD at HAYES ST, City of Newberg, Yamhill County, 01/01/2009 to 12/31/2013

Total crash records: 1

SER#	INVEST	S D	P R S W	E A U C O DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	SPCL USE	TRLR QTY	MOVE	A S	E L G H R DAY	DIST	FIRST STREET	DIRECT	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G E LICNS	PED	ACT	EVENT	CAUSE
						SECOND STREET	LOCTN	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E X RES	LOC	ERROR														
00830		N N N N N		09/23/2013	16	HAYES ST	INTER	3-LEG	N	N	N	CLD	S-1STOP	01 NONE	0	STRGHT																		07	
CITY				MO	0	SPRINGBROOK RD	N		TRF SIGNAL	N	N	SNO	REAR	PRVTE	N -S																		000	00	
				8A			06	0		N	N	DAY	INJ	PSNGR CAR													01	DRVR	NONE	48	F	OR-Y	026	000	07
														02 NONE	0	STOP																			
														PRVTE	N -S																			011	00
														PSNGR CAR													01	DRVR	INJC	37	F	OR-Y	000	000	00

Disclaimer: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811.720. The Crash Analysis and Reporting Unit is committed to providing the highest quality crash data to customers. However, because submittal of crash report forms is the responsibility of the individual driver, the Crash Analysis and Reporting Unit can not guarantee that all qualifying crashes are represented nor can assurances be made that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirement, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

APPENDIX G
**SIGNAL PLANS AND
TIMING PLANS**

SIGNAL PLAN

PACIFIC HWY W AT SPRINGBROOK ST.
HWY. 1W MP. 22.03



NOTE:
SIGNAL PLANS ARE
REVERSED FROM
ROADWAY PLANS

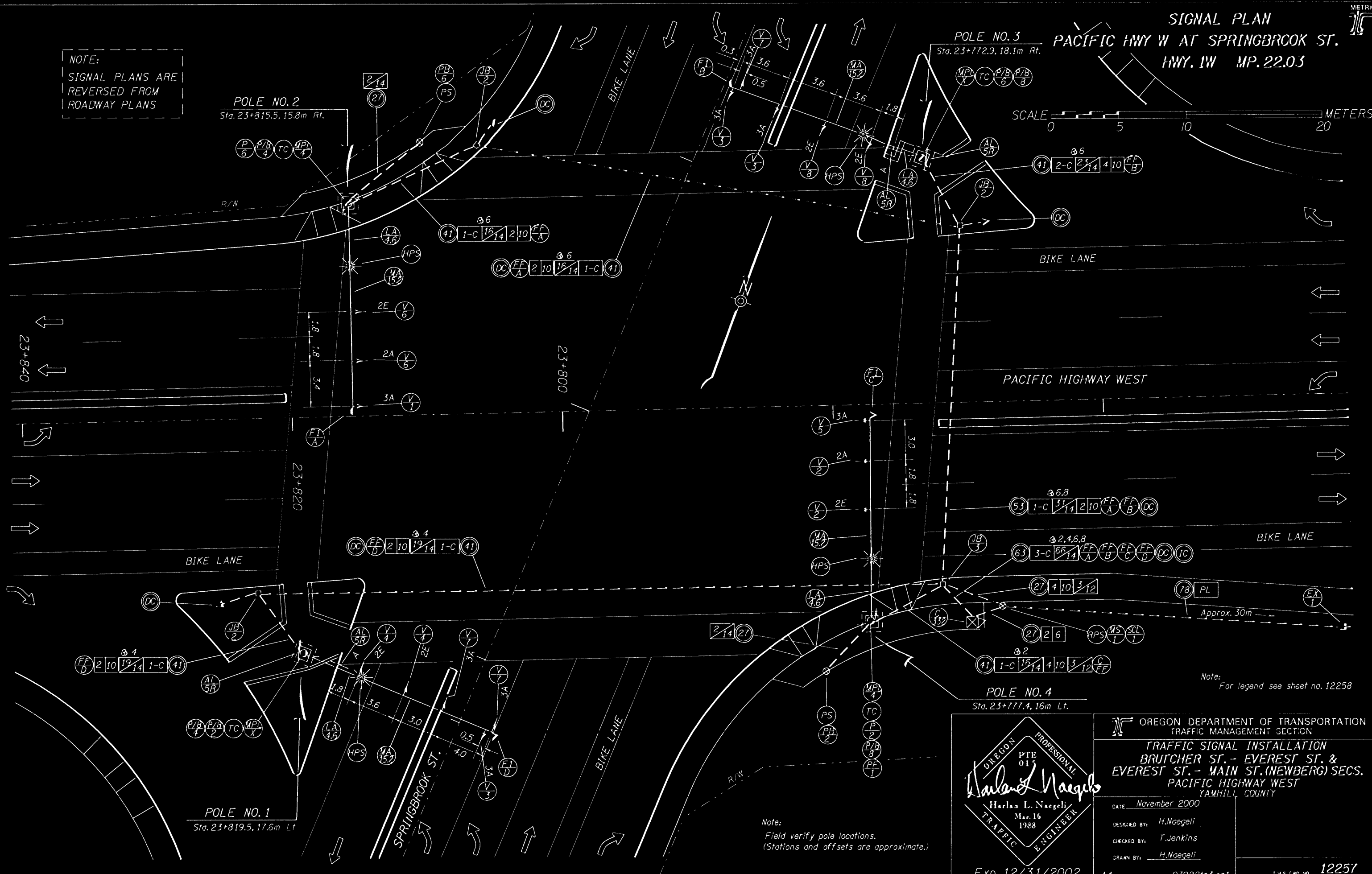
SCALE 0 5 10 20 METERS

POLE NO. 2
Sta. 23+815.5, 15.8m Rt.

POLE NO. 3
Sta. 23+772.9, 18.1m Rt.

POLE NO. 4
Sta. 23+777.4, 16m Lt.

POLE NO. 1
Sta. 23+819.5, 17.6m Lt.



Note:
For legend see sheet no. 12258

Note:
Field verify pole locations.
(Stations and offsets are approximate.)

OREGON PROFESSIONAL
PTE 015
Harlan L. Naegeli
Harlan L. Naegeli
Mar. 16
1988
TRAFFIC ENGINEER
Exp. 12/31/2002

OREGON DEPARTMENT OF TRANSPORTATION
TRAFFIC MANAGEMENT SECTION
TRAFFIC SIGNAL INSTALLATION
BRUTCHER ST. - EVEREST ST. &
EVEREST ST. - MAIN ST. (NEWBERG) SECS.
PACIFIC HIGHWAY WEST
YAMHILL COUNTY

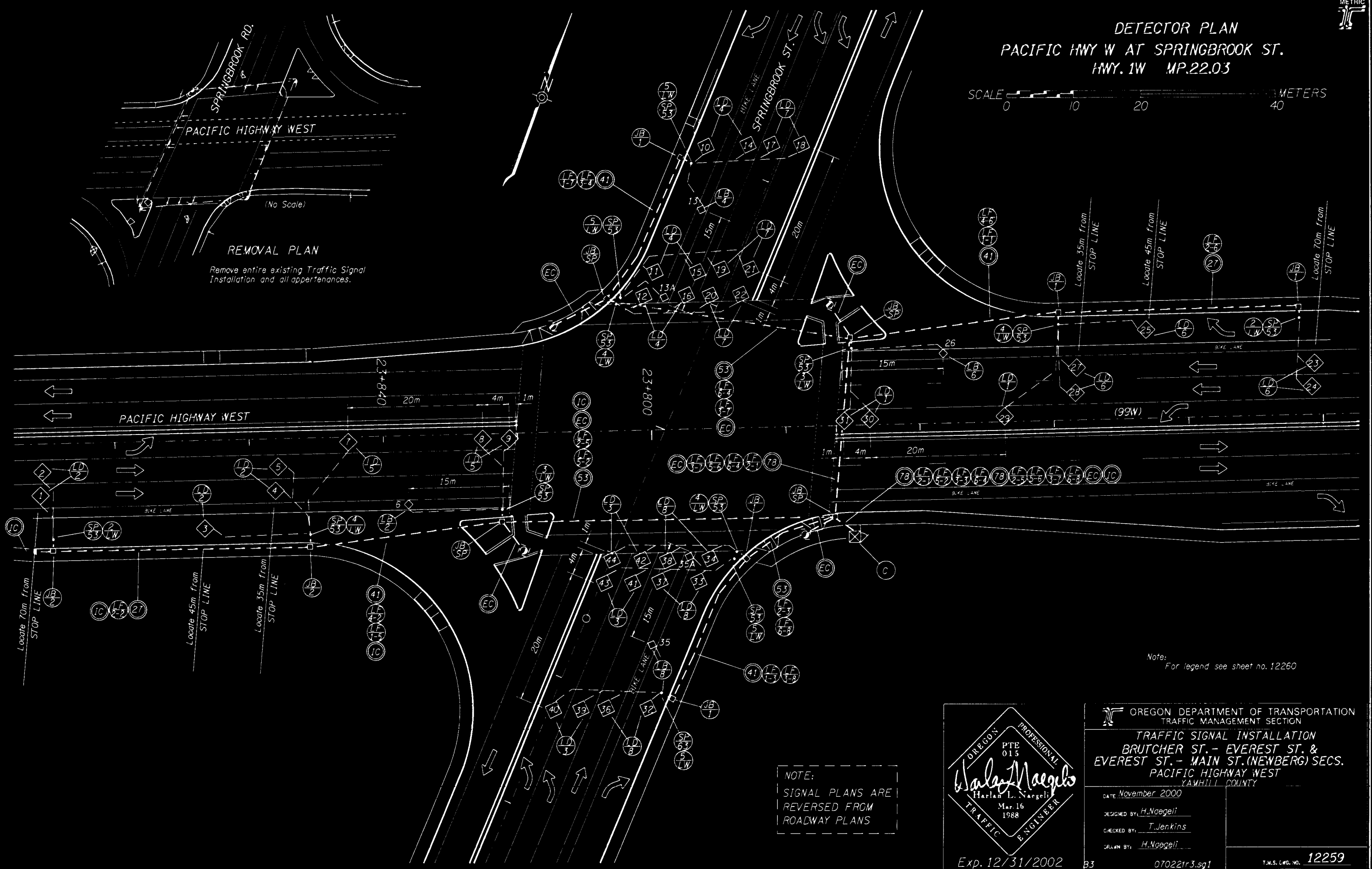
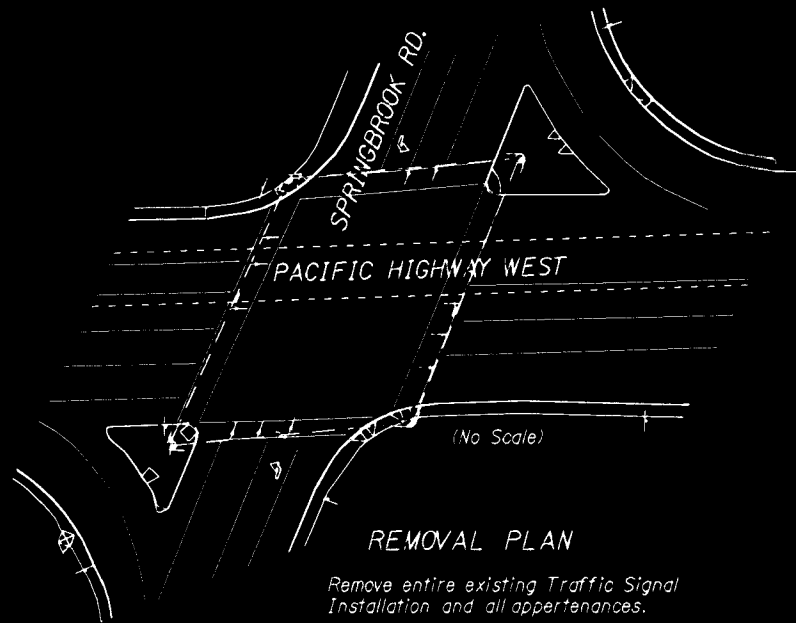
DATE: November 2000
DESIGNED BY: H. Naegeli
CHECKED BY: T. Jenkins
DRAWN BY: H. Naegeli

T.M.S. C.D.G. NO. 12257



DETECTOR PLAN PACIFIC HWY W AT SPRINGBROOK ST. HWY. 1W MP.22.03

SCALE 0 10 20 40 METERS



Note: For legend see sheet no. 12260

NOTE:
SIGNAL PLANS ARE
REVERSED FROM
ROADWAY PLANS

OREGON PROFESSIONAL
PTE 015
Harlan L. Naegeli
Harlan L. Naegeli
Mar. 16 1988
TRAFFIC ENGINEER

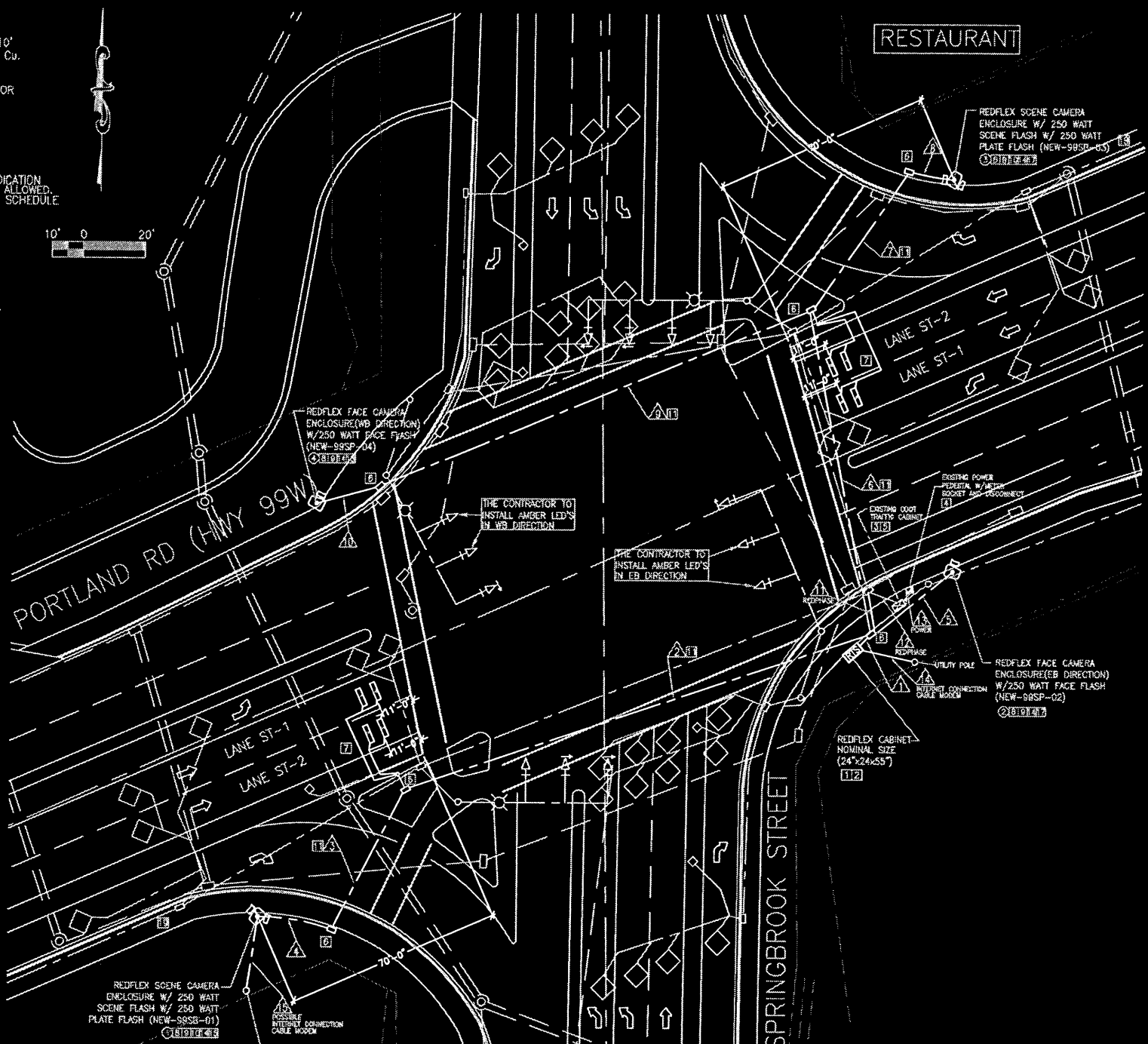
OREGON DEPARTMENT OF TRANSPORTATION
TRAFFIC MANAGEMENT SECTION
TRAFFIC SIGNAL INSTALLATION
BRUTCHER ST. - EVEREST ST. &
EVEREST ST. - MAIN ST. (NEWBERG) SECS.
PACIFIC HIGHWAY WEST
YAMHILL COUNTY

DATE November 2000
DESIGNED BY: H. Naegeli
CHECKED BY: T. Jenkins
DRAWN BY: H. Naegeli

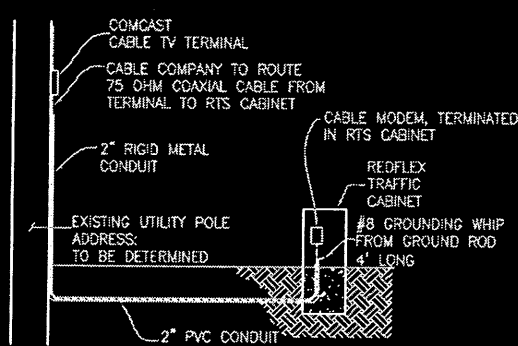
Exp. 12/31/2002 B3 070221r3.sg1 T.M.S. C.W.G. NO. 12259

CONSTRUCTION NOTES

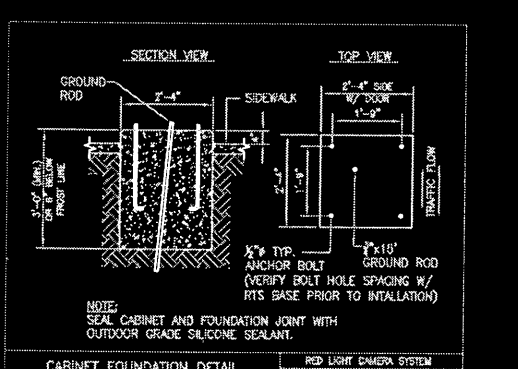
1. INSTALL FOUNDATION AND REDFLEX CONTROL CABINET. INSTALL 3/4" x 10" GROUND ROD AND BOND TO CABINET AND POLE BASES WITH #6 BARE CU. SEE CABINET FOUNDATION DETAIL.
2. CONTRACTOR SHALL TERMINATE AND LAND ALL POWER CIRCUITS INTO REDFLEX CABINET (AN ODOT ELECTRICIAN SHALL BE ON-SITE TO MONITOR CONTRACTOR'S WORK AND RESPOND TO THE TRAFFIC SIGNAL OPERATION IN THE EVENT IS NEEDED).
3. INSTALL IN-LINE FUSE HOLDER ON RED PHASE CONDUCTOR WITH 5 AMP FUSE INSIDE REDFLEX CABINET, TO PROTECT ODOT TRAFFIC CABINET.
4. INSTALL 40 AMP CIRCUIT BREAKER INTO EXISTING POWER PANEL FOR 120V SUPPLY TO REDFLEX CABINET.
5. INSTALL SOLID STATE RELAY (SSR) DEVICE ON EACH SEPARATE RED INDICATION IN ODOT CABINET. NO DIRECT CONNECTION TO RED PHASING SHALL BE ALLOWED. SEE CONDUCTOR SCHEDULE FOR APPROACHES AND SEE DETAIL. (NOTE: SCHEDULE CONTROLLER CABINET ACCESS WITH ODOT REGION 2 ELECTRICIAN AT (503) 986-2706 AT LEAST 48 HOURS BEFORE WORK IS TO OCCUR.)
6. INSTALL JB-1 (12" x 17" x 12") CONCRETE JUNCTION BOX PER ODOT STANDARDS.
7. INSTALL LOOPS AND CONDUIT SUBROUT FOR REDFLEX DETECTION. EACH LOOP MUST HAVE A SEPARATE HOME RUN AND LOOP LEAD IN CABLE. MAXIMUM 2 LOOPS PER SAWCUT. REDFLEX LOOPS TO BE LABELED AT REDFLEX CABINET.
8. INSTALL FOUNDATION, POLE BASE AND POLE FOR REDFLEX EQUIPMENT. SEE LOOP DETAIL AND DRAWING FOR QUANTITY AND LOOP ID#.
9. SEE DRAWING FOR LOCATION. SEE POLE DETAIL.
10. INSTALL CAMERA ENCLOSURE AND FLASH ON REDFLEX POLE, AIM AT SIGNAL HEADS.
11. INSTALL FLASH UNIT ON REDFLEX POLE, AIM AT VIOLATION LINE.
12. CONDUIT TO BE BORED UNDER ROADWAY-30" COVER MIN.
13. INSTALL AMBER LED'S PER ODOT STANDARDS (AN ODOT ELECTRICIAN SHALL BE ON-SITE TO MONITOR CONTRACTOR'S WORK AND RESPOND TO THE TRAFFIC SIGNAL OPERATION IN THE EVENT IS NEEDED).
14. NOT USED.
15. CONNECT POLE TO SOLID BARE BOND GROUND & GROUNDING ROD IN IN POLE ENCLOSURE & TO SYSTEM GROUND BONDED BACK AT REDFLEX CAMERA.
16. EXISTING TERMINATE POLE TO REDFLEX CABINET POWER CIRCUIT #1 (HOT & NEUTRAL).
17. EXISTING TERMINATE POLE TO REDFLEX CABINET POWER CIRCUIT #2 (HOT & NEUTRAL).
18. EXISTING TERMINATE POLE TO REDFLEX CABINET POWER CIRCUIT #3 (HOT & NEUTRAL).
19. TERMINATE POLE TO REDFLEX CABINET POWER CIRCUIT #4 (HOT & NEUTRAL).
20. THE CONTRACTOR TO INSTALL ADVANCED WARNING SIGNAGE, WHICH INCLUDES SIGN PANEL, POST AND INCIDENTAL HARDWARE PER ODOT STANDARDS. SPOT SIGN AT 350' AWAY FROM VIOLATION LINE.
21. NOT USED.



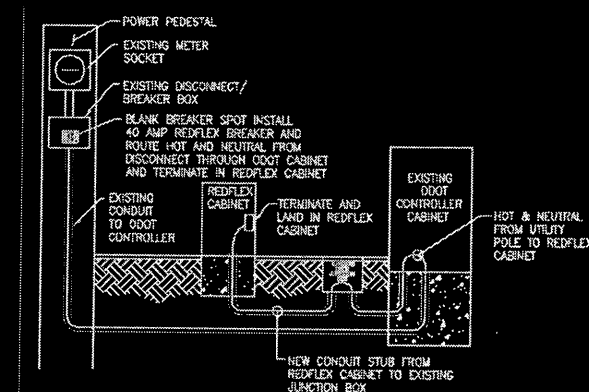
CONDUCTORS		CONDUITS										TERMINATION		
SOURCE	AWG#	1	2	3	4	5	6	7	8	9	10			
POLE 1	CATS											REDFLEX CABINET		
POLE 2	CATS	2			2							REDFLEX CABINET		
POLE 3	CATS											REDFLEX CABINET		
POLE 4	CATS	2					2	2				REDFLEX CABINET		
CITY POWER PEDestal (120V)	#8											REDFLEX CABINET		
TRAFFIC SIGNAL RED PHASE (E, W, NB, SB)	#14								4	4		REDFLEX CABINET		
REDFLEX LOOP LEAD-IN	DLC											REDFLEX CABINET		
POWER CIRCUIT 1 (REDFLEX CABINET)	#10	2	2	2	2							POLE 1		
POWER CIRCUIT 2 (REDFLEX CABINET)	#10											POLE 2		
POWER CIRCUIT 3 (REDFLEX CABINET)	#10											POLE 3		
POWER CIRCUIT 4 (REDFLEX CABINET)	#10						2	2	2			POLE 4		
SOLID BARE BOND	#8	1	1	1	1	1	1	1	1	1	1	ALL REDFLEX EQUIPMENT		
CABLE MODERN POINT OF SERVICE	CATS											REDFLEX CABINET		
TOTAL NEW CONDUCTORS		26	6	5	5	5	6	5	5	5	7	7	3	
CONDUIT SIZE (INCHES)														
NEW/EXISTING CONDUIT		N	N	N	N	N	N	N	N	N	N	E	E	



TYPICAL CABLE MODEM CONNECTION | REDFLEX CAMERA SYSTEM



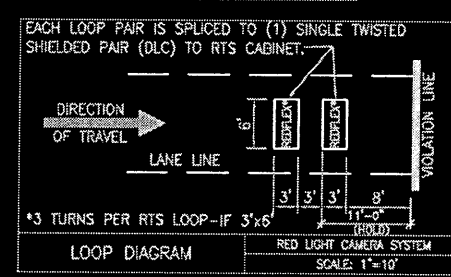
CABINET FOUNDATION DETAIL | RED LIGHT CAMERA SYSTEM SCALE: N.T.S.



POWER SERVICE CONNECTION (TYP) | REDFLEX CAMERA SYSTEM

POLE SCHEDULE

NO.	TYPE	HEIGHT	NEW/EXISTING
1	REDFLEX	10'	N
2	REDFLEX	10'	N
3	REDFLEX	10'	N
4	REDFLEX	10'	N



LOOP DIAGRAM | RED LIGHT CAMERA SYSTEM SCALE: 1"=10'

ABBREVIATIONS

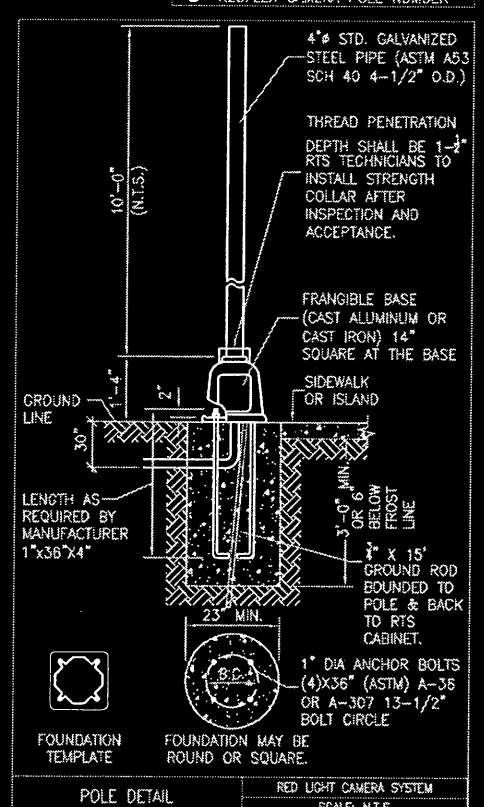
- TSP — TWISTED SHIELDED PAIR
- E.O.P. — EDGE OF PAVEMENT
- A.F.G. — ABOVE FINISH GRADE
- CAT 5 — CATEGORY FIVE CABLE
- DLC — DETECTOR LOOP CABLE (SHIELDED)
- STP — SINGLE TWISTED PAIR
- L.E.D. — LIGHT EMITTING DIODE
- EB — EASTBOUND (MOVEMENT)
- WB — WEST BOUND (MOVEMENT)

Thomas Cahill
 ODOT TRAFFIC ENGINEERING AND OPERATIONS APPROVED
 14274
 TEDS DRVG NO.

NEED PE STAMP

LEGEND

- EXISTING TRAFFIC CONTROL CABINET
- POWER CABINET
- REDFLEX CONTROL CABINET
- CAMERA W/FLASH
- PLATE FLASH
- REDFLEX CAMERA POLE NUMBER



POLE DETAIL | RED LIGHT CAMERA SYSTEM SCALE: N.T.S.

NO.	REVISION	DATE	BY
	REVISAS AS CONSTRUCTED BY		
CITY OF NEWBERG OREGON - ENGINEERING DEPT.			
REDLIGHT ENFORCMENT			
PORTLAND ROAD (HWY 99W) AND SPRINGBROOK STREET			
DATE: 01/10/10	SCALE: 1"=10'	PROJECT NO. 1252	
DESIGNED BY: [Signature]	CHECKED BY: [Signature]	DRAWING NO. 3 of 5	

Tuesday, October 28, 2014 12:33

Intersection Name	12 - OR99W at Springbrook	Local ID	12
Intersection Telephone Number			
System Name	64 - Newberg-Dundee	System ID	64
Controller Type	Voyage - C1-C11		
Controller Serial Number		Installation Date	
Programmed by		Programmed Date	

Graphic Map Background	Phase Rotation Diagram

Control Data (next/2/2)

Controller Function and Timing (next/2/1, next/2/2)

Security, Sequence, Initialization

Security Code	****	0 = disabled, or 1000-9999
Sequence	7	0 = sequential, 1 = quad left turn, 2-6 = special A-E, 7 = lead lag

	Lead Lag (next/2/2/3)			
	Phases 1 - 2	Phases 3 - 4	Phases 5 - 6	Phases 7 - 8
	2	2	2	2
	0 = no reversal, 1 = reversal, 2 = by coord plan or clock			

Initialization and Flash (next/2/2/5)

	Initialization	Flash Entry	Flash Exit	
Ring 1 Phase	1	0	1	phase 1-8
Ring 2 Phase	5	0	5	phase 1-8
Interval	0	0	0	0 = red, 1 = yellow, 2 = green
Power up Flash	0.0	0.0 - 25.5 seconds	First All Red	6.0
				0.0 - 25.5 seconds

Soft Flash (next/2/2/5)

Phase	1	2	3	4	5	6	7	8	0 = dark, 1=flash yel WIG, 2 = flash yel WAG, 3 = flash red WIG, 4 = flash red WAG				
	3	4	3	4	3	4	3	4					
Overlap	A	B	C	D	E	F	G	H	I	J	K	L	same as phase
	3	4	3	4	3	4	3	4	3	4	3	4	
Internal Logic Output	1	2	3	4	5	6	7	8	9	10	11	12	0 = normal, 1 = dark, 2 = flash WIG
	0	0	0	0	0	0	0	0	0	0	0	0	

Per Phase Functions (next/2/2/3, next/2/2/1)									
	1	2	3	4	5	6	7	8	ATTACHMENT 4
Phases Used	X	X	X	X	X	X	X	X	X = on
Restricted Phases									X = on (Sequence 2, 6, 7 only)
Exclusive Phases									X = on (Sequence 7 only)
Yellow Lock									X = on
Min Recall		X				X			
Max Recall									
Ped Recall									
Red Lock									
Max Out Recall Inhibit	X	X	X	X	X	X	X	X	
Soft Recall									
Free Walk Rest									
Conditional Ped									
Disable Inhibit Max Termination									
Call to Non Act 1									
Call to Non Act 2									
Dual Entry (next/2/2/9/3)									
Mode	1	0 = off, 1 = on, 2 = Not Used, 3 = by coord plan, 4 = by time clock circuit 61							
Dual Entry Phase -->	1	2	3	4	5	6	7	8	
Phase	0	0	0	8	0	0	0	4	0 = none, 1-8 = phase 1-8
Conditional Service, Five Section Head									
Conditional Service (next/2/2/9/3)			5 Section Head Logic (next/2/2/9/4)						
Phase	Mode	CS Max Time	X Omits Y		Anti-Trap			Yellow Blanking LT	
			X : Y		Trap Protected Phase	Next Phase	Phase		
Phase 1	0	0	6 : 1	0	1		< (5)	1	
Phase 3	0	0	8 : 3	0	3		< (7)	3	
Phase 5	0	0	2 : 5	0	5		< (1)	5	
Phase 7	0	0	4 : 7	0	7		< (3)	7	
0 = off, 1 = C.S.On. 2 = C.S. on by TOD circuit 57, 3 = N/A, 4 = C.S. and C.R. On, 5 = C.R. on by TOD circuit 57.			0=off, 1=side call, 2=no side call		X = On				

Phase Times (next/2/2/2, next/2/2/9/5)									
	1	2	3	4	5	6	7	8	ATTACHMENT 4
Movement									
Minimum Green	4	10	4	4	4	15	4	4	0 - 255 sec
Passage	2.5	4.2	2.3	2.3	2.3	4.2	2.3	2.3	0.0 - 25.5 sec
Yellow	4.0	4.5	4.0	4.0	4.0	4.5	4.0	4.0	0.0 - 25.5 sec
Red Clearance	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.0 - 25.5 sec or 0 - 255 sec
Max 1	25	50	20	25	20	50	20	25	0 - 255 sec
Max 2	25	50	20	25	20	50	20	25	0 - 255 sec
Walk	0	7	0	7	0	7	0	7	0 - 255 sec
Ped Clear	0	29	0	24	0	28	0	23	0 - 255 sec
Seconds Per Actuation	0.0	0.6	0.0	0.0	0.0	0.6	0.0	0.0	0.0 - 25.5 sec
Time Before Reduction	8	10	8	8	8	10	8	8	0 - 255 sec
Time to Reduce	3	20	3	3	3	20	3	3	0 - 255 sec
Minimum Gap	1.0	2.2	0.5	0.5	0.5	2.2	0.5	0.5	0.0 - 25.5 sec
Max Variable Initial	4	15	4	6	4	15	4	6	0 - 255 sec
Auto Max Adjust	0.0	5.0	0.0	0.0	0.0	5.0	0.0	0.0	0.0 - 25.5 sec
Auto Max Limit	0	65	0	0	0	65	0	0	0 - 255 sec
Inhibit Min Yellow									X = On
Red Decimal Off									X = On
Advance Walk	0	0	0	0	0	0	0	0	0 - 255 sec
Other Controller Functions (next/2/2/9)									
Phase -->	1	2	3	4	5	6	7	8	
Inhibit Simultaneous Gap Out	X		X	X	X		X	X	X = On
Last Car Passage	2	0 = recall phase, 1 = last car passage, 2 = NOT recall - Not last car passage							
Red Revert (+2 seconds)	3.0	0 - 25.5 sec							
Auto Ped Clear	X	X = On							
Flashing Don't Walk Into Yellow		X = On							
Soft Recall / Red Rest Delay	0.0	0 - 25.5 sec							
Ped Pushbutton	0	0 - 5 sec, 0 = disable							
Advance Flash Rate	0	0 = disable, 1 = 120 FPM							
Change Sequence		X = On (After a download with a power on - off cycle)							
Phase -->	1	2	3	4	5	6	7	8	
Red Clear Extension Detector	0	0	0	0	0	0	0	0	0 = none 1 - 32 = detector 1 - 32
Red Clear Extension Red Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec.

Local Detectors (next/2/2/4)

Detector Data

ATTACHMENT 4

	Yellow Lock	Detector Inhibit	Call Phase	Extend Phase	Switch Phase	Delay Time	Stretch / Disconnect Time	Delay or Disconnect Mode
Detector 1 - 17			1	1	0	0	1.5	0
Detector 2 - 18,19			1	1	0	0	0.0	0
Detector 3 -			3	3	0	0	1.5	0
Detector 4 -			3	3	0	0	0.0	0
Detector 5 - 6			5	5	0	0	1.5	0
Detector 6 - 7,8			5	5	0	0	0.0	0
Detector 7 -			7	7	0	0	1.5	0
Detector 8 -			7	7	0	0	0.0	0
Detector 9 - 1			2	2	0	0	1.0	0
Detector 10 - 2			2	2	0	0	2.0	0
Detector 11 - 3,4			2	2	0	0	0.0	0
Detector 12 - 5			2	2	0	1	3.0	0
Detector 13 -			2	2	0	5	0.0	0
Detector 14 - 9			4	4	0	1	1.5	0
Detector 15 - 10,11			4	4	0	0	1.5	0
Detector 16 -			4	4	0	10	0.0	0
Detector 17 -			4	4	0	0	0.0	0
Detector 18 -			4	4	0	5	3.0	0
Detector 19 - 12			6	6	0	0	1.0	0
Detector 20 - 13			6	6	0	0	1.0	0
Detector 21 - 14,15			6	6	0	0	0.0	0
Detector 22 - 16			6	6	0	1	3.0	0
Detector 23 -			6	6	0	5	1.0	0
Detector 24 - 20			8	8	0	1	1.5	0
Detector 25 - 23			8	8	0	0	1.5	0
Detector 26 - 21,22			8	8	0	10	0.0	0
Detector 27 - 24,25			8	8	0	0	0.0	0
Detector 28 -			8	8	0	5	3.0	0
Detector 29 -			0	0	0	0	0.0	0
Detector 30 -			0	0	0	0	0.0	0
Detector 31 -			0	0	0	0	0.0	0
Detector 32 -			0	0	0	0	0.0	0

yellow lock, detector inhibit, - X = On; call, extend, phase - 0 = none 1 - 8 = phase 1 - 8; delay time - 0 - 255 sec
stretch / disconnect time - 0.0 - 25.5 sec.; delay or disconnect Mode - 0 - 13

Detector Plans (next/2/2/4/5)

Loop Number											
Plan Detectors		0	0	0	0	0	0	0	0	0 - 32, 0 = none, 1 - 3 2 = detectors 1 - 32	
Detector Plan 1	Call Phase	0	0	0	0	0	0	0	0	0 - 8, 0 = none, 1 - 8 = phase 1 - 8	
	Extend Phase	0	0	0	0	0	0	0	0		
	Switch Phase	0	0	0	0	0	0	0	0		
	Delay Time	0	0	0	0	0	0	0	0		0 - 255 sec
	Stretch/Disconnect Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0 - 25.5 sec
	Delay/ Disconnect Mode	0	0	0	0	0	0	0	0		0 - 13
Detector Plan 2	Call Phase	0	0	0	0	0	0	0	0	0 - 8, 0 = none, 1 - 8 = phase 1 - 8	
	Extend Phase	0	0	0	0	0	0	0	0		
	Switch Phase	0	0	0	0	0	0	0	0		
	Delay Time	0	0	0	0	0	0	0	0		0 - 255 sec
	Stretch/Disconnect Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0 - 25.5 sec
	Delay/ Disconnect Mode	0	0	0	0	0	0	0	0		0 - 13
Detector Plan 3	Call Phase	0	0	0	0	0	0	0	0	0 - 8, 0 = none, 1 - 8 = phase 1 - 8	
	Extend Phase	0	0	0	0	0	0	0	0		
	Switch Phase	0	0	0	0	0	0	0	0		
	Delay Time	0	0	0	0	0	0	0	0		0 - 255 sec
	Stretch/Disconnect Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0 - 25.5 sec
	Delay/ Disconnect Mode	0	0	0	0	0	0	0	0		0 - 13

Detector Fail Monitor (next/2/2/4/3)					Detectors 33-64 (next/2/2/4/6)					
	Fail Monitor Enable	Recall Phase	Min Counts	Max Counts	ATTACHMENT 4					
					Call Phase	Extend Phase				
Detector 1 - 17		0	0	0	Detector 33 -	0	0			
Detector 2 - 18,19		0	0	0	Detector 34 -	0	0			
Detector 3 -		0	0	0	Detector 35 - 7,8	0	0			
Detector 4 -		0	0	0	Detector 36 -	0	0			
Detector 5 - 6		0	0	0	Detector 37 -	0	0			
Detector 6 - 7,8		0	0	0	Detector 38 -	0	0			
Detector 7 -		0	0	0	Detector 39 -	0	0			
Detector 8 -		0	0	0	Detector 40 -	0	0			
Detector 9 - 1		0	0	0	Detector 41 -	0	0			
Detector 10 - 2		0	0	0	Detector 42 -	0	0			
Detector 11 - 3,4		0	0	0	Detector 43 -	0	0			
Detector 12 - 5		0	0	0	Detector 44 -	0	0			
Detector 13 -		0	0	0	Detector 45 -	0	0			
Detector 14 - 9		0	0	0	Detector 46 -	0	0			
Detector 15 - 10,11		0	0	0	Detector 47 -	0	0			
Detector 16 -		0	0	0	Detector 48 -	0	0			
Detector 17 -		0	0	0	Detector 49 -	0	0			
Detector 18 -		0	0	0	Detector 50 -	0	0			
Detector 19 - 12		0	0	0	Detector 51 -	0	0			
Detector 20 - 13		0	0	0	Detector 52 -	0	0			
Detector 21 - 14,15		0	0	0	Detector 53 -	0	0			
Detector 22 - 16		0	0	0	Detector 54 -	0	0			
Detector 23 -		0	0	0	Detector 55 -	0	0			
Detector 24 - 20		0	0	0	Detector 56 -	0	0			
Detector 25 - 23		0	0	0	Detector 57 -	0	0			
Detector 26 - 21,22		0	0	0	Detector 58 -	0	0			
Detector 27 - 24,25		0	0	0	Detector 59 -	0	0			
Detector 28 -		0	0	0	Detector 60 -	0	0			
Detector 29 -		0	0	0	Detector 61 -	0	0			
Detector 30 -		0	0	0	Detector 62 -	0	0			
Detector 31 -		0	0	0	Detector 63 -	0	0			
Detector 32 -		0	0	0	Detector 64 -	0	0			
fail monitor enable - X = On, recall phase - 0 = none 1 - 8 = phase 1 - 8, min, max					call / extend phase - 0 = none 1 - 8 = phase 1 - 8					
Detector Fail Sample Period (all detectors)			0		0 - 255 minutes					
Video Fail Inputs (next/2/2/4/3) -->			1	2	3	4	5	6	7	8
Phase Recalled			0	0	0	0	0	0	0	0
0 = none, 1 - 8 = phase 1 - 8										
System Detectors (next/2/2/4/4)										
System Detectors -->			1	2	3	4	5	6	7	8
Local Detector			0	0	0	0	0	0	0	0
0 = none, 1 - 32 = phase 1 - 32										

Overlaps / FYLTA (next/2/2/8)														
Vehicle Overlaps		Phase or Movement	Phases								Extension Green	Clearance		ATTACHMENT # 0 = none 1 = overlap 2 = 60 FPM 3 = Not ped 4=Comp. Ph. 5=Prevent. Ext. 6=Not Veh. 7=Adv. FF E - L 0 = no Overlap 1 = Overlap Green, Yellow Red
			1	2	3	4	5	6	7	8		Yellow	Red	
Overlaps	A		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	B		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	C		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	D		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	E		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	F		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	G		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	H		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	I		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	J		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	K		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	L		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
Not Ped - Ped Overlaps (next/2/2/8/5)														
Ped Overlaps -->		A	B	C	D	E	F	G	H					
Overlaps	A									X = Nor Ped Ped Overlap				
	B													
	C													
	D													
Advance Warning (next/2/2/8/3)														
			E	F	G	H	I	J	K	L				
Enable			0	0	0	0	0	0	0	0	0 = disabled, 1 = enabled			
1st Conditional Overlap			0	0	0	0	0	0	0	0	0 = none, 1 - overlap E, 2 = overlap F, etc.			
2nd Conditional Overlap			0	0	0	0	0	0	0	0				
Advance Deactivation Delay			0	0	0	0	0	0	0	0	0 - 99 seconds			
Ped Overlaps (next/2/2/8/5)														
Phase -->		1	2	3	4	5	6	7	8	Walk	Ped Clear	Ped Recall	Phase, Ped Recall: X = on Walk, Ped Clear: 0 - 255 seconds	
Ped Overlap	A									0	0			
	B									0	0			
	C									0	0			
	D									0	0			
	E									0	0			
	F									0	0			
	G									0	0			
	H									0	0			
Flashing Yellow Left Turn Arrow (FYLTA) (next/2/2/8/6)														
Phase Pairs -->		1 - 2	3 - 4	5 - 6	7 - 8									
Enable		0	0	0	0	0 = off, 3 = 3 outputs, 4 = 4 outputs, 5 = 5 outputs								
Even Omits Odd		0	0	0	0	0 = off, 1 = on, 2 = on, place call across barrier								
Detector Switch Odd / Even		X	X	X	X	X = on, odd phase must be omitted								
Red Transition		3.0	3.0	3.0	3.0	0.0 or 2.0 - 25.5 sec								
Red Extension		0.0	0.0	0.0	0.0	0.0 - 25.5 sec								
Return to GLTA		0	0	0	0	0 = off, 1 = max out, 2 = yellow lock								
Flashing Yellow Left Turn Arrow (FYLTA) - Continued on last page														

Service Plans (next/2/2/6)

ATTACHMENT 4

Phase -->		1	2	3	4	5	6	7	8		
Service Plan 1	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 2	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 3	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 4	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 5	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 6	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	

Service Plans Cont.

ATTACHMENT 4

Phase -->		1	2	3	4	5	6	7	8		
Service Plan 7	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	

Phase -->		1	2	3	4	5	6	7	8		
Service Plan 8	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	

Max Plans (next/2/2/7)

Phase -->		1	2	3	4	5	6	7	8	
Max Plan 1	Normal Max	25	60	30	30	25	60	30	30	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 2	Normal Max	25	60	25	25	25	60	25	25	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 3	Normal Max	25	70	30	30	25	70	30	30	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 4	Normal Max	25	75	30	25	30	75	30	25	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 5	Normal Max	0	0	0	0	0	0	0	0	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 6	Normal Max	0	0	0	0	0	0	0	0	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 7	Normal Max	0	0	0	0	0	0	0	0	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 8	Normal Max	0	0	0	0	0	0	0	0	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec

Coordination Data (next/2/3)

ATTACHMENT 4

Coordination Modes (next/2/3/1, next/2/3/4/1, next/2/3/4/3)

Flash Mode	33	0=off, 1=on, 33=time clock, 34=comm, 35=hardwire, 36=NWS Set only, 37=AB3418 / NTCIP S
Coordination Plan Mode	33	0=free, 1-32 = coord plan 1-32, 33=time clock, 34=comm, 35=hardwire, 36=NWS Set only, 37=
Offset Seeking Mode	2	0=add only, 1= dwell, 2=fastway
Late Ped	0	0 = off, 1 = on
Coord Walk Rest	0	0 = off, 1 = on, 2 = by TOD circuit 160, 3 = end of walk, 4 = coord ped during perms
Repeated Phase Service	0	0=off, 1=on (no coord ped), 2=on (beginning green coord ped), 3=on (coord ped always)
Zero Mode (TS2 only)	0	0=start of main street, 1=end of main street, 2=by TOD circuit 144

Phase -->	1	2	3	4	5	6	7	8	0 = service allowed 1 = service prevented
Omit Phase During Repeated Phase Service	0	0	0	0	0	0	0	0	
Auto Permissive Min Green	0	0	0	0	0	0	0	0	0 - 255 seconds

Coordination Plans (next/2/3/2)

Coord Plan	Coordination Phases		Cycle Length	Offset Time	Min Cycle Length Dwell Time	Permissive	Service Plan	Max Plan	
	Ring 1	Ring 2							
1-	2	6	100	74	0	0	0	1	
2-	2	6	100	81	0	0	0	2	
3-	2	6	110	95	0	0	0	3	
4-	2	6	120	65	0	0	0	4	
5-	2	6	0	0	0	0	0	0	
6-	2	6	100	74	0	0	0	6	
7-	2	6	0	0	0	0	0	0	
8-	0	0	0	0	0	0	0	0	
9-	0	0	0	0	0	0	0	0	
10-	0	0	0	0	0	0	0	0	
11-	0	0	0	0	0	0	0	0	
12-	0	0	0	0	0	0	0	0	
13-	0	0	0	0	0	0	0	0	
14-	0	0	0	0	0	0	0	0	
15-	0	0	0	0	0	0	0	0	
16-	0	0	0	0	0	0	0	0	
17-	0	0	0	0	0	0	0	0	
18-	0	0	0	0	0	0	0	0	
19-	0	0	0	0	0	0	0	0	
20-	0	0	0	0	0	0	0	0	
21-	0	0	0	0	0	0	0	0	
22-	0	0	0	0	0	0	0	0	
23-	0	0	0	0	0	0	0	0	
24-	0	0	0	0	0	0	0	0	
25-	0	0	0	0	0	0	0	0	
26-	0	0	0	0	0	0	0	0	
27-	0	0	0	0	0	0	0	0	
28-	0	0	0	0	0	0	0	0	
29-	0	0	0	0	0	0	0	0	
30-	0	0	0	0	0	0	0	0	
31-	0	0	0	0	0	0	0	0	
32-	0	0	0	0	0	0	0	0	
0 - 8			0 - 255 sec.				0 - 8		

Coordination Plans cont.

ATTACHMENT 4

Coord Plan	* = Force Offs / Split Times (TS2)								* = Yield Points / Actuated Times (TS2)	
	1	2	3	4	5	6	7	8	Ring 1	Ring 2
1-	18	42	20	20	18	42	20	20	5	18
2-	17	47	18	18	17	47	18	18	5	15
3-	18	50	22	20	18	50	22	20	5	18
4-	18	64	20	18	20	62	20	18	5	18
5-	0	0	0	0	0	0	0	0	0	0
6-	18	42	20	20	18	42	20	20	5	18
7-	0	0	0	0	0	0	0	0	0	0
8-	0	0	0	0	0	0	0	0	0	0
9-	0	0	0	0	0	0	0	0	0	0
10-	0	0	0	0	0	0	0	0	0	0
11-	0	0	0	0	0	0	0	0	0	0
12-	0	0	0	0	0	0	0	0	0	0
13-	0	0	0	0	0	0	0	0	0	0
14-	0	0	0	0	0	0	0	0	0	0
15-	0	0	0	0	0	0	0	0	0	0
16-	0	0	0	0	0	0	0	0	0	0
17-	0	0	0	0	0	0	0	0	0	0
18-	0	0	0	0	0	0	0	0	0	0
19-	0	0	0	0	0	0	0	0	0	0
20-	0	0	0	0	0	0	0	0	0	0
21-	0	0	0	0	0	0	0	0	0	0
22-	0	0	0	0	0	0	0	0	0	0
23-	0	0	0	0	0	0	0	0	0	0
24-	0	0	0	0	0	0	0	0	0	0
25-	0	0	0	0	0	0	0	0	0	0
26-	0	0	0	0	0	0	0	0	0	0
27-	0	0	0	0	0	0	0	0	0	0
28-	0	0	0	0	0	0	0	0	0	0
29-	0	0	0	0	0	0	0	0	0	0
30-	0	0	0	0	0	0	0	0	0	0
31-	0	0	0	0	0	0	0	0	0	0
32-	0	0	0	0	0	0	0	0	0	0
0 - 255 sec * = force offs and yield points										

Circuit Mapping (next/2/3/3)																	
Circuit Map	Coord Plan	Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		ATTACHMENT 4		Time Clock Circuit	
1	1	97	LG1	100	LG7	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
2	2	97	LG1	100	LG7	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
3	3	97	LG1	98	LG3	100	LG7	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
4	4	97	LG1	98	LG3	100	LG7	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
5	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
6	6	97	LG1	98	LG3	100	LG7	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
7	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
8	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
9	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
10	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
11	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
12	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
13	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
14	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
15	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
16	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
17	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
18	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
19	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
20	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U

coord plan - 0 = free, 1 - 32 = coord plan 1 - 32, 33 = any, 34 none selected
time clock circuits - 0 = not used, or circuits 6 - 196

Dynamic Phase Length (next/2/3/4/4)									
Phase -->	1	2	3	4	5	6	7	8	
Back Detector	1	29	3	14	5	30	7	25	0 = none, 1-32 = detector 1-32
Lane Factor	1.0	2.0	2.0	1.0	2.0	2.0	2.0	1.0	0 = none, 1.0 - 5.0
Check Out Detector	0	5	0	0	0	1	0	0	0 = none, 1-32 = detector 1-32
Coord Delta Force Off	Set A	5	5	5	5	5	5	5	0 - 255 sec
	Set B	0	0	0	0	0	0	0	
	Set C	0	0	0	0	0	0	0	
	Set D	0	0	0	0	0	0	0	
Free Delta Max	Set A	0	0	0	0	0	0	0	
	Set B	0	0	0	0	0	0	0	
	Set C	0	0	0	0	0	0	0	
	Set D	0	0	0	0	0	0	0	

Platoon Progression (next/2/3/4/5)					
Entry Local Only			Master Local Only		
Platoon Max	0	0 - 255 sec	Smoothing Factor	0.0	0.0 - 1.0
Min Platoon Green	0	0 - 255 sec			
Entry Detector Gap	0.0	0.0 - 25.5			
Min Platoon Cycle	0	0 - 255 sec			

Inbound			Outbound		
Only for Entry Inbound Local or Master Local			Only for Entry Outbound Local or Master Local		
Entry IB Local also Last OB Local	0	0 - 50	Entry OB Local also Last IB Local	0	0 - 50
Speed	0	0 - 55 mph	Speed	0	0 - 55 mph
Distance from Entry Local	0	0 - 65000 feet	Distance from Entry Local	0	0 - 65000 feet

Entry Local Only			Entry Local Only		
Distance from Entry Local Detector	0	0 - 999 feet	Distance from Entry Local Detector	0	0 - 999 feet
Entry Local Detector	0	0 - 32	Entry Local Detector	0	0 - 32

Master Local			Master Local		
Master Mid - System Critical Detectors	0	0 - 16	Master Mid - System Critical Detectors	0	0 - 16

Force Off Percents													
Inbound						Outbound							
	1	3	4	5	7	8		1	3	4	5	7	8
Split 1	0	0	0	0	0	0	Split 1	0	0	0	0	0	0
Split 2	0	0	0	0	0	0	Split 2	0	0	0	0	0	0
0 - 100 %						0 - 100 %							

Time of Day Data (next/2/4)

ATTACHMENT 4

Day Program (next/2/4/1)

	Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On / Off		Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On/Off
1	1	06:00	X	1			51				
2	1	07:30	X	2			52				
3	1	10:30	X	3			53				
4	1	16:00	X	4			54				
5	1	18:00	X	3			55				
6	1	20:00	X	0			56				
7	2	07:30	X	1			57				
8	2	10:00	X	6			58				
9	2	18:00	X	0			59				
10							60				
11							61				
12							62				
13							63				
14							64				
15							65				
16							66				
17							67				
18							68				
19							69				
20							70				
21							71				
22							72				
23							73				
24							74				
25							75				
26							76				
27							77				
28							78				
29							79				
30							80				
31							81				
32							82				
33							83				
34							84				
35							85				
36							86				
37							87				
38							88				
39							89				
40							90				
41							91				
42							92				
43							93				
44							94				
45							95				
46							96				
47							97				
48							98				
49							99				
50							100				
	1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on		1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on

Day Program cont.

	Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On / Off		Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On / Off
101							151				
102							152				
103							153				
104							154				
105							155				
106							156				
107							157				
108							158				
109							159				
110							160				
111							161				
112							162				
113							163				
114							164				
115							165				
116							166				
117							167				
118							168				
119							169				
120							170				
121							171				
122							172				
123							173				
124							174				
125							175				
126							176				
127							177				
128							178				
129							179				
130							180				
131							181				
132							182				
133							183				
134							184				
135							185				
136							186				
137							187				
138							188				
139							189				
140							190				
141							191				
142							192				
143							193				
144							194				
145							195				
146							196				
147							197				
148							198				
149							199				
150							200				
	1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on		1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on

Week Program (next/2/4/2)								Year Program (next/2/4/3)			
	Sun	Mon	Tue	Wed	Thu	Fri	Sat	From Date	To Date	Week Program	ATTACHMENT 4
1	2	1	1	1	1	1	2	01/01/2014	12/31/2014	1	
2	2	1	1	1	1	1	2				
3	2	1	1	1	1	1	2				
4	2	1	1	1	1	1	2				
5	2	1	1	1	1	1	2				
6	2	1	1	1	1	1	2				
7	2	1	1	1	1	1	2				
8	2	1	1	1	1	1	2				
9	2	1	1	1	1	1	2				
10	2	1	1	1	1	1	2				
0 = none, 1 - 15 = day plan											
Exception Days (next/2/4/6)											
	DOW	WOM	DOM	MOY	Day Prog.						
1											
2											New Years Day - Date - January 1st
3											
4											Martin Luther King Day - DOW WOM - 3rd Monday of January
5											
6											President's Day - DOW WOM - 3rd Monday February
7											
8											
9											Memorial Day - DOW WOM - Last Monday May
10											
11											Fourth of July - Date - July 4th
12											
13											Labor Day - DOW WOM - 1st Monday September
14											
15											Columbus Day - DOW WOM - 2nd Monday October
16											
17											
18											Veteran's Day - Date - November 11th
19											
20											Thanksgiving - DOW WOM - 4th Thursday November
21											
22											
23											Christmas - Date - December 25th
24											
25											
26											
27											
28											
29											
30											
31											
32											
33											
34											
35											
	0-10	0-5	0-31	0-12	0-15						
Time Clock References (next/2/4/5)											
Synch reference Mode		<i>0</i>		0 = timed, 1 = by event		Exception day headings - DOW = Day of Week, WOM = Week of Month, DOM = Day of Month, MOY = Month of Year					
Synch Reference Time		<i>00:00</i>		00:00 - 23:59							
Daylight Savings Enable		<i>X</i>		X = on							
Reset Time		<i>00:00</i>		00:00 - 23:59							

Circuit Overrides (next/2/4/4)

			ATTACHMENT	
1 - Coord Line 1	CL1	TOD		51 - Ped Omit 3
2 - Coord Line 2	CL2	TOD	PO4	TOD
3 - Coord Line 4	CL4	TOD	PO5	TOD
4 - Coord Line 8	CL8	TOD	PO6	TOD
5 - Coord Line 16	C16	TOD	PO7	TOD
6 - Coord Operation	CRD	TOD	PO8	TOD
7 - Soft Flash	SFL	TOD	CVS	TOD
8 - Enable System Relays	ESR	TOD	ISG	TOD
9 - Call to Non Act 1	CN1	TOD	HWI	TOD
10 - Call to Non Act 2	CN2	TOD	POM	On
11 - Walk Rest Modifier	WRM	TOD	DLE	On
12 - Min Recall	MIN	TOD	EPD	TOD
13 - Max 2 Both Rings	MX2	TOD	CTC	TOD
14 - Coord Inhibit Max Ring 1, 2	IMT	TOD	DEP	TOD
15 - Enable Service Log	ESL	TOD	SP1	TOD
16 - Call to Free	CTF	TOD	SP2	TOD
17 - TOD Output 1	TO1	TOD	SP3	TOD
18 - TOD Output 2	TO2	TOD	SP4	TOD
19 - TOD Output 3	TO3	TOD	SP5	TOD
20 - TOD Output 4	TO4	TOD	SP6	TOD
21 - TOD Output 5	TO5	TOD	SP7	TOD
22 - TOD Output 6	TO6	TOD	SP8	TOD
23 - TOD Output 7	TO7	TOD	MP1	TOD
24 - TOD Output 8	TO8	TOD	MP2	TOD
25 - Vehicle Call Phase 1	VC1	TOD	MP3	TOD
26 - Vehicle Call Phase 2	VC2	TOD	MP4	TOD
27 - Vehicle Call Phase 3	VC3	TOD	MP5	TOD
28 - Vehicle Call Phase 4	VC4	TOD	MP6	TOD
29 - Vehicle Call Phase 5	VC5	TOD	MP7	TOD
30 - Vehicle Call Phase 6	VC6	TOD	MP8	TOD
31 - Vehicle Call Phase 7	VC7	TOD	TG1	TOD
32 - Vehicle Call Phase 8	VC8	TOD	TG2	TOD
33 - Ped Call Phase 1	PC1	TOD	TG3	TOD
34 - Ped Call Phase 2	PC2	TOD	TG4	TOD
35 - Ped Call Phase 3	PC3	TOD	TG5	TOD
36 - Ped Call Phase 4	PC4	TOD	TG6	TOD
37 - Ped Call Phase 5	PC5	TOD	TG7	TOD
38 - Ped Call Phase 6	PC6	TOD	TG8	TOD
39 - Ped Call Phase 7	PC7	TOD	IV1	TOD
40 - Ped Call Phase 8	PC8	TOD	IV2	TOD
41 - Vehicle Omit 1	VO1	TOD	IV3	TOD
42 - Vehicle Omit 2	VO2	TOD	IV4	TOD
43 - Vehicle Omit 3	VO3	TOD	IV5	TOD
44 - Vehicle Omit 4	VO4	TOD	IV6	TOD
45 - Vehicle Omit 5	VO5	TOD	IV7	TOD
46 - Vehicle Omit 6	VO6	TOD	IV8	TOD
47 - Vehicle Omit 7	VO7	TOD	LG1	TOD
48 - Vehicle Omit 8	VO8	TOD	LG3	TOD
49 - Ped Omit 1	PO1	TOD	LG5	TOD
50 - Ped Omit 2	PO2	TOD	LG7	TOD

On /
Off /
TOD

On /
Off /
TOD

Circuit Overrides cont.

				ATTACHMENT	
101 - Inhibit Overlap A	OLA	TOD			
102 - Inhibit Overlap B	OLB	TOD			
103 - Inhibit Overlap C	OLC	TOD			
104 - Inhibit Overlap D	OLD	TOD			
105 - Enable Schedule A Phone 1	AT1	TOD			
106 - Enable Schedule A Phone 2	AT2	TOD			
107 - Enable Schedule B Phone 1	BT1	TOD			
108 - Enable Schedule B Phone 2	BT2	TOD			
109 - Enable Schedule C Phone 1	CT1	TOD			
110 - Enable Schedule C Phone 2	CT2	TOD			
111 - Enable Volume to Call Phone 1	VT1	TOD			
112 - Enable Volume to Call Phone 2	VT2	TOD			
113 - Enable Volume Logging	EVL	On			
114 - Enable MOE Logging	EML	On			
115 - Detector Low Threshold Inhibit	DLI	TOD			
116 - Detector Continue Presence Inhibit	DPI	TOD			
117 - Inhibit Detector Based on Programming	IND	TOD			
118 - Inhibit Detector Delay	IDD	TOD			
119 - Inhibit Conditional Ped	ICP	TOD			
120 - Inhibit Transit Priority	ITP	TOD			
121 - Red Rest Ring 1,2	RRM	TOD			
122 - Enable Transcend	TRA	TOD			
123 - Omit Red Clear Ring 1,2	ORC	TOD			
124 - Not Used	N/U	TOD			
125 - Ped Recycle Ring 1,2	PCY	TOD	On /		On /
126 - Not Used	N/U	TOD	Off /		Off /
127 - Enable MOE Log to Call Phone 1	MT1	TOD	TOD		TOD
128 - Enable MOE Log to Call Phone 2	MT2	TOD			
129 - Transit Inhibit Short Time 1	IS1	TOD			
130 - Transit Inhibit Short Time 2	IS2	TOD			
131 - Transit Inhibit Short Time 3	IS3	TOD			
132 - Transit Inhibit Short Time 4	IS4	TOD			
133 - Transit Inhibit Short Time 5	IS5	TOD			
134 - Transit Inhibit Short Time 6	IS6	TOD			
135 - Transit Inhibit Short Time 7	IS7	TOD			
136 - Transit Inhibit Short Time 8	IS8	TOD			
137 - Enable Transit Priority Logging	ETL	TOD			
138 - Disable Flashing Yellow Arrow 1	DF1	TOD			
139 - Disable Flashing Yellow Arrow 3	DF3	TOD			
140 - Disable Flashing Yellow Arrow 5	DF5	TOD			
141 - Disable Flashing Yellow Arrow 7	DF7	TOD			
142 - Disable Auto Max	DAM	TOD			
143 - Disable Repeat Phase Service	DRS	TOD			
144 - Coord End of Main Street	EMS	TOD			
145 - Coord Hold 1	HD1	TOD			
146 - Coord Hold 2	HD2	TOD			
147 - Coord Hold 3	HD3	TOD			
148 - Coord Hold 4	HD4	TOD			
149 - Coord Hold 5	HD5	TOD			
150 - Coord Hold 6	HD6	TOD			
151 - Coord Hold 7					
152 - Coord Hold 8					
153 - PE Priority Return B					
154 - PE Priority Return C					
155 - PE Priority Return D					
156 - PE Priority Return E					
157 - Platoon Inbound					
158 - Platoon Outbound					
159 - Platoon Spl 2					
160 - Coord Walk Rest					
161 - Dynamic Phase Length Short Inhibit 1					
162 - Dynamic Phase Length Short Inhibit 2					
163 - Dynamic Phase Length Short Inhibit 3					
164 - Dynamic Phase Length Short Inhibit 4					
165 - Dynamic Phase Length Short Inhibit 5					
166 - Dynamic Phase Length Short Inhibit 6					
167 - Dynamic Phase Length Short Inhibit 7					
168 - Dynamic Phase Length Short Inhibit 8					
169 - Coord Late Left Turn 1					
170 - Coord Late Left Turn 3					
171 - Coord Late Left Turn 5					
172 - Coord Late Left Turn 7					
173 - Dynamic Phase Length Enable A					
174 - Dynamic Phase Length Enable B					
175 - Dynamic Phase Length Enable C					
176 - Dynamic Phase Length Enable D					
177 - Proactive Plan Select Average					
178 - Proactive Plan Select Inbound					
179 - Proactive Plan Select Outbound					
180 - Split Variant Inbound					
181 - Split Variant Outbound					
182 - Disable Coord Walk Rest Ring 1					
183 - Disable Coord Walk Rest Ring 2					
184 - Proactive Plan Select New Look					
185 - Disable Red Clearance Extension					
186 - Detector Plan Line 1					
187 - Detector Plan Line 2					
188 - Disable LRT 1 Vertical Flashing Bar					
189 - Disable LRT 2 Vertical Flashing Bar					
190 - Disable LRT 3 Vertical Flashing Bar					
191 - Disable LRT 4 Vertical Flashing Bar					
192 - Datakey Enable					
193 - Dynamic Phase Reversal Enable 1					
194 - Dynamic Phase Reversal Enable 3					
195 - Dynamic Phase Reversal Enable 5					
196 - Dynamic Phase Reversal Enable 7					
197 - Enable Coord Logging					
198 - Disable Gap FYLTA 1,3,5,7					
199 - Coordination Auto Walk					
200 - Enable Coordinated Auto Max					

Preemption Data (next/2/5)

ATTACHMENT 4

Sequence (next/2/5/1 - 8)							Instructions 0 - Service Phases 1-9 = Special Interval 1-9 10 - Preempt Sequence Allows FYLTA 11 - Preempt Interval Disables FYLTA 15 - Alternate Trap Protection 90 - Go to all Red 91 - Soft Flash On 92 - Soft Flash Off 93 - Enable Ped 94 - Disable Peds 95 - Priority Return 96 - Enable Coordination with peds 97 - Enable Coordination without peds 98 - Return with NO Calls 99 - Return with Vehicle Calls 100 - jump to step in Interval Time 101 - Use Interval Time as Resettable Gap Timer 196 - Coord Re-synch with Peds 197 - Coord Re-synch without Peds 200 - Light Rail Train phase without Peds 201 - Light Rail Train phase with Peds 202 - Return to highest queue/delay phase (this uses the Dynamic Phase Length Back Detectors) 216 - Light Rail Train Coord Re-synch with Peds 217 - Light Rail Train Coord Re-synch without Peds
Sequences / Intervals	Instruction	Phases Serviced	Interval Time	Hold On Input	Outputs On	Output Mode	
1	1	197	25	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
2	1	197	47	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
3	1	197	16	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
4	1	197	38	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
5	1	0		0	0	0	
	2	0		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	

Sequence cont.

ATTACHMENT 4

Sequences / Intervals	Instruction	Phases Serviced	Interval Time	Hold On Input	Outputs On	Output Mode
6	1	0	0	0		0
	2	0	0	0		0
	3	0	0	0		0
	4	0	0	0		0
	5	0	0	0		0
	6	0	0	0		0
	7	0	0	0		0
	8	0	0	0		0
	9	0	0	0		0
	10	0	0	0		0
7	1	0	0	0		0
	2	0	0	0		0
	3	0	0	0		0
	4	0	0	0		0
	5	0	0	0		0
	6	0	0	0		0
	7	0	0	0		0
	8	0	0	0		0
	9	0	0	0		0
	10	0	0	0		0
8	1	0	0	0		0
	2	0	0	0		0
	3	0	0	0		0
	4	0	0	0		0
	5	0	0	0		0
	6	0	0	0		0
	7	0	0	0		0
	8	0	0	0		0
	9	0	0	0		0
	10	0	0	0		0

Sequence Timing (next/2/5/0)

Sequence -->	1	2	3	4	5	6	7	8	
Input Memory									X = on
Input Priority	6	6	6	6	0	0	0	0	0 = lowest, - 8 = highest
Min Green	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec
Walk	0.1	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.0 would time the normal function time
Ped Clear	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Overlap Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec
Overlap Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Delay to Preempt	0	0	0	0	0	0	0	0	
Delay Ped Omit	0	0	0	0	0	0	0	0	0 - 255 sec
Delay Phase Omit	0	0	0	0	0	0	0	0	
Min Reservice	0	0	0	0	0	0	0	0	0 - 255 min
Overlap Inhibits	A								X = inhibit
	B								
	C								
	D								
Exit Parameters	Exit to Coord Plan Offset by X	0	0	0	0	0	0	0	0 - 20
	Exit Coord Plan Time	0	0	0	0	0	0	0	0 - 60 min
	Exit to Max Plan	0	0	0	0	0	0	0	0 - 8
	Exit Free Time	0	0	0	0	0	0	0	0 - 60 min
	Override Time	0	0	0	0	0	0	0	
	Fail Time	0	0	0	0	0	0	0	
Exit Mode Time	0	0	0	0	0	0	0	0	

Priority Return and Special Intervals (next/2/5/0/6, next/2/5/9)														
Phase / Overlap -->		1	2	3	4	5	6	7	8	A	B	C	D	ATTACHMENT 4
Priority Return	Enable	0	0 = disabled, 1 = enabled, 2 = enabled, skip preemption phases on exit											
	A (max)	0	0	0	0	0	0	0	0	0 - 100% of currently used max				
	B (max)	0	0	0	0	0	0	0	0					
	C (max)	0	0	0	0	0	0	0	0					
	D (max)	0	0	0	0	0	0	0	0					
	E (max)	0	0	0	0	0	0	0	0					
Ped Clear	0	0	0	0	0	0	0	0	0	0 - 100% of currently used ped clearance				
Queue Delay Recovery	0	0	0	0	0	0	0	0	0	0 - 255 sec.				
Special Intervals	1	0	0	0	0	0	0	0	0	0	0	0	0	0 = Dark 1 = green don't walk 2 = green walk 3 = green flashing don't walk 4 = yellow 5 = red 6 = flashing yellow WIG 7 = flashing yellow WAG 8 = flashing red WIG 9 = flashing red WAG 10 = walk only 11=flashing don't walk only
	2	0	0	0	0	0	0	0	0	0	0	0	0	
	3	0	0	0	0	0	0	0	0	0	0	0	0	
	4	0	0	0	0	0	0	0	0	0	0	0	0	
	5	0	0	0	0	0	0	0	0	0	0	0	0	
	6	0	0	0	0	0	0	0	0	0	0	0	0	
	7	0	0	0	0	0	0	0	0	0	0	0	0	
	8	0	0	0	0	0	0	0	0	0	0	0	0	
	9	0	0	0	0	0	0	0	0	0	0	0	0	
Light Rail Train (next/2/5/0/7)														
Light Rail Train -->		1	2	3	4									
Associated Preempt		0	0	0	0	0 = none, preempt 1 - 8								
Time to Green		0	0	0	0	0 - 255 sec								
Horizontal Bar Flash Time		0.0	0.0	0.0	0.0	0.0 - 25.5 sec								
Vertical Bar Flash Time		0.0	0.0	0.0	0.0	0 - 255 sec								
Min Duration		0	0	0	0	0 - 255 sec								

Communications Data (next/2/6)

ATTACHMENT 4

1st Central Phone Number				2nd Central Phone Number			
Modem Setup String				Intersection Name		<i>Hwy 99w @ Springbrook Rd</i>	
Subnet Mask		<i>0.0.0.0</i>					
IP (ethernet) Port		<i>0</i>					
Central Port		<i>0</i>					
System Mode		<i>0</i>					
System Port		<i>1</i>		Alternate System Port		<i>0</i>	
System ID	<i>64</i>	AB3418e Physical Address		<i>1</i>	IP Address		<i>0.0.0.0</i>
Local ID	<i>12</i>	AB3418e Group Address		<i>0</i>	Gateway Address		<i>0.0.0.0</i>
Baud Rates		Flow Control		Port Use			
Port 1 (Slot A2 Upper)		<i>2</i>		<i>1</i>		<i>Suggested Use - FSK</i>	
Port 2 (Slot A2 Lower)		<i>2</i>		<i>1</i>		<i>Suggested Use - Not Used</i>	
Port 3 (Slot A1 Upper)		<i>0</i>		<i>0</i>		<i>Suggested Use - Modem to Central</i>	
Port 4 (Slot A1 Lower or C50S)		<i>2</i>		<i>NU</i>		<i>Suggested Use - RS232 to Laptop</i>	
0 = 1200, 1 = 2400, 2 = 9600, 3 = 19200 baud				0 = off, 1 = on			
Reports							
Volume Log Period		<i>15</i>	minute	Volume/Occ Log Period		<i>0</i>	second
				MOE Log Period		<i>15</i>	minute
0 = disabled, 1,2,3,4,5,6,10,12,15,20,30,60 minutes							
Function Schedule Mapping (next/2/6/7)							
Alarm 1		<i>0</i>		Soft Flash		<i>1</i>	
Alarm 2		<i>0</i>		Manual Control Enable (MCE)		<i>4</i>	
Alarm 3		<i>0</i>		Emergency or Railroad Preempt		<i>1</i>	
Alarm 4		<i>0</i>		Not Used		<i>0</i>	
Alarm 5		<i>0</i>		Cycle Failure		<i>2</i>	
Not Used		<i>0</i>		Coordination Failure		<i>2</i>	
Not Used		<i>0</i>		Keyboard use / Data Changed		<i>3</i>	
Not Used		<i>0</i>		Coord Running / Free		<i>2</i>	
Power On / Off		<i>1</i>		Cabinet Door		<i>3</i>	
Checksum Failure		<i>4</i>		Extended Ped Pushbutton		<i>0</i>	
Video / Detector Failure		<i>4</i>		Monitor Status		<i>4</i>	
Master to Local Comm Lost		<i>0</i>		Red Extension		<i>0</i>	

Miscellaneous Data

ATTACHMENT 4

Transit Priority (next/2/7)									
	1	2	3	4	5	6	7	8	
Phases									Phases 1 - 8 (max of 2 compatible phases)
PE Enable (6.25Hz TP call on PE)									X = 6.25 Hz signal will activate TP
Priority	0	0	0	0	0	0	0	0	0 - 8, 8 = highest
Memory									X = on
Delay Time	0	0	0	0	0	0	0	0	0 - 255 sec
Minimum Reservice Time (per input)	0	0	0	0	0	0	0	0	0 - 255 min
Override Time	0	0	0	0	0	0	0	0	0 - 255 sec
Bus Extend	0	0	0	0	0	0	0	0	0 - 255 sec
Minimum Reservice Time (all inputs)	0	0 - 255 min							
Free Operation Mode	0	0 = use shortest of max 1 or 2, 1 - 8 = use max time of group 1 - 8, 9 = use time of day							

Transit Priority Alternate Force Off Plans									
Current Coord Plan	1	2	3	4	5	6	7	8	
Alternate TP Force Off Plan	0	0	0	0	0	0	0	0	0 = none 17 - 32 = coord plan 17 - 32
Current Coord Plan	9	10	11	12	13	14	15	16	
Alternate TP Force Off Plan	0	0	0	0	0	0	0	0	

Group Timing									
Phase -->	1	2	3	4	5	6	7	8	
Group 1	Max Times	0	0	0	0	0	0	0	0 - 255 sec 0 would time the normal function time
	Walk Times	0	0	0	0	0	0	0	
Group 2	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 3	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 4	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 5	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 6	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 7	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 8	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	

Truck Priority (next/2/7/9)					
Truck Priority-->	1	2	3	4	
Associated Transit Priority	0	0	0	0	0 = none 1 - 8 = transit priority 1 - 8
Leading Detector	0	0	0	0	0 = none, 1 - 32 = detector 1 - 32
Trailing Detector	0	0	0	0	
Stop Bar Distance	0	0	0	0	0 - 999 feet
Trap Distance	0	0	0	0	0.0 - 99.9 feet
Minimum Speed	0	0	0	0	0 - 100 mph
Minimum Length	0	0	0	0	0 - 255 feet
Downhill Grade	0	0	0	0	0 - 20 %
Uphill Grade	0	0	0	0	
Undersized Vehicle					X = Enabled

Change I/O	X = On (After a download with a power on - off cycle)
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Inputs (Non Default I/O is offset to the right) (next/2/8/1)											
C1-39	101	VD9	C1-55	15	VD5	C1-67	22	PED2	C1-15	254	N/U
C1-40	113	VD19	C1-56	11	VD1	C1-68	26	PED6	C11-16	254	N/U
C1-41	106	VD14	C1-57	17	VD7	C1-69	24	PED4	C11-17	254	N/U
C1-42	118	VD24	C1-58	13	VD3	C1-70	28	PED8	C11-18	254	N/U
C1-43	102	VD10	C1-59	16	VD6	C1-71	151	PE1	C11-19	254	N/U
C1-44	114	VD20	C1-60	12	VD2	C1-72	152	PE2	C11-20	254	N/U
C1-45	107	VD15	C1-61	18	VD8	C1-73	153	PE3	C11-21	254	N/U
C1-46	161	VD25	C1-62	14	VD4	C1-74	154	PE4	C11-22	254	N/U
C1-47	105	VD13	C11-10	254	N/U	C1-75	254	N/U	C11-23	254	N/U
C1-48	117	VD23	C11-11	254	N/U	C1-76	104	VD12	C11-24	254	N/U
C1-49	112	VD18	C11-12	254	N/U	C1-77	116	VD22	C11-25	254	N/U
C1-50	164	VD28	C11-13	254	N/U	C1-78	111	VD17	C11-26	254	N/U
C1-51	199	PEDI	C1-63	103	VD11	C1-79	163	VD27	C11-27	254	N/U
C1-52	155	PE5	C1-64	115	VD21	C1-80	82	IADV	C11-28	254	N/U
C1-53	85	MCE	C1-65	108	VD16	C1-81	137	MONS	C11-29	254	N/U
C1-54	254	N/U	C1-66	162	VD26	C1-82	62	ST1	C11-30	254	N/U

Outputs (Non Default I/O is offset to the right) (next/2/8/2)											
C1-2	44	4DWK	C1-19	48	8DWK	C1-35	131	TO1	C1-91	41	1DWK
C1-3	64	4WLK	C1-20	68	8WLK	C1-36	132	TO2	C1-93	61	1WLK
C1-4	14	4RED	C1-21	18	8RED	C1-37	133	TO3	C1-94	106	OLBR
C1-5	24	4YEL	C1-22	28	8YEL	C1-38	134	TO4	C1-95	105	OLBY
C1-6	34	4GRN	C1-23	38	8GRN	C1-100	53	3PCL	C1-96	104	OLBG
C1-7	13	3RED	C1-24	17	7RED	C1-101	51	1PCL	C1-97	103	OLAR
C1-8	23	3YEL	C1-25	27	7YEL	C1-102	187	SFL	C1-98	102	OLAY
C1-9	33	3GRN	C1-26	37	7GRN	C1-103	147	WDOG	C1-99	101	OLAG
C1-10	42	2DWK	C1-27	46	6DWK	C1-83	43	3DWK	C11-1	254	N/U
C1-11	62	2WLK	C1-28	66	6WLK	C1-84	63	3WLK	C11-2	254	N/U
C1-12	12	2RED	C1-29	16	6RED	C1-85	116	OLDR	C11-3	254	N/U
C1-13	22	2YEL	C1-30	26	6YEL	C1-86	115	OLDY	C11-4	254	N/U
C1-15	32	2GRN	C1-31	36	6GRN	C1-87	114	OLDG	C11-5	254	N/U
C1-16	11	1RED	C1-32	15	5RED	C1-88	113	OLCR	C11-6	254	N/U
C1-17	21	1YEL	C1-33	25	5YEL	C1-89	112	OLCY	C11-7	254	N/U
C1-18	31	1GRN	C1-34	35	5GRN	C1-90	111	OLCG	C11-8	254	N/U

Internal Logic (next/2/9)			
Step	Inst.	Description	Comment ATTACHMENT 4
1	201	Jumper Two Inputs	
2	101	Vehicle Detector 9	
3	165	Vehicle Detector 29	
4	201	Jumper Two Inputs	
5	102	Vehicle Detector 10	
6	165	Vehicle Detector 29	
7	201	Jumper Two Inputs	
8	113	Vehicle Detector 19	
9	166	Vehicle Detector 30	
10	201	Jumper Two Inputs	
11	114	Vehicle Detector 20	
12	166	Vehicle Detector 30	
13			
14			
15			
16			
17			
18			
19			
20			
21			
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Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
56			
57			
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66			
67			
68			
69			
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108			
109			
110			

Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
111			
112			
113			
114			
115			
116			
117			
118			
119			
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121			
122			
123			
124			
125			
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164			
165			

Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
166			
167			
168			
169			
170			
171			
172			
173			
174			
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176			
177			
178			
179			
180			
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217			
218			
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220			

Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
221			
222			
223			
224			
225			
226			
227			
228			
229			
230			
231			
232			
233			
234			
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253			
254			
255			
256			

FYLTA - Continued (next/2/2/8/6)

Phase Pairs -->		1 - 2	3 - 4	5 - 6	7 - 8	
Gap-Dependent FYLTA (next/2/2/8/6-A)	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	0	0	0	0 - 255 sec
	Not Ped	0	0	0	0	0 - 255 sec

FYLTA Gap-Dependent Plans (next/2/2/8/6)

Phase Pairs -->		1 - 2	3 - 4	5 - 6	7 - 8	
FYLTA Gap-Dependent Plan A	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	0	0	0	0 - 255 sec
	Not Ped	0	0	0	0	0 - 255 sec
FYLTA Gap-Dependent Plan B	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	0	0	0	0 - 255 sec
	Not Ped	0	0	0	0	0 - 255 sec
FYLTA Gap-Dependent Plan C	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	311/690	0	0	0 - 255 sec

	Not Ped	0	0	0	0	0 - 255 sec	
FYLTA Gap-Dependent Plan D	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors	ATTACHMENT 4
	Min Delay	0	0	0	0	0 - 255 sec	
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec	
	Max Delay	0	0	0	0	0 - 255 sec	
	Not Ped	0	0	0	0	0 - 255 sec	

Preemption - Continued

Railroad Communications (IEEE 1570) (next/2/5/0/8)

	ATC	Wayside	
Railroad Number	0	0	0 - 999, represents railroad
Railroad Line Number	0	0	0 - 999, represents railroad line
Group Number	0	0	0 - 999, represents physical group of equipment
Subnode Number	0	0	0 - 99, subnode within physical group of equipment
Device Number	0	0	0 - 99, device within physical group of equipment

Associated Preempt 0 0 - 8

Communication Port 0 0 - 4

Reports - Continued

Reports - Service Delay Modes (next/2/6/0)

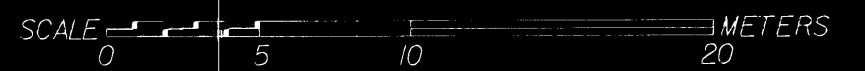
Phase -->	1	2	3	4	5	6	7	8	
Mode	0	0	0	0	0	0	0	0	0 = disable, 1 = enable, 2 = Ped, 3 = Veh/P

Ped Overlap -->	A	B	C	D	E	F	G	H	
Mode	0	0	0	0	0	0	0	0	0 = disable, 1 = enable

Detector -->	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Detector -->	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Detector -->	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Detector -->	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	



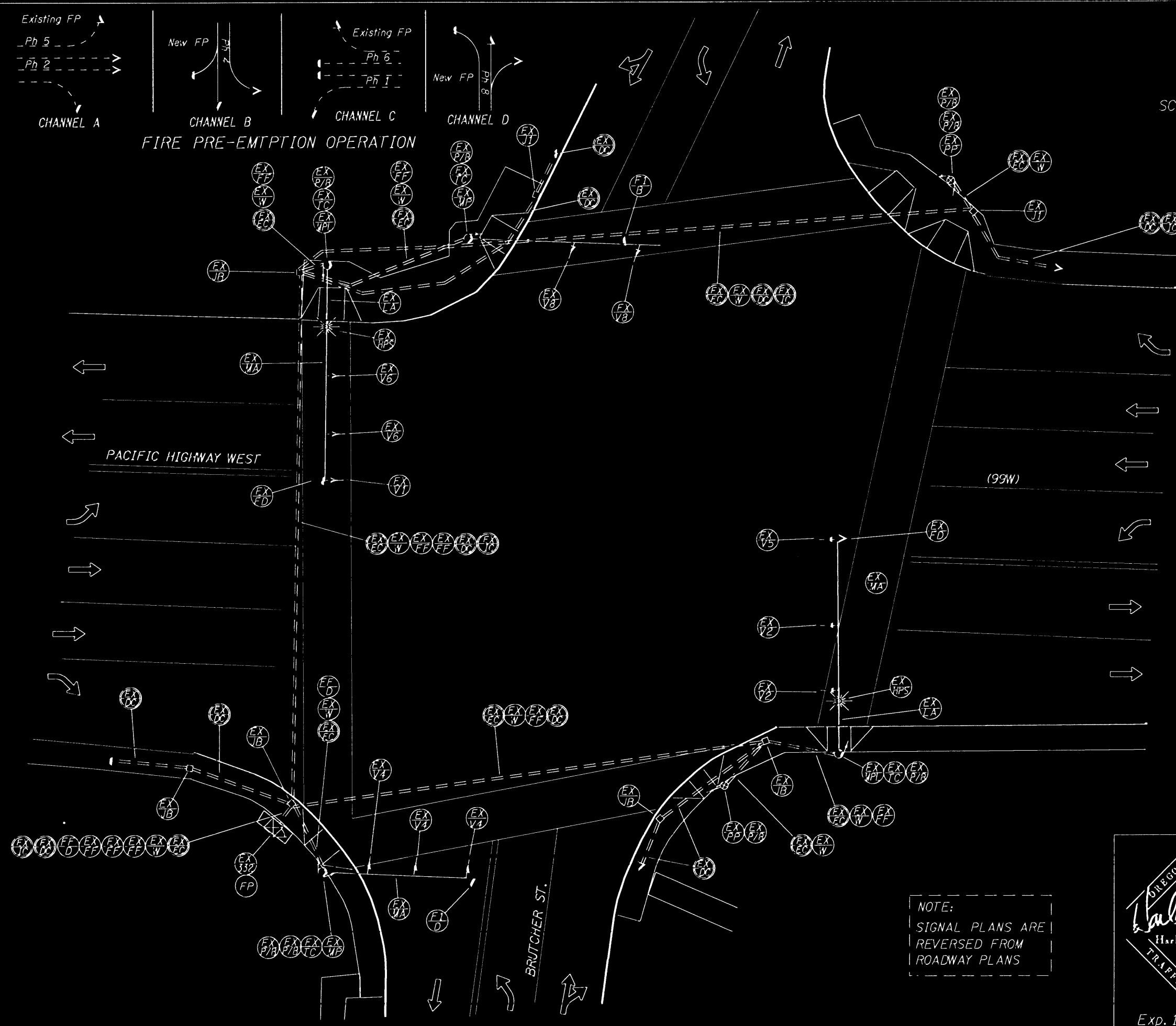
SIGNAL MODIFICATION PLAN PACIFIC HWY W AT BRUTCHER ST. HWY. 1W MP. 21.81



LEGEND

- (EX C) Retain and protect existing Model 170 controller and Model 332 cabinet
- (EX JP) Retain and protect existing traffic signal mast arm pole
- (EX JPL) Retain and protect existing traffic signal mast arm pole with luminaire extension
- (EX PP) Retain and protect existing pedestrian signal pedestal
- (EX MA) Retain and protect existing traffic signal mast arm
- (EX LA) Retain and protect existing traffic signal luminaire arm
- (EX VPh) Retain and protect existing phase (Ph) vehicle signal
- (EX P/B) Retain and protect existing pedestrian signal, pushbutton and instructions
- (EX TC) Retain and protect existing terminal cabinet
- (EX JB) Retain and protect existing junction box
- (EX JI) Retain and protect existing 440 mm x 265 mm x 305 mm precast concrete junction box
- (EX W) Retain and protect existing wiring
- (EX FF) Retain and protect existing fire pre-emption feeder cable
- (EX FD) Retain and protect existing fire pre-emption detector unit
- (EX EC) Retain and protect existing electrical conduit
- (EX DC) Retain and protect existing detector conduit
- (EX IC) Retain and protect existing interconnect conduit
- (EX LPS) Retain and protect existing luminaire
- (EX Ch) Install channel (Ch), (N) barrel fire pre-emption detector unit
- (EX Ch) Install channel (Ch) fire pre-emption detector feeder cable
- (FP) Install fire pre-emption device in controller

Ph = Phase shown
S = Size shown
Ch = Channel shown
N = Number shown



NOTE:
SIGNAL PLANS ARE
REVERSED FROM
ROADWAY PLANS

OREGON PROFESSIONAL
PTE 015
Harlan L. Naegeli
Harlan L. Naegeli
Mar. 16
1988
TRAFFIC ENGINEER

OREGON DEPARTMENT OF TRANSPORTATION
TRAFFIC MANAGEMENT SECTION
TRAFFIC SIGNAL INSTALLATION
BRUTCHER ST. - EVEREST ST. &
EVEREST ST. - MAIN ST. (NEWBERG) SECS.
PACIFIC HIGHWAY WEST
YAMHILL COUNTY

DATE: November 2000
DESIGNED BY: H. Naegeli
CHECKED BY: T. Jenkins
DRAWN BY: H. Naegeli

Exp. 12/31/2002
A6 070221r 3.sg1 T.M.S. DRG. NO. 12284

DETECTOR REPLACEMENT PLAN
 PACIFIC HWY W AT BRUTCHER ST.
 HWY. 1W MP. 21.81



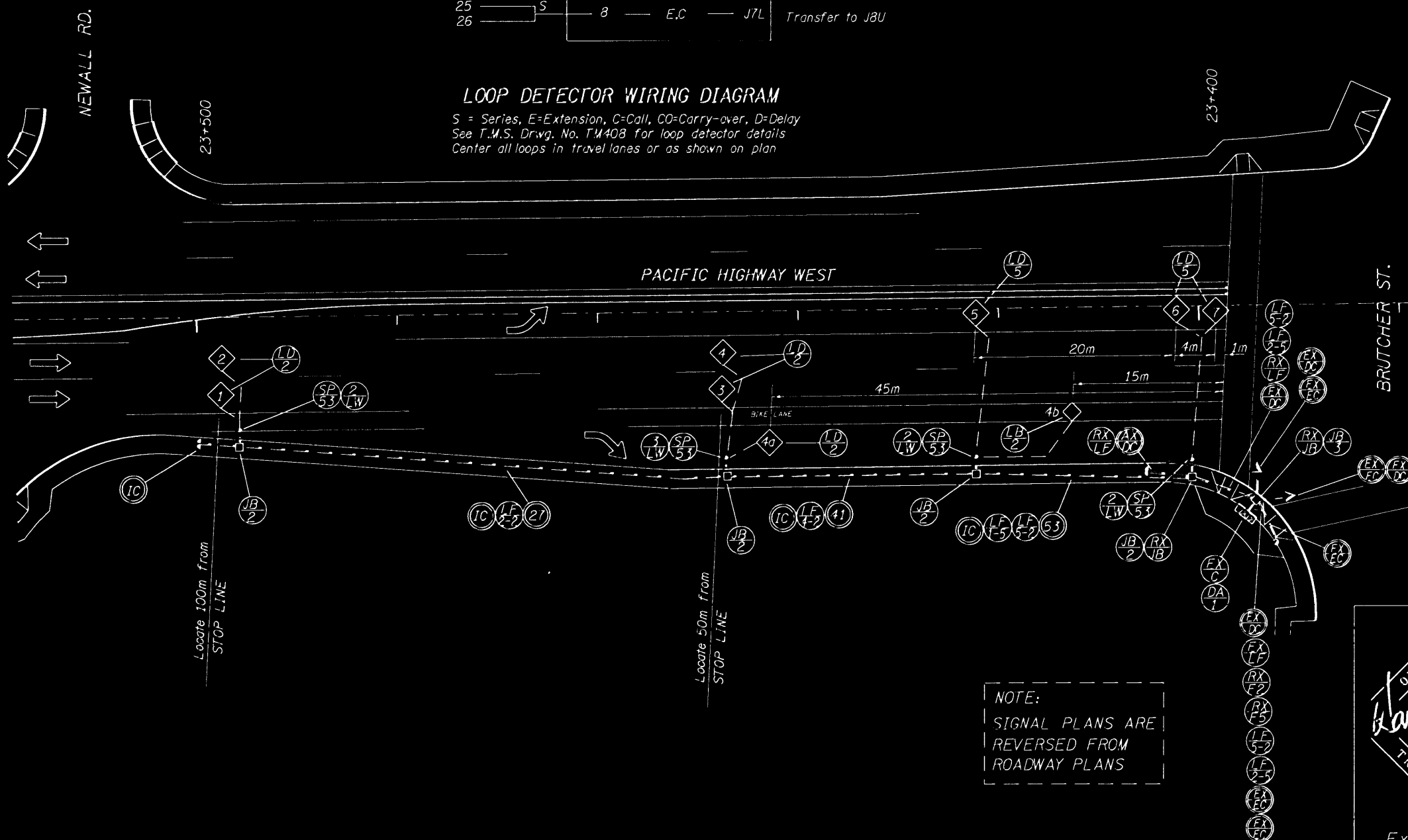
LEGEND

- (EX/C) Retain and protect existing Model 170 controller and Model 332 cabinet
 - (EX/TF) Retain and protect existing loop feeder cables
 - (EX/EC) Retain and protect existing electrical conduit
 - (EX/DC) Retain and protect existing detector conduit
 - (EX/IC) Retain and protect existing interconnect conduit
 - (RX/FP) Remove existing phase (Ph) loop feeder cable
 - (RX/JB) Remove existing junction box
 - (AX/DC) Abandon existing detector conduit
 - (DA/N) Install (N) loop detector amplifier in existing controller cabinet
 - (JB/2) Install 580 mm x 305 mm x 305 mm (min. dimension) precast concrete junction box
 - (JB/3) Install 780 mm x 430 mm x 305 mm (min. dimension) precast concrete junction box
 - (SP/S) Install 150 mm max. sand pocket block-out with (S) mm conduit to junction box
 - (LD/Ph) Install phase (Ph) 1.2 m diamond vehicle detector loop
 - (LB/Ph) Install phase (Ph) 0.7 m diamond bicycle detector loop
 - (LX/Ph) Install (X) phase (Ph) loop feeder cables
 - (N/LW) Install (N) pair of loop wires
 - (S) Install (S) mm electrical conduit
 - (IC) Interconnect conduit (See Interconnect Plan)
- Ph = Phase shown
 X = Number of cables shown
 N = Number shown
 S = Size shown

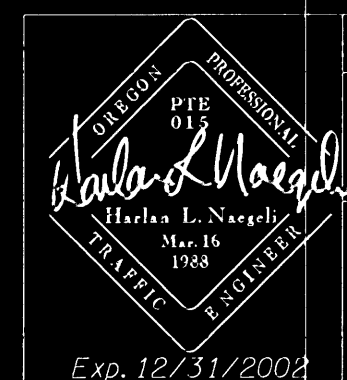
Loop Number	Phase	Function	Slot	
1	2	E,C	I2U	Controller Cabinet
2	2	E,C	I2L	
3	2	E,C	I3U	
4	2	E,C	I3U	
4a	2	E,C,CO	I4U	Transfer to I3U w/carryover
4b	2	E,C	I3L	
5	5	E,C,CO	J1U	
6	5	E,C	J9U	
7	4	E,C	I7U	Transfer to I8U
8	4	E,C,CO	I6U	
9	4	E,C,CO	I6L	
10	4	E,C	I7U	
11	6	E,C	J2U	
12	6	E,C	J2L	
13	6	E,C	J3U	
14	6	E,C	J3U	
15	1	E,C,CO	I1U	
16	1	E,C	I9U	
17	8	E,C,CO	J6U	
18	8	E,C,CO	J6L	
19	8	E,C	J7U	
20	8	E,C	J7L	
21	8	E,C	J7L	Transfer to J8U
22	8	E,C	J7L	
23	8	E,C	J7L	
24	8	E,C	J7L	
25	8	E,C	J7L	
26	8	E,C	J7L	

LOOP DETECTOR WIRING DIAGRAM

S = Series, E=Extension, C=Call, CO=Carry-over, D=Delay
 See T.M.S. Drwg. No. TM408 for loop detector details
 Center all loops in travel lanes or as shown on plan



NOTE:
 SIGNAL PLANS ARE
 REVERSED FROM
 ROADWAY PLANS



OREGON DEPARTMENT OF TRANSPORTATION
 TRAFFIC MANAGEMENT SECTION

TRAFFIC SIGNAL INSTALLATION
 BRUTCHER ST. - EVEREST ST. &
 EVEREST ST. - MAIN ST. (NEWBERG) SECS.
 PACIFIC HIGHWAY WEST
 YAMHILL COUNTY

DATE: November 2000
 DESIGNED BY: H. Naegeli
 CHECKED BY: T. Jenkins
 DRAWN BY: H. Naegeli

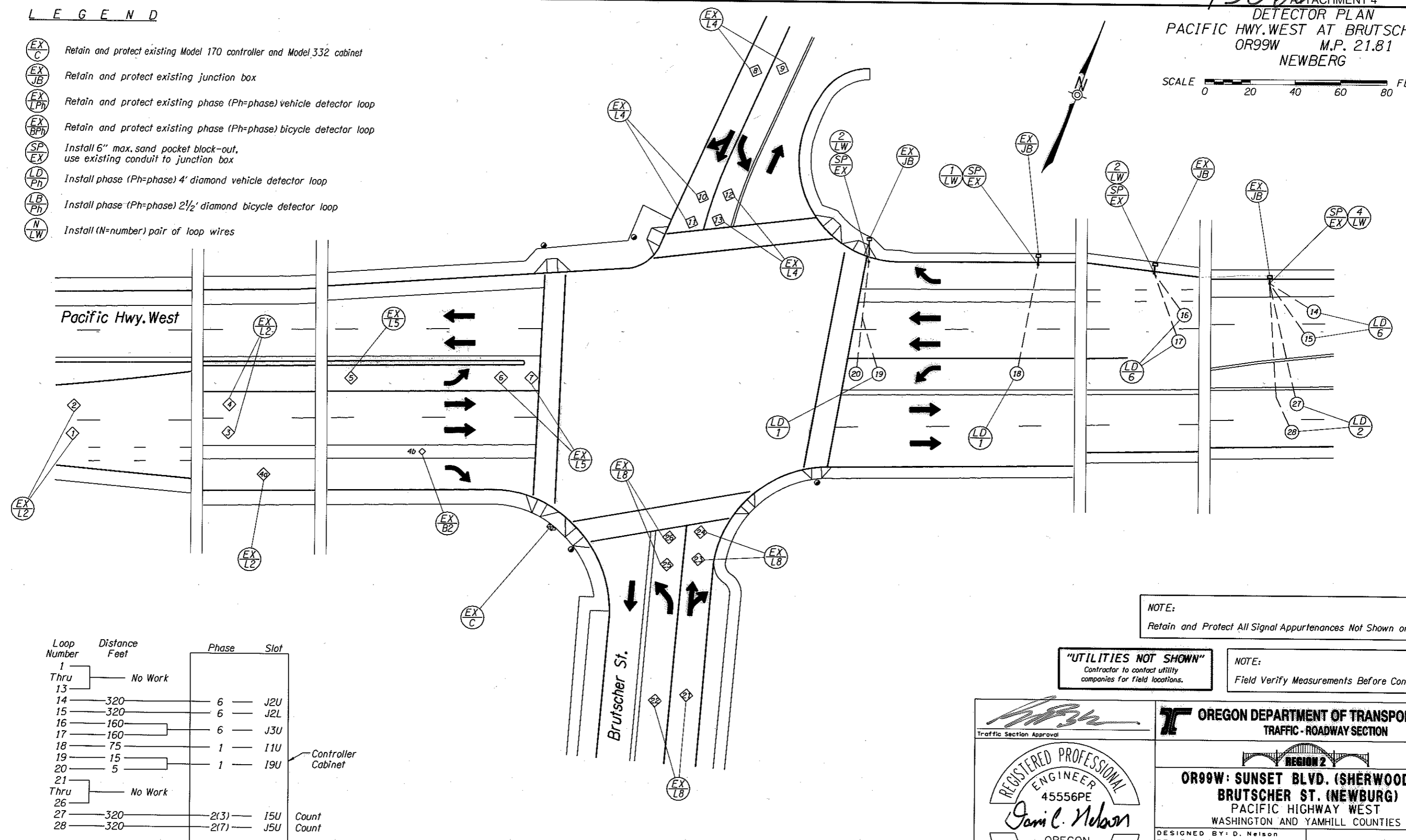
B6 07022tr3.sg1 T.M.S. DRG. NO. 12285

DETECTOR PLAN
 PACIFIC HWY. WEST AT BRUTSCHER ST.
 OR99W M.P. 21.81
 NEWBERG

SCALE 0 20 40 60 80 FEET

LEGEND

- Retain and protect existing Model 170 controller and Model 332 cabinet
- Retain and protect existing junction box
- Retain and protect existing phase (Ph=phase) vehicle detector loop
- Retain and protect existing phase (Ph=phase) bicycle detector loop
- Install 6" max. sand pocket block-out, use existing conduit to junction box
- Install phase (Ph=phase) 4' diamond vehicle detector loop
- Install phase (Ph=phase) 2 1/2' diamond bicycle detector loop
- Install (N=number) pair of loop wires



NOTE:
 Retain and Protect All Signal Appurtenances Not Shown on Plans

"UTILITIES NOT SHOWN"
 Contractor to contact utility companies for field locations.

NOTE:
 Field Verify Measurements Before Construction

Loop Number	Distance Feet	Phase	Slot
1			
Thru 13	No Work		
14	320	6	J2U
15	320	6	J2L
16	160	6	J3U
17	160		
18	75	1	11U
19	15	1	19U
20	5		
21			
Thru 26	No Work		
27	320	2(3)	15U
28	320	2(7)	J5U

Controller Cabinet

Count Count

LOOP DETECTOR WIRING DIAGRAM
 "Distance" is from Stop Line to center of loop in feet

NOTE:
 Use existing loop feeder cables and match new loops to present configuration

Traffic Section Approval

REGISTERED PROFESSIONAL ENGINEER
 45556PE
 Dani C. Nelson
 OREGON
 JULY 9, 2001
 DANI C. NELSON

EXPIRES: December 31, 2010

OREGON DEPARTMENT OF TRANSPORTATION
 TRAFFIC - ROADWAY SECTION

REGION 2

OR99W: SUNSET BLVD. (SHERWOOD) - BRUTSCHER ST. (NEWBERG)
 PACIFIC HIGHWAY WEST
 WASHINGTON AND YAMHILL COUNTIES

DESIGNED BY: D. Nelson
 REVIEWED BY: S. Cramer
 DRAWN BY: D. Nelson
 FC: 091 MP: 21.81

DETECTOR PLAN

TSSU D. 03070 T.R.S. DWG. NO. 15560

Tuesday, October 28, 2014 12:30

Intersection Name	13 - OR99W at Brustcher	Local ID	13
Intersection Telephone Number			
System Name	64 - Newberg-Dundee	System ID	64
Controller Type	Voyage - C1-C11		
Controller Serial Number		Installation Date	
Programmed by		Programmed Date	

Graphic Map Background	Phase Rotation Diagram

Control Data (next/2/2)

Controller Function and Timing (next/2/1, next/2/2)

Security, Sequence, Initialization

Security Code	****	0 = disabled, or 1000-9999
Sequence	7	0 = sequential, 1 = quad left turn, 2-6 = special A-E, 7 = lead lag

	Lead Lag (next/2/2/3)			
	Phases 1 - 2	Phases 3 - 4	Phases 5 - 6	Phases 7 - 8
	2	2	2	2
	0 = no reversal, 1 = reversal, 2 = by coord plan or clock			

Initialization and Flash (next/2/2/5)

	Initialization	Flash Entry	Flash Exit	
Ring 1 Phase	1	0	1	phase 1-8
Ring 2 Phase	5	0	5	phase 1-8
Interval	0	0	0	0 = red, 1 = yellow, 2 = green
Power up Flash	0.0	0.0 - 25.5 seconds	First All Red	8.0
				0.0 - 25.5 seconds

Soft Flash (next/2/2/5)

Phase	1	2	3	4	5	6	7	8	0 = dark, 1=flash yel WIG, 2 = flash yel WAG, 3 = flash red WIG, 4 = flash red WAG				
	3	4	3	4	3	4	3	4					
Overlap	A	B	C	D	E	F	G	H	I	J	K	L	same as phase
	3	4	3	4	3	4	3	4	3	4	3	4	
Internal Logic Output	1	2	3	4	5	6	7	8	9	10	11	12	0 = normal, 1 = dark, 2 = flash WIG
	0	0	0	0	0	0	0	0	0	0	0	0	

Per Phase Functions (next/2/2/3, next/2/2/1)									ATTACHMENT 4
	1	2	3	4	5	6	7	8	
Phases Used	X	X		X	X	X		X	X = on
Restricted Phases									X = on (Sequence 2, 6, 7 only)
Exclusive Phases									X = on (Sequence 7 only)
Yellow Lock									X = on
Min Recall		X				X			
Max Recall									
Ped Recall									
Red Lock									
Max Out Recall Inhibit	X	X	X	X	X	X	X	X	
Soft Recall									
Free Walk Rest									
Conditional Ped									
Disable Inhibit Max Termination									
Call to Non Act 1									
Call to Non Act 2									
Dual Entry (next/2/2/9/3)									
Mode	1	0 = off, 1 = on, 2 = Not Used, 3 = by coord plan, 4 = by time clock circuit 61							
Dual Entry Phase -->	1	2	3	4	5	6	7	8	
Phase	0	0	0	8	0	0	0	4	0 = none, 1-8 = phase 1-8
Conditional Service, Five Section Head									
Conditional Service (next/2/2/9/3)			5 Section Head Logic (next/2/2/9/4)						
Phase	Mode	CS Max Time	X Omits Y		Anti-Trap		Yellow Blanking LT		
					Trap Protected Phase	Next Phase	Phase		
Phase 1	0	0	X : Y				< (5)	1	
Phase 3	0	0	6 : 1	0	1		< (7)	3	
Phase 5	0	0	8 : 3	0	3		< (1)	5	
Phase 7	0	0	2 : 5	0	5		< (3)	7	
0 = off, 1 = C.S.On. 2 = C.S. on by TOD circuit 57, 3 = N/A, 4 = C.S. and C.R. On, 5 = C.R. on by TOD circuit 57.			0=off, 1=side call, 2=no side call		X = On				

Phase Times (next/2/2/2, next/2/2/9/5)									
	1	2	3	4	5	6	7	8	ATTACHMENT 4
Movement									
Minimum Green	4	10	0	6	4	10	0	6	0 - 255 sec
Passage	2.3	4.8	0.0	2.5	2.3	4.8	0.0	2.5	0.0 - 25.5 sec
Yellow	4.0	4.5	0.0	4.0	4.0	4.5	0.0	4.0	0.0 - 25.5 sec
Red Clearance	0.5	0.5	0.0	0.5	0.5	0.5	0.0	0.5	0.0 - 25.5 sec or 0 - 255 sec
Max 1	20	50	0	25	20	50	0	25	0 - 255 sec
Max 2	20	50	0	25	20	50	0	25	0 - 255 sec
Walk	0	7	0	7	0	7	0	7	0 - 255 sec
Ped Clear	0	16	0	25	0	17	0	24	0 - 255 sec
Seconds Per Actuation	0.0	0.6	0.0	0.0	0.0	0.6	0.0	0.0	0.0 - 25.5 sec
Time Before Reduction	8	10	0	5	8	10	0	5	0 - 255 sec
Time to Reduce	3	20	0	5	3	20	0	5	0 - 255 sec
Minimum Gap	0.5	2.8	0.0	2.0	0.5	2.8	0.0	2.0	0.0 - 25.5 sec
Max Variable Initial	4	21	0	6	4	21	0	6	0 - 255 sec
Auto Max Adjust	0.0	5.0	0.0	0.0	0.0	5.0	0.0	0.0	0.0 - 25.5 sec
Auto Max Limit	0	65	0	0	0	65	0	0	0 - 255 sec
Inhibit Min Yellow									X = On
Red Decimal Off									X = On
Advance Walk	0	0	0	0	0	0	0	0	0 - 255 sec
Other Controller Functions (next/2/2/9)									
Phase -->	1	2	3	4	5	6	7	8	
Inhibit Simultaneous Gap Out	X			X	X			X	X = On
Last Car Passage	2	0 = recall phase, 1 = last car passage, 2 = NOT recall - Not last car passage							
Red Revert (+2 seconds)	3.0	0 - 25.5 sec							
Auto Ped Clear	X	X = On							
Flashing Don't Walk Into Yellow		X = On							
Soft Recall / Red Rest Delay	0.0	0 - 25.5 sec							
Ped Pushbutton	0	0 - 5 sec, 0 = disable							
Advance Flash Rate	0	0 = disable, 1 = 120 FPM							
Change Sequence		X = On (After a download with a power on - off cycle)							
Phase -->	1	2	3	4	5	6	7	8	
Red Clear Extension Detector	0	0	0	0	0	0	0	0	0 = none 1 - 32 = detector 1 - 32
Red Clear Extension Red Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec.

Local Detectors (next/2/2/4)

Detector Data							ATTACHMENT 4	
Yellow Lock	Detector Inhibit	Call Phase	Extend Phase	Switch Phase	Delay Time	Stretch / Disconnect Time	Delay or Disconnect Mode	
Detector 1 - 18		1	1	0	0	1.5	0	
Detector 2 - 19,20		1	1	0	0	0.0	0	
Detector 3 - 27 ATR		3	3	0	0	0.0	0	
Detector 4 -		3	3	0	0	0.0	0	
Detector 5 - 5		5	5	0	0	1.5	0	
Detector 6 - 6,7		5	5	0	0	0.0	0	
Detector 7 - 28 ATR		7	7	0	0	0.0	0	
Detector 8 -		7	7	0	0	0.0	0	
Detector 9 - 1		2	2	0	0	1.0	0	
Detector 10 - 2		2	2	0	0	1.0	0	
Detector 11 - 3,4		2	2	0	0	0.0	0	
Detector 12 - 4b		2	2	0	1	3.0	0	
Detector 13 - 4a		2	2	0	5	1.0	0	
Detector 14 - 8		4	4	0	0	1.5	0	
Detector 15 - 9		4	4	0	0	1.5	0	
Detector 16 - 10,11		4	4	0	1	0.0	0	
Detector 17 - 12,13		4	4	0	0	0.0	0	
Detector 18 -		4	4	0	0	0.0	0	
Detector 19 - 14		6	6	0	0	1.0	0	
Detector 20 - 15		6	6	0	0	1.0	0	
Detector 21 - 16,17		6	6	0	0	0.0	0	
Detector 22 -		6	6	0	0	0.0	0	
Detector 23 -		6	6	0	0	0.0	0	
Detector 24 - 21		8	8	0	0	1.5	0	
Detector 25 - 22		8	8	0	0	1.5	0	
Detector 26 - 23,24		8	8	0	1	0.0	0	
Detector 27 - 25,26		8	8	0	0	0.0	0	
Detector 28 -		8	8	0	0	0.0	0	
Detector 29 -		0	0	0	0	0.0	0	
Detector 30 -		0	0	0	0	0.0	0	
Detector 31 -		0	0	0	0	0.0	0	
Detector 32 -		0	0	0	0	0.0	0	

yellow lock, detector inhibit, - X = On; call, extend, phase - 0 = none 1 - 8 = phase 1 - 8; delay time - 0 - 255 sec
stretch / disconnect time - 0.0 - 25.5 sec.; delay or disconnect Mode - 0 - 13

Detector Plans (next/2/2/4/5)

Detector Plans (next/2/2/4/5)											
Loop Number											
	Plan Detectors	0	0	0	0	0	0	0	0	0 - 32, 0 = none, 1 - 3 2 = detectors 1 - 32	
Detector Plan 1	Call Phase	0	0	0	0	0	0	0	0	0 - 8, 0 = none, 1 - 8 = phase 1 - 8	
	Extend Phase	0	0	0	0	0	0	0	0		
	Switch Phase	0	0	0	0	0	0	0	0		
	Delay Time	0	0	0	0	0	0	0	0		0 - 255 sec
	Stretch/Disconnect Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0 - 25.5 sec
	Delay/ Disconnect Mode	0	0	0	0	0	0	0	0		0 - 13
Detector Plan 2	Call Phase	0	0	0	0	0	0	0	0	0 - 8, 0 = none, 1 - 8 = phase 1 - 8	
	Extend Phase	0	0	0	0	0	0	0	0		
	Switch Phase	0	0	0	0	0	0	0	0		
	Delay Time	0	0	0	0	0	0	0	0		0 - 255 sec
	Stretch/Disconnect Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0 - 25.5 sec
	Delay/ Disconnect Mode	0	0	0	0	0	0	0	0		0 - 13
Detector Plan 3	Call Phase	0	0	0	0	0	0	0	0	0 - 8, 0 = none, 1 - 8 = phase 1 - 8	
	Extend Phase	0	0	0	0	0	0	0	0		
	Switch Phase	0	0	0	0	0	0	0	0		
	Delay Time	0	0	0	0	0	0	0	0		0 - 255 sec
	Stretch/Disconnect Time	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0 - 25.5 sec
	Delay/ Disconnect Mode	0	0	0	0	0	0	0	0		0 - 13

Detector Fail Monitor (next/2/2/4/3)					Detectors 33-64 (next/2/2/4/6)					
	Fail Monitor Enable	Recall Phase	Min Counts	Max Counts	ATTACHMENT 4					
					Call Phase	Extend Phase				
Detector 1 - 18		0	0	0	Detector 33 -	0	0			
Detector 2 - 19,20		0	0	0	Detector 34 -	0	0			
Detector 3 - 27 ATR		0	0	0	Detector 35 - 6,7	0	0			
Detector 4 -		0	0	0	Detector 36 -	0	0			
Detector 5 - 5		0	0	0	Detector 37 -	0	0			
Detector 6 - 6,7		0	0	0	Detector 38 -	0	0			
Detector 7 - 28 ATR		0	0	0	Detector 39 -	0	0			
Detector 8 -		0	0	0	Detector 40 -	0	0			
Detector 9 - 1		0	0	0	Detector 41 -	0	0			
Detector 10 - 2		0	0	0	Detector 42 -	0	0			
Detector 11 - 3,4		0	0	0	Detector 43 -	0	0			
Detector 12 - 4b		0	0	0	Detector 44 -	0	0			
Detector 13 - 4a		0	0	0	Detector 45 -	0	0			
Detector 14 - 8		0	0	0	Detector 46 -	0	0			
Detector 15 - 9		0	0	0	Detector 47 -	0	0			
Detector 16 - 10,11		0	0	0	Detector 48 -	0	0			
Detector 17 - 12,13		0	0	0	Detector 49 -	0	0			
Detector 18 -		0	0	0	Detector 50 -	0	0			
Detector 19 - 14		0	0	0	Detector 51 -	0	0			
Detector 20 - 15		0	0	0	Detector 52 -	0	0			
Detector 21 - 16,17		0	0	0	Detector 53 -	0	0			
Detector 22 -		0	0	0	Detector 54 -	0	0			
Detector 23 -		0	0	0	Detector 55 -	0	0			
Detector 24 - 21		0	0	0	Detector 56 -	0	0			
Detector 25 - 22		0	0	0	Detector 57 -	0	0			
Detector 26 - 23,24		0	0	0	Detector 58 -	0	0			
Detector 27 - 25,26		0	0	0	Detector 59 -	0	0			
Detector 28 -		0	0	0	Detector 60 -	0	0			
Detector 29 -		0	0	0	Detector 61 -	0	0			
Detector 30 -		0	0	0	Detector 62 -	0	0			
Detector 31 -		0	0	0	Detector 63 -	0	0			
Detector 32 -		0	0	0	Detector 64 -	0	0			
fail monitor enable - X = On, recall phase - 0 = none 1 - 8 = phase 1 - 8, min, max					call / extend phase - 0 = none 1 - 8 = phase 1 - 8					
Detector Fail Sample Period (all detectors)			0		0 - 255 minutes					
Video Fail Inputs (next/2/2/4/3) -->			1	2	3	4	5	6	7	8
Phase Recalled			0	0	0	0	0	0	0	0
0 = none, 1 - 8 = phase 1 - 8										
System Detectors (next/2/2/4/4)										
System Detectors -->			1	2	3	4	5	6	7	8
Local Detector			0	0	0	0	0	0	0	0
0 = none, 1 - 32 = phase 1 - 32										

Overlaps / FYLTA (next/2/2/8)														
Vehicle Overlaps		Phase or Movement	Phases								Extension Green	Clearance		ATTACHMENT # 0 = none 1 = overlap 2 = 60 FPM 3 = Not ped 4=Comp. Ph. 5=Prevent. Ext. 6=Not Veh. 7=Adv. FF E - L 0 = no Overlap 1 = Overlap Green, Yellow Red
			1	2	3	4	5	6	7	8		Yellow	Red	
Overlaps	A		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	B		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	C		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	D		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	E		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	F		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	G		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	H		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	I		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	J		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	K		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
	L		0	0	0	0	0	0	0	0	0.0	0.0	0.0	
Not Ped - Ped Overlaps (next/2/2/8/5)														
Ped Overlaps -->		A	B	C	D	E	F	G	H					
Overlaps	A									X = Nor Ped Ped Overlap				
	B													
	C													
	D													
Advance Warning (next/2/2/8/3)														
			E	F	G	H	I	J	K	L				
Enable			0	0	0	0	0	0	0	0	0 = disabled, 1 = enabled			
1st Conditional Overlap			0	0	0	0	0	0	0	0	0 = none, 1 - overlap E, 2 = overlap F, etc.			
2nd Conditional Overlap			0	0	0	0	0	0	0	0				
Advance Deactivation Delay			0	0	0	0	0	0	0	0	0 - 99 seconds			
Ped Overlaps (next/2/2/8/5)														
Phase -->		1	2	3	4	5	6	7	8	Walk	Ped Clear	Ped Recall	Phase, Ped Recall: X = on Walk, Ped Clear: 0 - 255 seconds	
Ped Overlap	A									0	0			
	B									0	0			
	C									0	0			
	D									0	0			
	E									0	0			
	F									0	0			
	G									0	0			
	H									0	0			
	Flashing Yellow Left Turn Arrow (FYLTA) (next/2/2/8/6)													
Phase Pairs -->		1 - 2	3 - 4	5 - 6	7 - 8									
Enable		0	0	0	0	0 = off, 3 = 3 outputs, 4 = 4 outputs, 5 = 5 outputs								
Even Omits Odd		0	0	0	0	0 = off, 1 = on, 2 = on, place call across barrier								
Detector Switch Odd / Even		X	X	X	X	X = on, odd phase must be omitted								
Red Transition		3.0	3.0	3.0	3.0	0.0 or 2.0 - 25.5 sec								
Red Extension		0.0	0.0	0.0	0.0	0.0 - 25.5 sec								
Return to GLTA		0	0	0	0	0 = off, 1 = max out, 2 = yellow lock								
Flashing Yellow Left Turn Arrow (FYLTA) - Continued on last page														

Service Plans (next/2/2/6)

ATTACHMENT 4

Phase -->		1	2	3	4	5	6	7	8		
Service Plan 1	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 2	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 3	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 4	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 5	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	
Service Plan 6	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	

Service Plans Cont.

ATTACHMENT 4

Phase -->		1	2	3	4	5	6	7	8		
Service Plan 7	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	

Phase -->		1	2	3	4	5	6	7	8		
Service Plan 8	Call Mode	0	0	0	0	0	0	0	0		
	0 = actuated, 1 = omit, 2 = CNA, 3 = min recall, 4 = max recall, 5 = soft recall, 6 = ped recall, 7 = omit ped, 8 = red rest										
	Minimum Green	0	0	0	0	0	0	0	0	0	0 - 255 sec.
	Passage	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 or 3.0 - 25.5
	Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec.
	Walk	0	0	0	0	0	0	0	0	0	0 - 255 sec.
Pedestrian Clearance	0	0	0	0	0	0	0	0	0	0 - 255 sec.	

Max Plans (next/2/2/7)

Phase -->		1	2	3	4	5	6	7	8	
Max Plan 1	Normal Max	25	65	0	25	25	65	0	25	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 2	Normal Max	25	80	0	30	25	80	0	30	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 3	Normal Max	25	80	0	35	30	80	0	40	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 4	Normal Max	30	90	0	35	30	90	0	35	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 5	Normal Max	0	0	0	0	0	0	0	0	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 6	Normal Max	25	90	0	30	25	90	0	30	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 7	Normal Max	20	65	0	20	20	65	0	20	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec
Max Plan 8	Normal Max	0	0	0	0	0	0	0	0	0 - 255 sec
	Fail Max	0	0	0	0	0	0	0	0	
	Auto Max Adjust	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Auto Max Limit	0	0	0	0	0	0	0	0	0 - 255 sec

Coordination Data (next/2/3)

ATTACHMENT 4

Coordination Modes (next/2/3/1, next/2/3/4/1, next/2/3/4/3)

Flash Mode	33	0=off, 1=on, 33=time clock, 34=comm, 35=hardwire, 36=NWS Set only, 37=AB3418 / NTCIP S
Coordination Plan Mode	33	0=free, 1-32 = coord plan 1-32, 33=time clock, 34=comm, 35=hardwire, 36=NWS Set only, 37=
Offset Seeking Mode	2	0=add only, 1= dwell, 2=fastway
Late Ped	0	0 = off, 1 = on
Coord Walk Rest	0	0 = off, 1 = on, 2 = by TOD circuit 160, 3 = end of walk, 4 = coord ped during perms
Repeated Phase Service	0	0=off, 1=on (no coord ped), 2=on (beginning green coord ped), 3=on (coord ped always)
Zero Mode (TS2 only)	0	0=start of main street, 1=end of main street, 2=by TOD circuit 144

	Phase -->	1	2	3	4	5	6	7	8	0 = service allowed 1 = service prevented
Omit Phase During Repeated Phase Service		0	0	0	0	0	0	0	0	
Auto Permissive Min Green		0	0	0	0	0	0	0	0	0 - 255 seconds

Coordination Plans (next/2/3/2)

Coord Plan	Coordination Phases		Cycle Length	Offset Time	Min Cycle Length Dwell Time	Permissive	Service Plan	Max Plan	
	Ring 1	Ring 2							
1-	2	6	90	57	0	0	0	1	
2-	2	6	100	80	0	0	0	2	
3-	2	6	110	28	0	0	0	3	
4-	2	6	120	55	0	0	0	4	
5-	2	6	0	0	0	0	0	0	
6-	2	6	100	71	0	0	0	6	
7-	2	6	0	0	0	0	0	0	
8-	0	0	0	0	0	0	0	0	
9-	0	0	0	0	0	0	0	0	
10-	0	0	0	0	0	0	0	0	
11-	0	0	0	0	0	0	0	0	
12-	0	0	0	0	0	0	0	0	
13-	0	0	0	0	0	0	0	0	
14-	0	0	0	0	0	0	0	0	
15-	0	0	0	0	0	0	0	0	
16-	0	0	0	0	0	0	0	0	
17-	0	0	0	0	0	0	0	0	
18-	0	0	0	0	0	0	0	0	
19-	0	0	0	0	0	0	0	0	
20-	0	0	0	0	0	0	0	0	
21-	0	0	0	0	0	0	0	0	
22-	0	0	0	0	0	0	0	0	
23-	0	0	0	0	0	0	0	0	
24-	0	0	0	0	0	0	0	0	
25-	0	0	0	0	0	0	0	0	
26-	0	0	0	0	0	0	0	0	
27-	0	0	0	0	0	0	0	0	
28-	0	0	0	0	0	0	0	0	
29-	0	0	0	0	0	0	0	0	
30-	0	0	0	0	0	0	0	0	
31-	0	0	0	0	0	0	0	0	
32-	0	0	0	0	0	0	0	0	
0 - 8			0 - 255 sec.				0 - 8		

Coordination Plans cont.

ATTACHMENT 4

Coord Plan	* = Force Offs / Split Times (TS2)								* = Yield Points / Actuated Times (TS2)	
	1	2	3	4	5	6	7	8	Ring 1	Ring 2
1-	15	57	0	18	15	57	0	18	5	15
2-	15	65	0	20	15	65	0	20	15	5
3-	18	64	0	28	20	62	0	28	20	5
4-	20	72	0	28	20	72	0	28	5	5
5-	0	0	0	0	0	0	0	0	0	0
6-	18	62	0	20	18	62	0	20	20	5
7-	0	0	0	0	0	0	0	0	0	0
8-	0	0	0	0	0	0	0	0	0	0
9-	0	0	0	0	0	0	0	0	0	0
10-	0	0	0	0	0	0	0	0	0	0
11-	0	0	0	0	0	0	0	0	0	0
12-	0	0	0	0	0	0	0	0	0	0
13-	0	0	0	0	0	0	0	0	0	0
14-	0	0	0	0	0	0	0	0	0	0
15-	0	0	0	0	0	0	0	0	0	0
16-	0	0	0	0	0	0	0	0	0	0
17-	0	0	0	0	0	0	0	0	0	0
18-	0	0	0	0	0	0	0	0	0	0
19-	0	0	0	0	0	0	0	0	0	0
20-	0	0	0	0	0	0	0	0	0	0
21-	0	0	0	0	0	0	0	0	0	0
22-	0	0	0	0	0	0	0	0	0	0
23-	0	0	0	0	0	0	0	0	0	0
24-	0	0	0	0	0	0	0	0	0	0
25-	0	0	0	0	0	0	0	0	0	0
26-	0	0	0	0	0	0	0	0	0	0
27-	0	0	0	0	0	0	0	0	0	0
28-	0	0	0	0	0	0	0	0	0	0
29-	0	0	0	0	0	0	0	0	0	0
30-	0	0	0	0	0	0	0	0	0	0
31-	0	0	0	0	0	0	0	0	0	0
32-	0	0	0	0	0	0	0	0	0	0
0 - 255 sec * = force offs and yield points										

Circuit Mapping (next/2/3/3)																	
Circuit Map	Coord Plan	Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		Time Clock Circuit		ATTACHMENT 4 Time Clock Circuit		Time Clock Circuit	
1	1	97	LG1	173	DPA	161	SI1	164	SI4	165	SI5	0	N/U	0	N/U	0	N/U
2	2	99	LG5	173	DPA	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
3	3	99	LG5	173	DPA	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
4	4	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
5	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
6	6	99	LG5	173	DPA	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
7	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
8	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
9	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
10	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
11	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
12	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
13	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
14	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
15	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
16	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
17	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
18	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
19	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U
20	34	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U	0	N/U

coord plan - 0 = free, 1 - 32 = coord plan 1 - 32, 33 = any, 34 none selected
time clock circuits - 0 = not used, or circuits 6 - 196

Dynamic Phase Length (next/2/3/4/4)									
Phase -->	1	2	3	4	5	6	7	8	
Back Detector	1	29	0	30	5	31	0	32	0 = none, 1-32 = detector 1-32
Lane Factor	1.0	2.0	0	2.0	1.0	2.0	0	2.0	0 = none, 1.0 - 5.0
Check Out Detector	0	5	0	0	0	1	0	0	0 = none, 1-32 = detector 1-32
Coord Delta Force Off	Set A	5	5	0	5	5	5	0	0 - 255 sec
	Set B	0	0	0	0	0	0	0	
	Set C	0	0	0	0	0	0	0	
	Set D	0	0	0	0	0	0	0	
Free Delta Max	Set A	0	0	0	0	0	0	0	
	Set B	0	0	0	0	0	0	0	
	Set C	0	0	0	0	0	0	0	
	Set D	0	0	0	0	0	0	0	

Platoon Progression (next/2/3/4/5)													
Entry Local Only						Master Local Only							
Platoon Max	0	0 - 255 sec				Smoothing Factor	0.0	0.0 - 1.0					
Min Platoon Green	0	0 - 255 sec											
Entry Detector Gap	0.0	0.0 - 25.5											
Min Platoon Cycle	0	0 - 255 sec											
Inbound						Outbound							
Only for Entry Inbound Local or Master Local						Only for Entry Outbound Local or Master Local							
Entry IB Local also Last OB Local	0	0 - 50				Entry OB Local also Last IB Local	0	0 - 50					
Speed	0	0 - 55 mph				Speed	0	0 - 55 mph					
Distance from Entry Local	0	0 - 65000 feet				Distance from Entry Local	0	0 - 65000 feet					
Entry Local Only						Entry Local Only							
Distance from Entry Local Detector	0	0 - 999 feet				Distance from Entry Local Detector	0	0 - 999 feet					
Entry Local Detector	0	0	0 - 32			Entry Local Detector	0	0	0 - 32				
Master Local						Master Local							
Master Mid - System Critical Detectors	0	0	0 - 16			Master Mid - System Critical Detectors	0	0	0 - 16				
Force Off Percents													
Inbound	1	3	4	5	7	8	Outbound	1	3	4	5	7	8
Split 1	0	0	0	0	0	0	Split 1	0	0	0	0	0	0
Split 2	0	0	0	0	0	0	Split 2	0	0	0	0	0	0
0 - 100 %						0 - 100 %							

Time of Day Data (next/2/4)

ATTACHMENT 4

Day Program (next/2/4/1)

	Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On / Off		Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On/Off
1	1	06:00	X	1			51				
2	1	07:30	X	2			52				
3	1	10:30	X	3			53				
4	1	16:00	X	4			54				
5	1	18:00	X	3			55				
6	1	20:00	X	0			56				
7	2	07:30	X	1			57				
8	2	10:00	X	6			58				
9	2	18:00	X	0			59				
10							60				
11							61				
12							62				
13							63				
14							64				
15							65				
16							66				
17							67				
18							68				
19							69				
20							70				
21							71				
22							72				
23							73				
24							74				
25							75				
26							76				
27							77				
28							78				
29							79				
30							80				
31							81				
32							82				
33							83				
34							84				
35							85				
36							86				
37							87				
38							88				
39							89				
40							90				
41							91				
42							92				
43							93				
44							94				
45							95				
46							96				
47							97				
48							98				
49							99				
50							100				
	1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on		1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on

Day Program cont.

	Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On / Off		Day Prog.	Time	Coord Plan	Coord Plan or Circuit	State On / Off
101							151				
102							152				
103							153				
104							154				
105							155				
106							156				
107							157				
108							158				
109							159				
110							160				
111							161				
112							162				
113							163				
114							164				
115							165				
116							166				
117							167				
118							168				
119							169				
120							170				
121							171				
122							172				
123							173				
124							174				
125							175				
126							176				
127							177				
128							178				
129							179				
130							180				
131							181				
132							182				
133							183				
134							184				
135							185				
136							186				
137							187				
138							188				
139							189				
140							190				
141							191				
142							192				
143							193				
144							194				
145							195				
146							196				
147							197				
148							198				
149							199				
150							200				
	1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on		1 - 15	hh : mm	X = on	coord plan 0 - 32 or circuit 1-196	X = on

Circuit Overrides (next/2/4/4)

			ATTACHMENT	
1 - Coord Line 1	CL1	TOD		51 - Ped Omit 3
2 - Coord Line 2	CL2	TOD	PO4	52 - Ped Omit 4
3 - Coord Line 4	CL4	TOD	PO5	53 - Ped Omit 5
4 - Coord Line 8	CL8	TOD	PO6	54 - Ped Omit 6
5 - Coord Line 16	C16	TOD	PO7	55 - Ped Omit 7
6 - Coord Operation	CRD	TOD	PO8	56 - Ped Omit 8
7 - Soft Flash	SFL	TOD	CVS	57 - Conditional Service
8 - Enable System Relays	ESR	TOD	ISG	58 - Inhibit Simultaneous Gap Out
9 - Call to Non Act 1	CN1	TOD	HWI	59 - Inhibit Hardwire
10 - Call to Non Act 2	CN2	TOD	POM	60 - Ped Override Mode
11 - Walk Rest Modifier	WRM	TOD	DLE	61 - Dual Entry
12 - Min Recall	MIN	TOD	EPD	62 - Exclusive Ped
13 - Max 2 Both Rings	MX2	TOD	CTC	63 - Call to Time Clock Mode
14 - Coord Inhibit Max Ring 1, 2	IMT	TOD	DEP	64 - Dual Enhanced Ped
15 - Enable Service Log	ESL	TOD	SP1	65 - Service Plan 1
16 - Call to Free	CTF	TOD	SP2	66 - Service Plan 2
17 - TOD Output 1	TO1	TOD	SP3	67 - Service Plan 3
18 - TOD Output 2	TO2	TOD	SP4	68 - Service Plan 4
19 - TOD Output 3	TO3	TOD	SP5	69 - Service Plan 5
20 - TOD Output 4	TO4	TOD	SP6	70 - Service Plan 6
21 - TOD Output 5	TO5	TOD	SP7	71 - Service Plan 7
22 - TOD Output 6	TO6	TOD	SP8	72 - Service Plan 8
23 - TOD Output 7	TO7	TOD	MP1	73 - Max Plan 1
24 - TOD Output 8	TO8	TOD	MP2	74 - Max Plan 2
25 - Vehicle Call Phase 1	VC1	TOD	MP3	75 - Max Plan 3
26 - Vehicle Call Phase 2	VC2	TOD	MP4	76 - Max Plan 4
27 - Vehicle Call Phase 3	VC3	TOD	MP5	77 - Max Plan 5
28 - Vehicle Call Phase 4	VC4	TOD	MP6	78 - Max Plan 6
29 - Vehicle Call Phase 5	VC5	TOD	MP7	79 - Max Plan 7
30 - Vehicle Call Phase 6	VC6	TOD	MP8	80 - Max Plan 8
31 - Vehicle Call Phase 7	VC7	TOD	TG1	81 - Transit Priority Max Group 1
32 - Vehicle Call Phase 8	VC8	TOD	TG2	82 - Transit Priority Max Group 2
33 - Ped Call Phase 1	PC1	TOD	TG3	83 - Transit Priority Max Group 3
34 - Ped Call Phase 2	PC2	TOD	TG4	84 - Transit Priority Max Group 4
35 - Ped Call Phase 3	PC3	TOD	TG5	85 - Transit Priority Max Group 5
36 - Ped Call Phase 4	PC4	TOD	TG6	86 - Transit Priority Max Group 6
37 - Ped Call Phase 5	PC5	TOD	TG7	87 - Transit Priority Max Group 7
38 - Ped Call Phase 6	PC6	TOD	TG8	88 - Transit Priority Max Group 8
39 - Ped Call Phase 7	PC7	TOD	IV1	89 - Inhibit Volume Density 1
40 - Ped Call Phase 8	PC8	TOD	IV2	90 - Inhibit Volume Density 2
41 - Vehicle Omit 1	VO1	TOD	IV3	91 - Inhibit Volume Density 3
42 - Vehicle Omit 2	VO2	TOD	IV4	92 - Inhibit Volume Density 4
43 - Vehicle Omit 3	VO3	TOD	IV5	93 - Inhibit Volume Density 5
44 - Vehicle Omit 4	VO4	TOD	IV6	94 - Inhibit Volume Density 6
45 - Vehicle Omit 5	VO5	TOD	IV7	95 - Inhibit Volume Density 7
46 - Vehicle Omit 6	VO6	TOD	IV8	96 - Inhibit Volume Density 8
47 - Vehicle Omit 7	VO7	TOD	LG1	97 - Lag 1
48 - Vehicle Omit 8	VO8	TOD	LG3	98 - Lag 3
49 - Ped Omit 1	PO1	TOD	LG5	99 - Lag 5
50 - Ped Omit 2	PO2	TOD	LG7	100 - Lag 7

On /
Off /
TOD

On /
Off /
TOD

Circuit Overrides cont.

				ATTACHMENT	
101 - Inhibit Overlap A	OLA	TOD			
102 - Inhibit Overlap B	OLB	TOD			
103 - Inhibit Overlap C	OLC	TOD			
104 - Inhibit Overlap D	OLD	TOD			
105 - Enable Schedule A Phone 1	AT1	TOD			
106 - Enable Schedule A Phone 2	AT2	TOD			
107 - Enable Schedule B Phone 1	BT1	TOD			
108 - Enable Schedule B Phone 2	BT2	TOD			
109 - Enable Schedule C Phone 1	CT1	TOD			
110 - Enable Schedule C Phone 2	CT2	TOD			
111 - Enable Volume to Call Phone 1	VT1	TOD			
112 - Enable Volume to Call Phone 2	VT2	TOD			
113 - Enable Volume Logging	EVL	On			
114 - Enable MOE Logging	EML	On			
115 - Detector Low Threshold Inhibit	DLI	TOD			
116 - Detector Continue Presence Inhibit	DPI	TOD			
117 - Inhibit Detector Based on Programming	IND	TOD			
118 - Inhibit Detector Delay	IDD	TOD			
119 - Inhibit Conditional Ped	ICP	TOD			
120 - Inhibit Transit Priority	ITP	TOD			
121 - Red Rest Ring 1,2	RRM	TOD			
122 - Enable Transcend	TRA	TOD			
123 - Omit Red Clear Ring 1,2	ORC	TOD			
124 - Not Used	N/U	TOD			
125 - Ped Recycle Ring 1,2	PCY	TOD	On /		On /
126 - Not Used	N/U	TOD	Off /		Off /
127 - Enable MOE Log to Call Phone 1	MT1	TOD	TOD		TOD
128 - Enable MOE Log to Call Phone 2	MT2	TOD			
129 - Transit Inhibit Short Time 1	IS1	TOD			
130 - Transit Inhibit Short Time 2	IS2	TOD			
131 - Transit Inhibit Short Time 3	IS3	TOD			
132 - Transit Inhibit Short Time 4	IS4	TOD			
133 - Transit Inhibit Short Time 5	IS5	TOD			
134 - Transit Inhibit Short Time 6	IS6	TOD			
135 - Transit Inhibit Short Time 7	IS7	TOD			
136 - Transit Inhibit Short Time 8	IS8	TOD			
137 - Enable Transit Priority Logging	ETL	TOD			
138 - Disable Flashing Yellow Arrow 1	DF1	TOD			
139 - Disable Flashing Yellow Arrow 3	DF3	TOD			
140 - Disable Flashing Yellow Arrow 5	DF5	TOD			
141 - Disable Flashing Yellow Arrow 7	DF7	TOD			
142 - Disable Auto Max	DAM	TOD			
143 - Disable Repeat Phase Service	DRS	TOD			
144 - Coord End of Main Street	EMS	TOD			
145 - Coord Hold 1	HD1	TOD			
146 - Coord Hold 2	HD2	TOD			
147 - Coord Hold 3	HD3	TOD			
148 - Coord Hold 4	HD4	TOD			
149 - Coord Hold 5	HD5	TOD			
150 - Coord Hold 6	HD6	TOD			
151 - Coord Hold 7					
152 - Coord Hold 8					
153 - PE Priority Return B					
154 - PE Priority Return C					
155 - PE Priority Return D					
156 - PE Priority Return E					
157 - Platoon Inbound					
158 - Platoon Outbound					
159 - Platoon Spl 2					
160 - Coord Walk Rest					
161 - Dynamic Phase Length Short Inhibit 1					
162 - Dynamic Phase Length Short Inhibit 2					
163 - Dynamic Phase Length Short Inhibit 3					
164 - Dynamic Phase Length Short Inhibit 4					
165 - Dynamic Phase Length Short Inhibit 5					
166 - Dynamic Phase Length Short Inhibit 6					
167 - Dynamic Phase Length Short Inhibit 7					
168 - Dynamic Phase Length Short Inhibit 8					
169 - Coord Late Left Turn 1					
170 - Coord Late Left Turn 3					
171 - Coord Late Left Turn 5					
172 - Coord Late Left Turn 7					
173 - Dynamic Phase Length Enable A					
174 - Dynamic Phase Length Enable B					
175 - Dynamic Phase Length Enable C					
176 - Dynamic Phase Length Enable D					
177 - Proactive Plan Select Average					
178 - Proactive Plan Select Inbound					
179 - Proactive Plan Select Outbound					
180 - Split Variant Inbound					
181 - Split Variant Outbound					
182 - Disable Coord Walk Rest Ring 1					
183 - Disable Coord Walk Rest Ring 2					
184 - Proactive Plan Select New Look					
185 - Disable Red Clearance Extension					
186 - Detector Plan Line 1					
187 - Detector Plan Line 2					
188 - Disable LRT 1 Vertical Flashing Bar					
189 - Disable LRT 2 Vertical Flashing Bar					
190 - Disable LRT 3 Vertical Flashing Bar					
191 - Disable LRT 4 Vertical Flashing Bar					
192 - Datakey Enable					
193 - Dynamic Phase Reversal Enable 1					
194 - Dynamic Phase Reversal Enable 3					
195 - Dynamic Phase Reversal Enable 5					
196 - Dynamic Phase Reversal Enable 7					
197 - Enable Coord Logging					
198 - Disable Gap FYLTA 1,3,5,7					
199 - Coordination Auto Walk					
200 - Enable Coordinated Auto Max					

Preemption Data (next/2/5)

ATTACHMENT 4

Sequence (next/2/5/1 - 8)							Instructions
Sequences / Intervals	Instruction	Phases Serviced	Interval Time	Hold On Input	Outputs On	Output Mode	
1	1	197	25	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
2	1	197	4	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
3	1	197	16	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
4	1	197	8	0	1	0	
	2	98		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	
5	1	0		0	0	0	
	2	0		0	0	0	
	3	0		0	0	0	
	4	0		0	0	0	
	5	0		0	0	0	
	6	0		0	0	0	
	7	0		0	0	0	
	8	0		0	0	0	
	9	0		0	0	0	
	10	0		0	0	0	

0 - Service Phases
 1-9 = Special Interval 1-9
 10 - Preempt Sequence Allows FYLTA
 11 - Preempt Interval Disables FYLTA
 15 - Alternate Trap Protection
 90 - Go to all Red
 91 - Soft Flash On
 92 - Soft Flash Off
 93 - Enable Ped
 94 - Disable Peds
 95 - Priority Return
 96 - Enable Coordination with peds
 97 - Enable Coordination without peds
 98 - Return with NO Calls
 99 - Return with Vehicle Calls
 100 - jump to step in Interval Time
 101 - Use Interval Time as Resettable Gap Timer
 196 - Coord Re-synch with Peds
 197 - Coord Re-synch without Peds
 200 - Light Rail Train phase without Peds
 201 - Light Rail Train phase with Peds
 202 - Return to highest queue/delay phase (this uses the Dynamic Phase Length Back Detectors)
 216 - Light Rail Train Coord Re-synch with Peds
 217 - Light Rail Train Coord Re-synch without Peds

Phases Serviced - phases 1 - 8
 Interval Time - 0 - 255 sec or interval 1 - 10
 Hold on Input:
 0 = Do not hold
 1 = Hold
 2 = Ped Service to Rest in Walk

Outputs On - output 1 - 8
 Output Modes -
 0 = all steady on
 1 = all flash together
 2 = odd flashes WIG, even flashes WAG
 3 = 1 - 4 steady on, 5 - 8 all flash together

Sequence cont.

ATTACHMENT 4

Sequences / Intervals	Instruction	Phases Serviced	Interval Time	Hold On Input	Outputs On	Output Mode
6	1	0	0	0		0
	2	0	0	0		0
	3	0	0	0		0
	4	0	0	0		0
	5	0	0	0		0
	6	0	0	0		0
	7	0	0	0		0
	8	0	0	0		0
	9	0	0	0		0
	10	0	0	0		0
7	1	0	0	0		0
	2	0	0	0		0
	3	0	0	0		0
	4	0	0	0		0
	5	0	0	0		0
	6	0	0	0		0
	7	0	0	0		0
	8	0	0	0		0
	9	0	0	0		0
	10	0	0	0		0
8	1	0	0	0		0
	2	0	0	0		0
	3	0	0	0		0
	4	0	0	0		0
	5	0	0	0		0
	6	0	0	0		0
	7	0	0	0		0
	8	0	0	0		0
	9	0	0	0		0
	10	0	0	0		0

Sequence Timing (next/2/5/0)

Sequence -->	1	2	3	4	5	6	7	8	
Input Memory									X = on
Input Priority	6	6	6	6	0	0	0	0	0 = lowest, - 8 = highest
Min Green	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec
Walk	0.1	0.1	0.1	0.1	0.0	0.0	0.0	0.0	0.0 would time the normal function time
Ped Clear	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Overlap Yellow	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0 - 25.5 sec
Overlap Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Delay to Preempt	0	0	0	0	0	0	0	0	
Delay Ped Omit	0	0	0	0	0	0	0	0	0 - 255 sec
Delay Phase Omit	0	0	0	0	0	0	0	0	
Min Reservice	0	0	0	0	0	0	0	0	0 - 255 min
Overlap Inhibits	A								X = inhibit
	B								
	C								
	D								
Exit Parameters	Exit to Coord Plan Offset by X	0	0	0	0	0	0	0	0 - 20
	Exit Coord Plan Time	0	0	0	0	0	0	0	0 - 60 min
	Exit to Max Plan	0	0	0	0	0	0	0	0 - 8
	Exit Free Time	0	0	0	0	0	0	0	0 - 60 min
	Override Time	0	0	0	0	0	0	0	
	Fail Time	0	0	0	0	0	0	0	
Exit Mode Time	0	0	0	0	0	0	0	0	

Priority Return and Special Intervals (next/2/5/0/6, next/2/5/9)														
Phase / Overlap -->		1	2	3	4	5	6	7	8	A	B	C	D	ATTACHMENT 4
Priority Return	Enable	0	0 = disabled, 1 = enabled, 2 = enabled, skip preemption phases on exit											
	A (max)	0	0	0	0	0	0	0	0	0 - 100% of currently used max				
	B (max)	0	0	0	0	0	0	0	0					
	C (max)	0	0	0	0	0	0	0	0					
	D (max)	0	0	0	0	0	0	0	0					
	E (max)	0	0	0	0	0	0	0	0					
Ped Clear	0	0	0	0	0	0	0	0	0	0 - 100% of currently used ped clearance				
Queue Delay Recovery	0	0	0	0	0	0	0	0	0	0 - 255 sec.				
Special Intervals	1	0	0	0	0	0	0	0	0	0	0	0	0	0 = Dark 1 = green don't walk 2 = green walk 3 = green flashing don't walk 4 = yellow 5 = red 6 = flashing yellow WIG 7 = flashing yellow WAG 8 = flashing red WIG 9 = flashing red WAG 10 = walk only 11=flashing don't walk only
	2	0	0	0	0	0	0	0	0	0	0	0	0	
	3	0	0	0	0	0	0	0	0	0	0	0	0	
	4	0	0	0	0	0	0	0	0	0	0	0	0	
	5	0	0	0	0	0	0	0	0	0	0	0	0	
	6	0	0	0	0	0	0	0	0	0	0	0	0	
	7	0	0	0	0	0	0	0	0	0	0	0	0	
	8	0	0	0	0	0	0	0	0	0	0	0	0	
	9	0	0	0	0	0	0	0	0	0	0	0	0	
Light Rail Train (next/2/5/0/7)														
Light Rail Train -->		1	2	3	4									
Associated Preempt		0	0	0	0	0 = none, preempt 1 - 8								
Time to Green		0	0	0	0	0 - 255 sec								
Horizontal Bar Flash Time		0.0	0.0	0.0	0.0	0.0 - 25.5 sec								
Vertical Bar Flash Time		0.0	0.0	0.0	0.0	0 - 255 sec								
Min Duration		0	0	0	0	0 - 255 sec								

Communications Data (next/2/6)

ATTACHMENT 4

1st Central Phone Number				2nd Central Phone Number			
Modem Setup String		<i>AT&F&N6&W</i>		Intersection Name		<i>Hwy 99w at Brutscher</i>	
Subnet Mask		<i>255.0.0.0</i>					
IP (ethernet) Port		<i>25000</i>					
Central Port		<i>6</i>					
System Mode		<i>2</i>					
System Port		<i>1</i>		Alternate System Port		<i>0</i>	
System ID	<i>64</i>	AB3418e Physical Address	<i>1</i>	IP Address	<i>10.1.1.1</i>		
Local ID	<i>13</i>	AB3418e Group Address	<i>0</i>	Gateway Address	<i>167.131.62.1</i>		
Baud Rates		Flow Control		Port Use			
Port 1 (Slot A2 Upper)		<i>2</i>		<i>1</i>		<i>Suggested Use - FSK</i>	
Port 2 (Slot A2 Lower)		<i>2</i>		<i>0</i>		<i>Suggested Use - Not Used</i>	
Port 3 (Slot A1 Upper)		<i>0</i>		<i>1</i>		<i>Suggested Use - Modem to Central</i>	
Port 4 (Slot A1 Lower or C50S)		<i>2</i>		<i>NU</i>		<i>Suggested Use - RS232 to Laptop</i>	
0 = 1200, 1 = 2400, 2 = 9600, 3 = 19200 baud				0 = off, 1 = on			
Reports							
Volume Log Period		<i>15</i>	minute	Volume/Occ Log Period		<i>0</i>	second
				MOE Log Period		<i>15</i>	minute
0 = disabled, 1,2,3,4,5,6,10,12,15,20,30,60 minutes							
Function Schedule Mapping (next/2/6/7)							
Alarm 1	<i>0</i>	0 = none 1 = schedule A 2 = schedule B 3 = schedule C 4 = schedule R	Soft Flash	<i>1</i>	0 = none 1 = schedule A 2 = schedule B 3 = schedule C 4 = schedule R		
Alarm 2	<i>0</i>		Manual Control Enable (MCE)	<i>4</i>			
Alarm 3	<i>0</i>		Emergency or Railroad Preempt	<i>1</i>			
Alarm 4	<i>0</i>		Not Used	<i>0</i>			
Alarm 5	<i>0</i>		Cycle Failure	<i>2</i>			
Not Used	<i>0</i>		Coordination Failure	<i>2</i>			
Not Used	<i>0</i>		Keyboard use / Data Changed	<i>3</i>			
Not Used	<i>0</i>		Coord Running / Free	<i>2</i>			
Power On / Off	<i>1</i>		Cabinet Door	<i>3</i>			
Checksum Failure	<i>4</i>		Extended Ped Pushbutton	<i>0</i>			
Video / Detector Failure	<i>4</i>	Monitor Status	<i>4</i>				
Master to Local Comm Lost	<i>0</i>	Red Extension	<i>0</i>				

Miscellaneous Data

ATTACHMENT 4

Transit Priority (next/2/7)									
	1	2	3	4	5	6	7	8	
Phases									Phases 1 - 8 (max of 2 compatible phases)
PE Enable (6.25Hz TP call on PE)									X = 6.25 Hz signal will activate TP
Priority	0	0	0	0	0	0	0	0	0 - 8, 8 = highest
Memory									X = on
Delay Time	0	0	0	0	0	0	0	0	0 - 255 sec
Minimum Reservice Time (per input)	0	0	0	0	0	0	0	0	0 - 255 min
Override Time	0	0	0	0	0	0	0	0	0 - 255 sec
Bus Extend	0	0	0	0	0	0	0	0	0 - 255 sec
Minimum Reservice Time (all inputs)	0	0 - 255 min							
Free Operation Mode	0	0 = use shortest of max 1 or 2, 1 - 8 = use max time of group 1 - 8, 9 = use time of day							

Transit Priority Alternate Force Off Plans									
Current Coord Plan	1	2	3	4	5	6	7	8	
Alternate TP Force Off Plan	0	0	0	0	0	0	0	0	0 = none 17 - 32 = coord plan 17 - 32
Current Coord Plan	9	10	11	12	13	14	15	16	
Alternate TP Force Off Plan	0	0	0	0	0	0	0	0	

Group Timing									
Phase -->	1	2	3	4	5	6	7	8	
Group 1	Max Times	0	0	0	0	0	0	0	0 - 255 sec 0 would time the normal function time
	Walk Times	0	0	0	0	0	0	0	
Group 2	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 3	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 4	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 5	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 6	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 7	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	
Group 8	Max Times	0	0	0	0	0	0	0	
	Walk Times	0	0	0	0	0	0	0	

Truck Priority (next/2/7/9)					
Truck Priority-->	1	2	3	4	
Associated Transit Priority	0	0	0	0	0 = none 1 - 8 = transit priority 1 - 8
Leading Detector	0	0	0	0	0 = none, 1 - 32 = detector 1 - 32
Trailing Detector	0	0	0	0	
Stop Bar Distance	0	0	0	0	0 - 999 feet
Trap Distance	0	0	0	0	0.0 - 99.9 feet
Minimum Speed	0	0	0	0	0 - 100 mph
Minimum Length	0	0	0	0	0 - 255 feet
Downhill Grade	0	0	0	0	0 - 20 %
Uphill Grade	0	0	0	0	
Undersized Vehicle					X = Enabled

Change I/O	X = On (After a download with a power on - off cycle)
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Inputs (Non Default I/O is offset to the right) (next/2/8/1)											
C1-39	101	VD9	C1-55	15	VD5	C1-67	22	PED2	C1-15	254	N/U
C1-40	113	VD19	C1-56	11	VD1	C1-68	26	PED6	C1-16	254	N/U
C1-41	106	VD14	C1-57	17	VD7	C1-69	24	PED4	C1-17	254	N/U
C1-42	118	VD24	C1-58	13	VD3	C1-70	28	PED8	C1-18	254	N/U
C1-43	102	VD10	C1-59	16	VD6	C1-71	151	PE1	C1-19	254	N/U
C1-44	114	VD20	C1-60	12	VD2	C1-72	152	PE2	C1-20	254	N/U
C1-45	107	VD15	C1-61	18	VD8	C1-73	153	PE3	C1-21	254	N/U
C1-46	161	VD25	C1-62	14	VD4	C1-74	154	PE4	C1-22	254	N/U
C1-47	105	VD13	C11-10	254	N/U	C1-75	121	CLKR	C1-23	254	N/U
C1-48	117	VD23	C11-11	254	N/U	C1-76	104	VD12	C1-24	254	N/U
C1-49	112	VD18	C11-12	254	N/U	C1-77	116	VD22	C1-25	254	N/U
C1-50	164	VD28	C11-13	254	N/U	C1-78	111	VD17	C1-26	254	N/U
C1-51	199	PEDI	C1-63	103	VD11	C1-79	163	VD27	C1-27	254	N/U
C1-52	155	PE5	C1-64	115	VD21	C1-80	82	IADV	C1-28	254	N/U
C1-53	85	MCE	C1-65	108	VD16	C1-81	137	MONS	C1-29	254	N/U
C1-54	254	N/U	C1-66	162	VD26	C1-82	62	ST1	C1-30	254	N/U

Outputs (Non Default I/O is offset to the right) (next/2/8/2)											
C1-2	44	4DWK	C1-19	48	8DWK	C1-35	131	TO1	C1-91	41	1DWK
C1-3	64	4WLK	C1-20	68	8WLK	C1-36	132	TO2	C1-93	61	1WLK
C1-4	14	4RED	C1-21	18	8RED	C1-37	133	TO3	C1-94	106	OLBR
C1-5	24	4YEL	C1-22	28	8YEL	C1-38	134	TO4	C1-95	105	OLBY
C1-6	34	4GRN	C1-23	38	8GRN	C1-100	53	3PCL	C1-96	104	OLBG
C1-7	13	3RED	C1-24	17	7RED	C1-101	51	1PCL	C1-97	103	OLAR
C1-8	23	3YEL	C1-25	27	7YEL	C1-102	187	SFL	C1-98	102	OLAY
C1-9	33	3GRN	C1-26	37	7GRN	C1-103	147	WDOG	C1-99	101	OLAG
C1-10	42	2DWK	C1-27	46	6DWK	C1-83	43	3DWK	C11-1	254	N/U
C1-11	62	2WLK	C1-28	66	6WLK	C1-84	63	3WLK	C11-2	254	N/U
C1-12	12	2RED	C1-29	16	6RED	C1-85	116	OLDR	C11-3	254	N/U
C1-13	22	2YEL	C1-30	26	6YEL	C1-86	115	OLDY	C11-4	254	N/U
C1-15	32	2GRN	C1-31	36	6GRN	C1-87	114	OLDG	C11-5	254	N/U
C1-16	11	1RED	C1-32	15	5RED	C1-88	113	OLCR	C11-6	254	N/U
C1-17	21	1YEL	C1-33	25	5YEL	C1-89	112	OLCY	C11-7	254	N/U
C1-18	31	1GRN	C1-34	35	5GRN	C1-90	111	OLCG	C11-8	254	N/U

Internal Logic (next/2/9)			
Step	Inst.	Description	Comment ATTACHMENT 4
1	201	Jumper Two Inputs	
2	101	Vehicle Detector 9	
3	165	Vehicle Detector 29	
4	201	Jumper Two Inputs	
5	102	Vehicle Detector 10	
6	165	Vehicle Detector 29	
7	201	Jumper Two Inputs	
8	106	Vehicle Detector 14	
9	166	Vehicle Detector 30	
10	201	Jumper Two Inputs	
11	107	Vehicle Detector 15	
12	166	Vehicle Detector 30	
13	201	Jumper Two Inputs	
14	113	Vehicle Detector 19	
15	167	Vehicle Detector 31	
16	201	Jumper Two Inputs	
17	114	Vehicle Detector 20	
18	167	Vehicle Detector 31	
19	201	Jumper Two Inputs	
20	118	Vehicle Detector 24	
21	168	Vehicle Detector 32	
22	201	Jumper Two Inputs	
23	161	Vehicle Detector 25	
24	168	Vehicle Detector 32	
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Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
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Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
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Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
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Internal Logic cont.

Step	Inst.	Description	Comment ATTACHMENT 4
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FYLTA - Continued (next/2/2/8/6)

Phase Pairs -->		1 - 2	3 - 4	5 - 6	7 - 8	
Gap-Dependent FYLTA (next/2/2/8/6-A)	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	0	0	0	0 - 255 sec
	Not Ped	0	0	0	0	0 - 255 sec

FYLTA Gap-Dependent Plans (next/2/2/8/6)

Phase Pairs -->		1 - 2	3 - 4	5 - 6	7 - 8	
FYLTA Gap-Dependent Plan A	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	0	0	0	0 - 255 sec
	Not Ped	0	0	0	0	0 - 255 sec
FYLTA Gap-Dependent Plan B	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	0	0	0	0 - 255 sec
	Not Ped	0	0	0	0	0 - 255 sec
FYLTA Gap-Dependent Plan C	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors
	Min Delay	0	0	0	0	0 - 255 sec
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec
	Max Delay	0	342/690	0	0	0 - 255 sec
	Not Ped	0	0	0	0	0 - 255 sec

	Not Ped	0	0	0	0	0 - 255 sec											
FYLTA Gap-Dependent Plan D	Detector Input	0	0	0	0	0 = disable, 1 - 64 detectors	ATTACHMENT 4										
	Min Delay	0	0	0	0	0 - 255 sec											
	Detector Gap	0.0	0.0	0.0	0.0	0 - 25.5 sec											
	Max Delay	0	0	0	0	0 - 255 sec											
	Not Ped	0	0	0	0	0 - 255 sec											
Preemption - Continued																	
Railroad Communications (IEEE 1570) (next/2/5/0/8)																	
		ATC		Wayside													
Railroad Number	0	0	0	0	0	0 - 999, represents railroad											
Railroad Line Number	0	0	0	0	0	0 - 999, represents railroad line											
Group Number	0	0	0	0	0	0 - 999, represents physical group of equipment											
Subnode Number	0	0	0	0	0	0 - 99, subnode within physical group of equipment											
Device Number	0	0	0	0	0	0 - 99, device within physical group of equipment											
Associated Preempt	0				0 - 8												
Communication Port	0				0 - 4												
Reports - Continued																	
Reports - Service Delay Modes (next/2/6/0)																	
Phase -->	1	2	3	4	5	6	7	8									
Mode	0	0	0	0	0	0	0	0	0 = disable, 1 = enable, 2 = Ped, 3 = Veh/P								
Ped Overlap -->	A	B	C	D	E	F	G	H									
Mode	0	0	0	0	0	0	0	0	0 = disable, 1 = enable								
Detector -->	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Detector -->	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Detector -->	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Detector -->	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	
Enable	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

LEGEND

- INSTALL MODEL 170 CONTROLLER IN MODEL 332 CABINET WITH RISER FRAME. ORIENT FRONT (LOUVERED) DOOR AS SHOWN. CONSTRUCT FOUNDATION AS PER STANDARD DWG. NO. TM423.
- INSTALL SPECIAL HEAVY-DUTY (NON-STANDARD) TRAFFIC SIGNAL MAST ARM POLE WITH LUMINAIRE POLE EXTENSION (35 FT. MOUNTING HEIGHT). SEE SPECIFICATIONS.
- INSTALL (L) FT. TRAFFIC SIGNAL MAST ARM.
- INSTALL (L) FT. LUMINAIRE ARM.
- INSTALL PHASE (PH) VEHICLE SIGNAL WITH LEDS.
- INSTALL PHASE (PH) PEDESTRIAN SIGNAL WITH LEDS, PUSHBUTTON, AND INSTRUCTION SIGN.
- INSTALL TERMINAL CABINET.
- INSTALL ALUMINUM STREET NAME SIGN ON MAST ARM (SEE SPECIAL PROVISIONS).
- INSTALL 17"x10"x12" (MIN. DIMENSION) PRECAST CONCRETE JUNCTION BOX.
- INSTALL 30"x17"x12" (MIN. DIMENSION) PRECAST CONCRETE JUNCTION BOX.
- INSTALL CHANNEL (CH), (N) BARREL EMERGENCY VEHICLE PRE-EMPTION DETECTOR UNIT.
- INSTALL CHANNEL (CH) EMERGENCY VEHICLE PRE-EMPTION DETECTOR FEEDER CABLE.
- INSTALL PHOTOCONTROL ELECTRONIC RELAY CELL ON POLE (18' TO 30' ABOVE POLE BASE ON NORTH SIDE).
- INSTALL COBRAHEAD STYLE LUMINAIRE, 250 WATT HIGH PRESSURE SODIUM, MULTI-TAP BALLAST SET AT 240 VOLTS, 35' MOUNTING HEIGHT. LUMINAIRE SHALL BE GE MSCL25S3N22FMC3 OR APPROVED EQUAL.
- RETAIN AND PROTECT EXISTING POWER VAULT (POWER SOURCE).
- INSTALL PEDESTRIAN SIGNAL PEDESTAL.
- INSTALL 2 INCH CONDUIT STUB IN CONTROLLER FOR CONNECTION BY TELEPHONE COMPANY AS DIRECTED BY VERIZON (CONTACT 503-620-9354). PHONE LINE MUST BE OPERATIONAL PRIOR TO SIGNAL TURN-ON.
- INSTALL (S) INCH ELECTRICAL CONDUIT. CONDUIT LOCATION SHOWN AS APPROXIMATE ONLY. FINAL LOCATION TO BE DETERMINED IN FIELD AND APPROVED BY ENGINEER.
- DETECTOR CONDUIT (REFER TO DETECTOR PLAN).
- INSTALL (X) CONTROL CABLES (MSA SPEC. 19-1) WITH (N) AWG NO. (G) CONDUCTORS.
- INSTALL REMOTE POWER SERVICE POST.
- SERVICE CABINET, 120/240 VOLT, FOR BOTH SIGNAL AND ILLUMINATION CIRCUITS.
- INSTALL 120/240 VOLT METER BASE.
- INSTALL ALUMINUM (30"x36", TYPE W7) LEFT ARROW "ONLY" SIGN
- INSTALL ALUMINUM (30"x36", TYPE W7) RIGHT ARROW "RIGHT" SIGN

- INSTALL (N) NO. 8 TYPE XHHW OR THWN WIRE (SIGNAL SYSTEM COMMON).
- INSTALL (N) NO. (G) TYPE XHHW OR THWN WIRE.
- INSTALL (N) NO. (G) STRANDED BARE COPPER WIRE (GROUND).
- INSTALL POLY PULL LINE (210# MIN. STRENGTH).

N = NUMBER SHOWN
 X = NUMBER OF CABLES SHOWN
 PH = PHASE SHOWN
 L = LENGTH SHOWN
 CH = CHANNEL SHOWN
 S = SIZE SHOWN
 G = AWG SIZE SHOWN
 T = TYPE SHOWN
 A = STANDARD PLUMBIZER
 TB = TENON MOUNT ADJUSTABLE SIGNAL BRACKET, PELCO #AB 3028-SS.
 P = TYPE P-1 SIGNAL POLE SIDE MOUNT
 2 = 12" RED, 12" YELLOW, 12" GREEN.
 3R = 12" RED RTA, 12" YELLOW RTA, 12" GREEN RTA
 4L = 12" RED LTA, 12" YELLOW LTA, 12" GREEN LTA

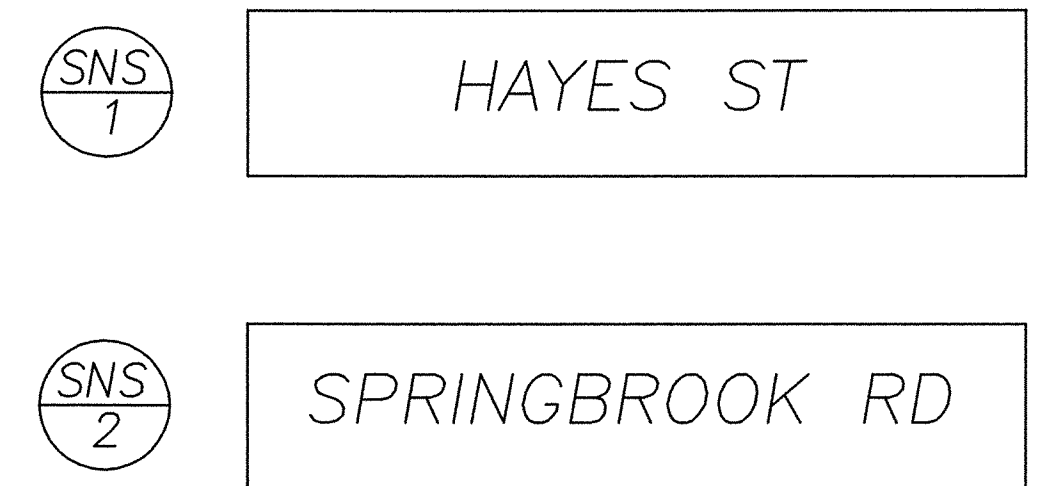
POLE ENTRANCE CHART

POLE NO.	1	2	3	4
LUMINAIRE ARM	-	7	1	3
MAST ARM	-	7	1	3
TERMINAL CABINET	-	3	5	7
PED SIGNAL CLAM SHELL	1/7	1	5	5/7
PED PUSHBUTTON	3/5	5	1	1/3

GENERAL NOTES

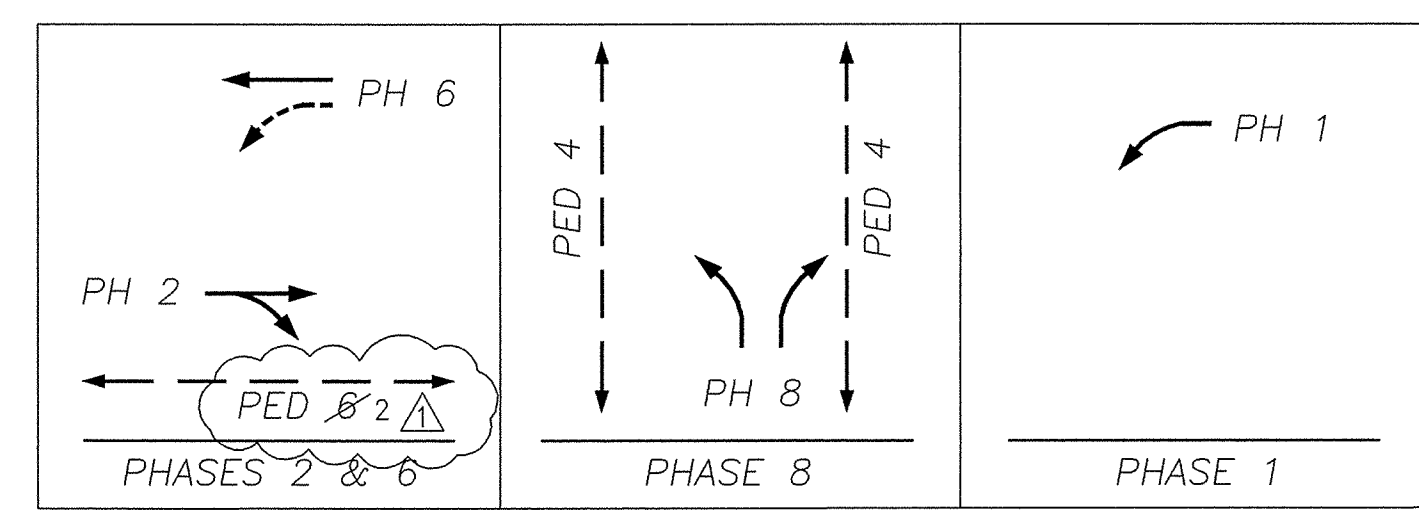
1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE OREGON DEPARTMENT OF TRANSPORTATION (TRAFFIC ENGINEERING SECTION) STANDARD DRAWINGS, THE OREGON DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, THE CITY OF NEWBERG STANDARD CONSTRUCTION SPECIFICATIONS, AND THE SPECIAL PROVISIONS FOR THIS CONTRACT.
2. THE CONTRACTOR SHALL VERIFY THE LOCATIONS OF THE EXISTING UTILITIES AND COORDINATE WORK WITH THE UTILITY COMPANIES/AGENCIES TO ELIMINATE ANY CONFLICTS.
3. THE CONTRACTOR SHALL COORDINATE WORK WITH PGE FOR POWER SERVICE CONNECTION. THE CONTRACTOR SHALL INSTALL CONDUIT AND WIRING TO POWER SOURCE AS REQUIRED BY PGE.
4. LOCATIONS OF ALL POLES, CONDUITS, SIGNAL HEADS, AND JUNCTION BOXES (AS SHOWN ON PLANS) ARE APPROXIMATE. ACTUAL LOCATIONS SHALL BE DESIGNATED IN THE FIELD BY THE ENGINEER. CONDUIT SHALL BE PLACED IN THE SAME TRENCH WITH OTHER CONDUITS WHENEVER POSSIBLE.
5. NEW SIGNAL POLE FOUNDATIONS SHALL BE GRADED TO MATCH ELEVATION OF BACK OF SIDEWALK.
6. IN CASES WHERE EXISTING SIDEWALK IS DISTURBED FOR SIGNAL CONSTRUCTION, SIDEWALK SHALL BE REBUILT TO CITY STANDARDS AT CONTRACTORS EXPENSE.

STREET NAME SIGNS

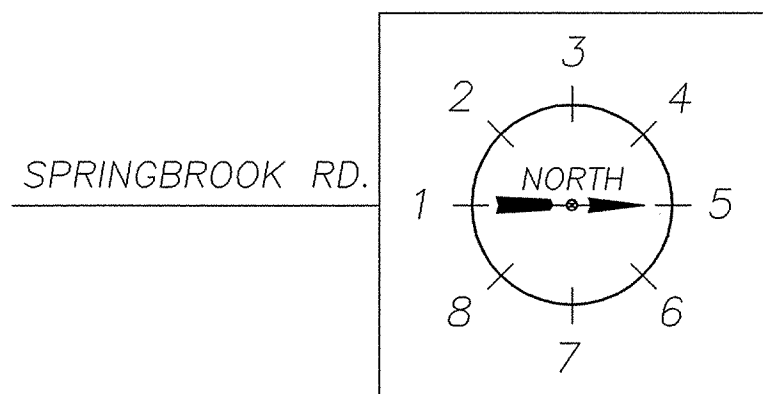
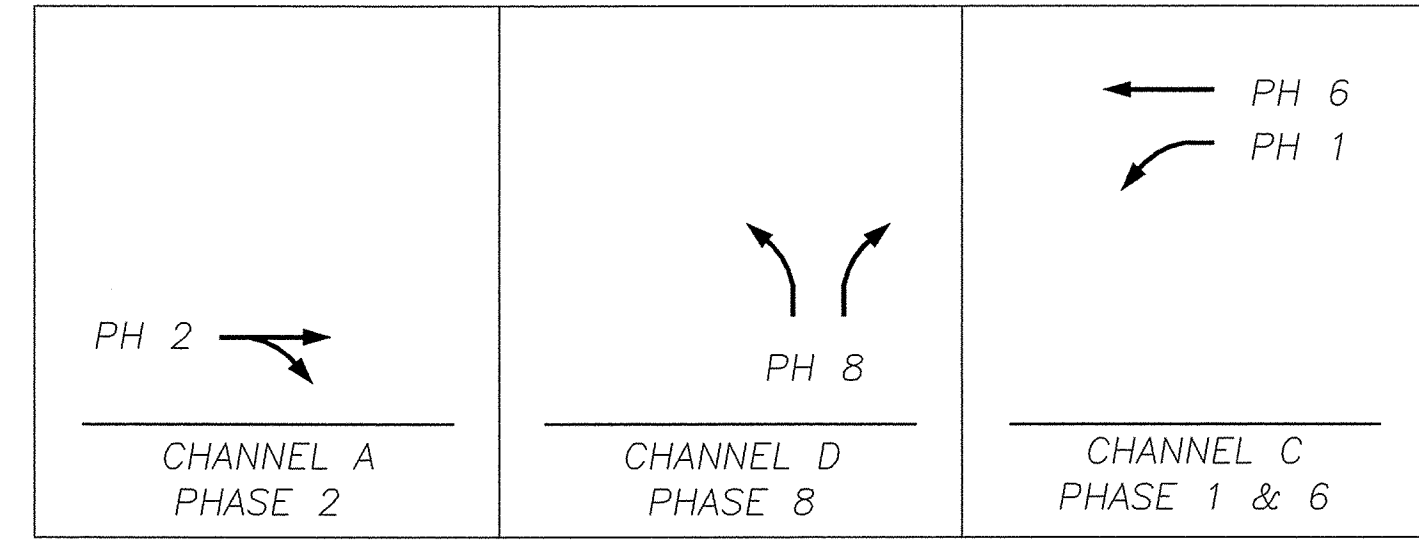


SIGNAL PHASING

NORMAL PHASE ROTATION



FIRE PRE-EMPTION OPERATION



REGISTERED PROFESSIONAL ENGINEER
 NOV 17 2006
 FRANK R. CHARBONNEAU

11-14-06
 REGISTERED PROFESSIONAL ENGINEER
 9301
 FRANK R. CHARBONNEAU
 OREGON
 JULY 22, 1977
 EXPIRES: 12/31/07

A2006017A

AS-BUILT DRAWING
 DATE PLOTTED: Nov 14, 2006 - 2:55pm

TRAFFIC SIGNAL INSTALLATION PLANS
SPRINGBROOK RD AT HAYES ST.
 SIGNAL LEGEND AND DETAILS
 CITY OF NEWBERG

Job Number
03-49
 Sheet
1 of 3
 OREGON

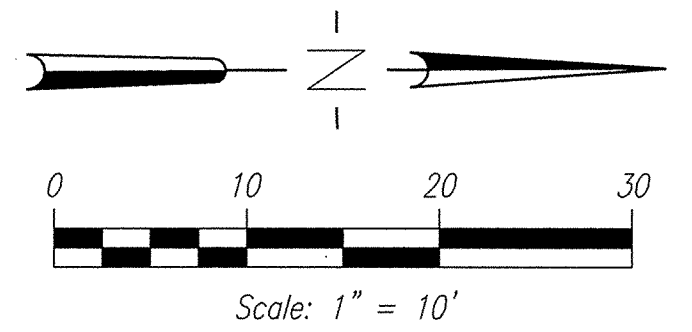
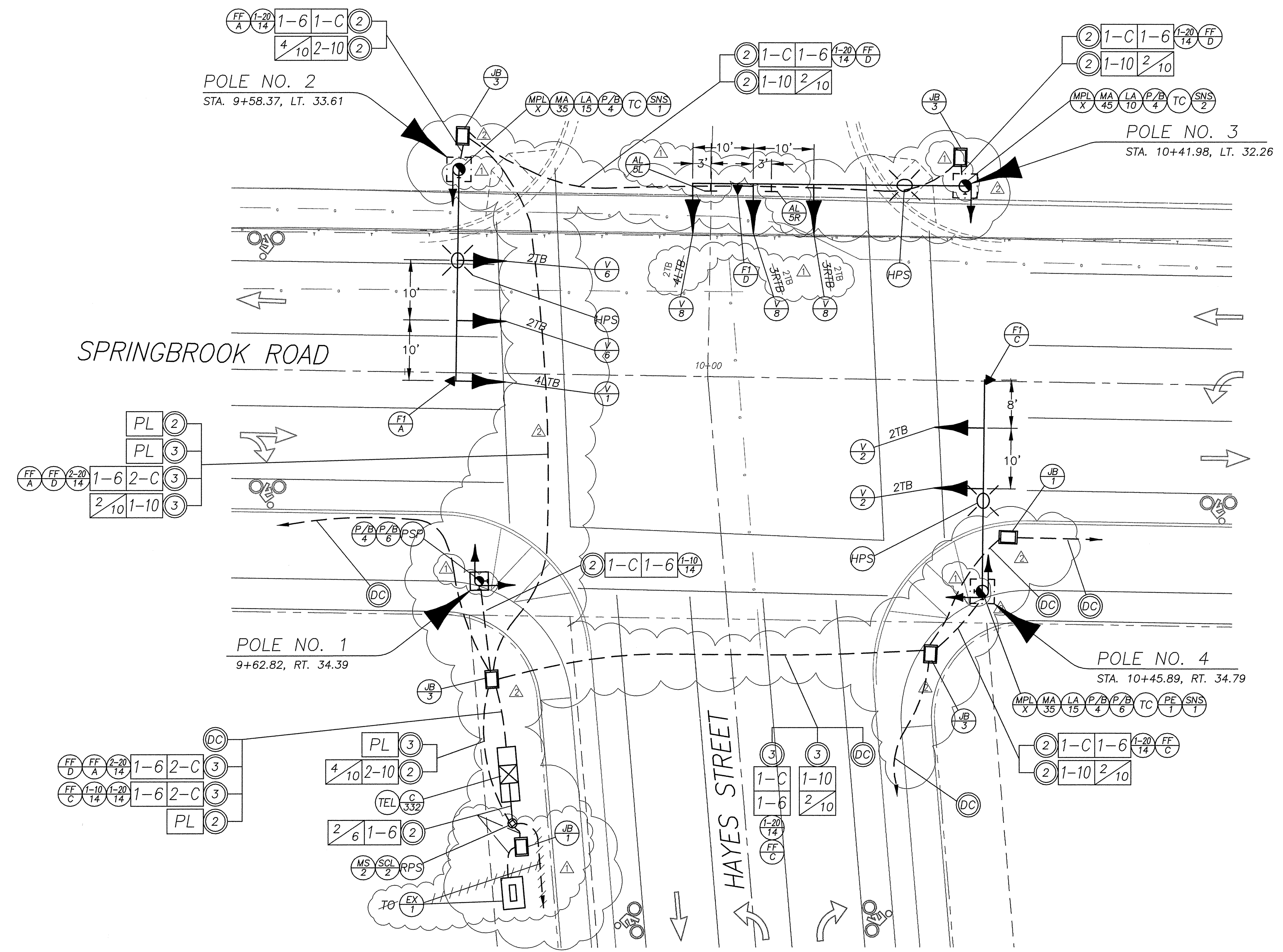
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REVISIONS	
	8/16/06 REVISED AS CONSTRUCTED
	11/14/06 REVISED AS CONSTRUCTED

ALPHA ENGINEERING INC.
 PLANNING • DEVELOPMENT SERVICES • SURVEYING
 OFFICE 503-452-8003 • FAX 503-452-8043
 PLAZA WEST • SUITE 230 • 9600 SW OAK • PORTLAND, OR 97223

CHARBONNEAU ENGINEERING LLC
 9370 SW Greenburg Road, Suite 411 (503) 293-1118
 Portland, OR 97223 FAX (503) 293-1119
 344/690

Date	03/30/04
Scale	N.T.S.
Designed By	FRC
Drawn By	AJR/MEK
Checked By	FRC



DATE PLOTTED: Nov 14, 2006 - 2:44pm CE DRAWING FILE: C:\Drawings\2006\Charbonneau\03-49 Hayes St. at Springbrook Rd\0349signal.dwg

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▲	11/14/06 REVISED AS CONSTRUCTED

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 9370 SW Greenburg Road, Suite 411 (503) 293-1118
 Portland, OR 97223 FAX (503) 293-1119
 345/690

Date: 03/30/04
 Scale: AS SHOWN
 Designed By: FRC
 Drawn By: RFD, JRC
 Checked By: FRC

A20060017B

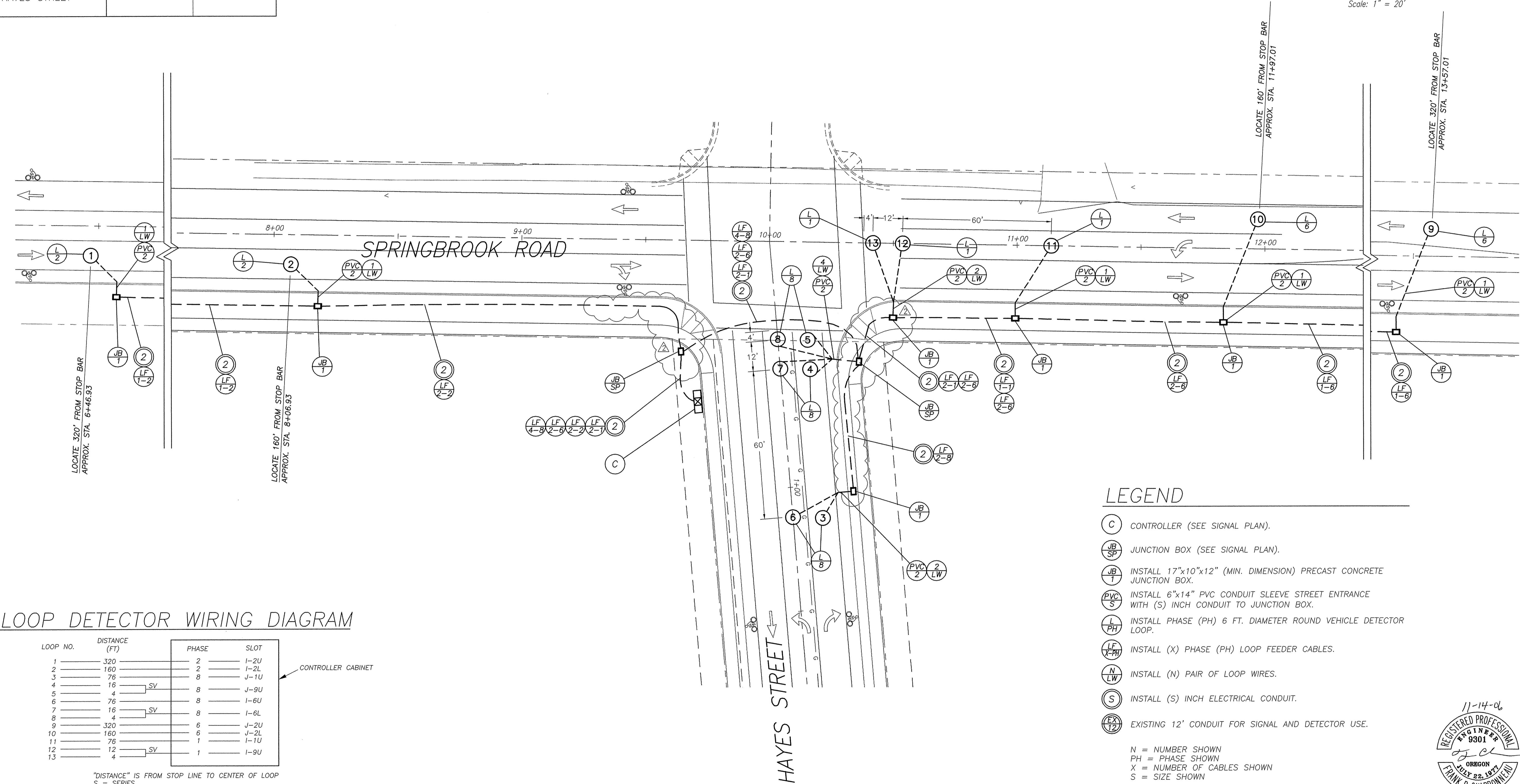
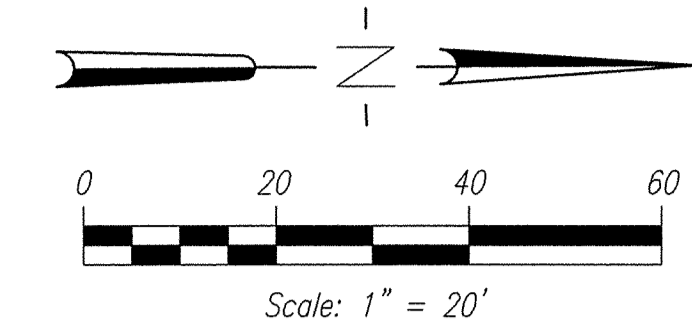
AS-BUILT DRAWING
 DATE PLOTTED: Nov 14, 2006 - 2:44pm
 TRAFFIC SIGNAL INSTALLATION PLANS
SPRINGBROOK ROAD AT HAYES STREET
 SIGNAL PLAN
 CITY OF NEWBERG

11-14-06
 REGISTERED PROFESSIONAL ENGINEER
 9301
 FRANK R. CHARBONNEAU
 OREGON JULY 22, 1971
 EXPIRES: 12/31/07

Job Number: 03-49
 Sheet: 2 of 3
 OREGON

DESIGN APPROACH SPEEDS

STREET	APPROACH	DESIGN SPEED
SPRINGBROOK ROAD	NORTHBOUND	40 MPH
	SOUTHBOUND	40 MPH
HAYES STREET	WESTBOUND	25 MPH



LOOP DETECTOR WIRING DIAGRAM

LOOP NO.	DISTANCE (FT)	PHASE	SLOT
1	320	2	I-2U
2	160	2	I-2L
3	76	8	J-1U
4	16	8	J-9U
5	4	8	I-6U
6	76	8	I-6L
7	16	8	J-2U
8	4	6	J-2L
9	320	1	I-1U
10	160	1	I-9U
11	76	1	I-9U
12	12	1	I-9U
13	4	1	I-9U

CONTROLLER CABINET

"DISTANCE" IS FROM STOP LINE TO CENTER OF LOOP
S = SERIES

LEGEND

- (C) CONTROLLER (SEE SIGNAL PLAN).
 - (JB SP) JUNCTION BOX (SEE SIGNAL PLAN).
 - (JB T) INSTALL 17"x10"x12" (MIN. DIMENSION) PRECAST CONCRETE JUNCTION BOX.
 - (PVC S) INSTALL 6"x14" PVC CONDUIT SLEEVE STREET ENTRANCE WITH (S) INCH CONDUIT TO JUNCTION BOX.
 - (L PH) INSTALL PHASE (PH) 6 FT. DIAMETER ROUND VEHICLE DETECTOR LOOP.
 - (LF X-PH) INSTALL (X) PHASE (PH) LOOP FEEDER CABLES.
 - (LW N) INSTALL (N) PAIR OF LOOP WIRES.
 - (S) INSTALL (S) INCH ELECTRICAL CONDUIT.
 - (EX 12) EXISTING 12" CONDUIT FOR SIGNAL AND DETECTOR USE.
- N = NUMBER SHOWN
PH = PHASE SHOWN
X = NUMBER OF CABLES SHOWN
S = SIZE SHOWN



AS-BUILT DRAWING
DATE PLOTTED: Nov 14, 2006 - 2:49pm

AZ0060111C

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Date	03/30/04
Scale	AS SHOWN
Designed By	FRC
Drawn By	RFD
Checked By	FRC

TRAFFIC SIGNAL INSTALLATION PLANS
SPRINGBROOK ROAD AT HAYES STREET
DETECTOR PLAN
CITY OF NEWBERG

Job Number
03-49
Sheet
3 of 3
OREGON

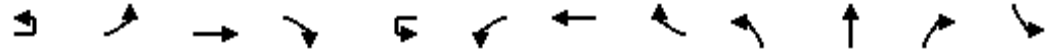
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APPENDIX H
**SYNCRHO HCM 2000
REPORTS**

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↔	↕	↗		↔	↕	↗	↖	↕	↗	↖
Volume (vph)	4	97	1095	61	11	144	1475	273	346	198	143	346
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5		5.0	5.5	5.5	5.0	5.0	5.0	5.0
Lane Util. Factor		1.00	0.95	1.00		1.00	0.95	1.00	0.97	1.00	1.00	0.97
Frbp, ped/bikes		1.00	1.00	0.98		1.00	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		1787	3406	1438		1719	3505	1519	3433	1863	1455	3433
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		1787	3406	1438		1719	3505	1519	3433	1863	1455	3433
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	4	99	1117	62	11	147	1505	279	353	202	146	353
RTOR Reduction (vph)	0	0	0	35	0	0	0	113	0	0	128	0
Lane Group Flow (vph)	0	103	1117	27	0	158	1505	166	353	202	18	353
Confl. Peds. (#/hr)								1				
Confl. Bikes (#/hr)				1								
Heavy Vehicles (%)	1%	1%	6%	10%	5%	5%	3%	4%	2%	2%	11%	2%
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	5	5	2		1	1	6		3	8		7
Permitted Phases				2			6				8	
Actuated Green, G (s)		11.4	52.5	52.5		19.0	60.1	60.1	16.5	15.0	15.0	15.0
Effective Green, g (s)		10.9	52.0	52.0		18.5	59.6	59.6	16.0	14.5	14.5	14.5
Actuated g/C Ratio		0.09	0.43	0.43		0.15	0.50	0.50	0.13	0.12	0.12	0.12
Clearance Time (s)		4.5	5.0	5.0		4.5	5.0	5.0	4.5	4.5	4.5	4.5
Vehicle Extension (s)		2.3	4.2	4.2		2.5	4.2	4.2	2.3	2.3	2.3	2.3
Lane Grp Cap (vph)		162	1475	623		265	1740	754	457	225	175	414
v/s Ratio Prot		0.06	c0.33			0.09	c0.43		c0.10	c0.11		0.10
v/s Ratio Perm				0.02				0.11			0.01	
v/c Ratio		0.64	0.76	0.04		0.60	0.86	0.22	0.77	0.90	0.10	0.85
Uniform Delay, d1		52.6	28.7	19.6		47.3	26.6	17.1	50.2	52.0	46.9	51.7
Progression Factor		1.00	1.00	1.00		0.88	0.79	0.54	1.00	1.00	1.00	1.00
Incremental Delay, d2		6.5	3.7	0.1		2.0	4.2	0.5	7.5	33.2	0.1	15.2
Delay (s)		59.2	32.4	19.8		43.6	25.4	9.6	57.7	85.2	47.1	66.9
Level of Service		E	C	B		D	C	A	E	F	D	E
Approach Delay (s)			33.9				24.6			63.4		
Approach LOS			C				C			E		
Intersection Summary												
HCM 2000 Control Delay			38.6				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.87									
Actuated Cycle Length (s)			120.0				Sum of lost time (s)			20.5		
Intersection Capacity Utilization			85.9%				ICU Level of Service			E		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	SBT	SBR
Lane Configurations	↑	↑
Volume (vph)	143	114
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	5.0
Lane Util. Factor	1.00	1.00
Frbp, ped/bikes	1.00	0.98
Flpb, ped/bikes	1.00	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	1727	1515
Flt Permitted	1.00	1.00
Satd. Flow (perm)	1727	1515
Peak-hour factor, PHF	0.98	0.98
Adj. Flow (vph)	146	116
RTOR Reduction (vph)	0	103
Lane Group Flow (vph)	146	13
Confl. Peds. (#/hr)		9
Confl. Bikes (#/hr)		1
Heavy Vehicles (%)	10%	4%
Turn Type	NA	Perm
Protected Phases	4	
Permitted Phases		4
Actuated Green, G (s)	13.5	13.5
Effective Green, g (s)	13.0	13.0
Actuated g/C Ratio	0.11	0.11
Clearance Time (s)	4.5	4.5
Vehicle Extension (s)	2.3	2.3
Lane Grp Cap (vph)	187	164
v/s Ratio Prot	0.08	
v/s Ratio Perm		0.01
v/c Ratio	0.78	0.08
Uniform Delay, d1	52.1	48.1
Progression Factor	1.00	1.00
Incremental Delay, d2	17.9	0.1
Delay (s)	70.0	48.2
Level of Service	E	D
Approach Delay (s)	64.1	
Approach LOS	E	
Intersection Summary		

HCM Unsignalized Intersection Capacity Analysis
2: FM Right-In Driveway & Highway 99W

11/21/2014



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑↑		
Volume (veh/h)	1394	218	0	1930	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1437	225	0	1990	0	0
Pedestrians					1	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	478			898		
pX, platoon unblocked				0.74	0.86	0.74
vC, conflicting volume	1438			2101	720	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	876			174	0	
tC, single (s)	4.1			6.8	6.9	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	100			100	100	
cM capacity (veh/h)	573			695	797	

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3
Volume Total	719	719	225	663	663	663
Volume Left	0	0	0	0	0	0
Volume Right	0	0	225	0	0	0
cSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.42	0.42	0.13	0.39	0.39	0.39
Queue Length 95th (ft)	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						

Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	42.3%			ICU Level of Service	A	
Analysis Period (min)	15					

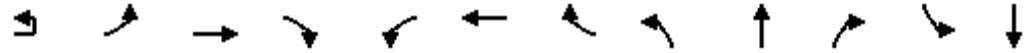
HCM Unsignalized Intersection Capacity Analysis 3: FM RIRO Driveway & Highway 99W

11/21/2014

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑		↑
Volume (veh/h)	1247	138	0	1919	0	23
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1286	142	0	1978	0	24
Pedestrians						1
Lane Width (ft)						12.0
Walking Speed (ft/s)						4.0
Percent Blockage						0
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	885			491		
pX, platoon unblocked			0.76	0.77	0.76	
vC, conflicting volume			1287	2276	644	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			757	674	0	
tC, single (s)			4.1	6.8	6.9	
tC, 2 stage (s)						
tF (s)			2.2	3.5	3.3	
p0 queue free %			100	100	97	
cM capacity (veh/h)			659	301	833	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	643	643	142	989	989	24
Volume Left	0	0	0	0	0	0
Volume Right	0	0	142	0	0	24
cSH	1700	1700	1700	1700	1700	833
Volume to Capacity	0.38	0.38	0.08	0.58	0.58	0.03
Queue Length 95th (ft)	0	0	0	0	0	2
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.5
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.5
Approach LOS						A
Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			56.8%	ICU Level of Service		B
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis
4: Brutscher Street & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔	↕	↗	↔	↕	↗	↖	↕		↖	↗
Volume (vph)	6	20	1177	74	232	1621	41	211	13	111	14	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5	5.0	5.5	5.5	5.0	5.0		5.0	5.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes		1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.99		1.00	0.99
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Frt		1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.87		1.00	0.88
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)		1805	3406	1462	1752	3471	1582	1752	1543		1805	1587
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00	0.71	1.00		0.59	1.00
Satd. Flow (perm)		1805	3406	1462	1752	3471	1582	1311	1543		1126	1587
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	6	21	1226	77	242	1689	43	220	14	116	15	16
RTOR Reduction (vph)	0	0	0	31	0	0	15	0	95	0	0	45
Lane Group Flow (vph)	0	27	1226	46	242	1689	28	220	35	0	15	26
Confl. Peds. (#/hr)				1						1		
Confl. Bikes (#/hr)							1			1		
Heavy Vehicles (%)	0%	0%	6%	8%	3%	4%	0%	3%	8%	5%	0%	7%
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA
Protected Phases	5	5	2		1	6			8			4
Permitted Phases				2			6	8				4
Actuated Green, G (s)		4.6	65.6	65.6	18.0	79.0	79.0	22.4	22.4		22.4	22.4
Effective Green, g (s)		4.1	65.1	65.1	17.5	78.5	78.5	21.9	21.9		21.9	21.9
Actuated g/C Ratio		0.03	0.54	0.54	0.15	0.65	0.65	0.18	0.18		0.18	0.18
Clearance Time (s)		4.5	5.0	5.0	4.5	5.0	5.0	4.5	4.5		4.5	4.5
Vehicle Extension (s)		2.3	4.8	4.8	2.3	4.8	4.8	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)		61	1847	793	255	2270	1034	239	281		205	289
v/s Ratio Prot		0.01	0.36		c0.14	c0.49			0.02			0.02
v/s Ratio Perm				0.03			0.02	c0.17			0.01	
v/c Ratio		0.44	0.66	0.06	0.95	0.74	0.03	0.92	0.13		0.07	0.09
Uniform Delay, d1		56.8	19.6	13.0	50.8	14.0	7.3	48.2	41.0		40.6	40.8
Progression Factor		0.81	1.06	1.87	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2		2.2	1.4	0.1	41.8	2.3	0.0	37.3	0.1		0.1	0.1
Delay (s)		48.2	22.2	24.4	92.6	16.2	7.4	85.4	41.2		40.8	40.9
Level of Service		D	C	C	F	B	A	F	D		D	D
Approach Delay (s)			22.8			25.4			69.0			40.8
Approach LOS			C			C			E			D

Intersection Summary		
HCM 2000 Control Delay	28.9	HCM 2000 Level of Service C
HCM 2000 Volume to Capacity ratio	0.83	
Actuated Cycle Length (s)	120.0	Sum of lost time (s) 15.5
Intersection Capacity Utilization	79.7%	ICU Level of Service D
Analysis Period (min)	15	

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014

Movement	SBR
Lane Configurations	
Volume (vph)	53
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frbp, ped/bikes	
Flpb, ped/bikes	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.96
Adj. Flow (vph)	55
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Confl. Peds. (#/hr)	2
Confl. Bikes (#/hr)	
Heavy Vehicles (%)	4%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM Unsignalized Intersection Capacity Analysis

5: Springbrook Road & FM Main Drive Aisle

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (veh/h)	0	351	335	113	0	353
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	373	356	120	0	376
Pedestrians			1			1
Lane Width (ft)			12.0			12.0
Walking Speed (ft/s)			4.0			4.0
Percent Blockage			0			0
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (ft)			717			401
pX, platoon unblocked	0.96	0.93			0.93	
vC, conflicting volume	793	417			356	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	591	331			265	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	43			100	
cM capacity (veh/h)	454	660			1214	




















Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	373	477	376
Volume Left	0	0	0
Volume Right	373	120	0
cSH	660	1700	1700
Volume to Capacity	0.57	0.28	0.22
Queue Length 95th (ft)	89	0	0
Control Delay (s)	17.3	0.0	0.0
Lane LOS	C		
Approach Delay (s)	17.3	0.0	0.0
Approach LOS	C		

Intersection Summary			
Average Delay		5.3	
Intersection Capacity Utilization		53.9%	ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

6: FM Fuel/US Bank & FM Main Drive Aisle

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	20	62	44	151	159	65	137	5	48	15	5	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	67	48	164	173	71	149	5	52	16	5	27
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	SB 1							
Volume Total (vph)	22	115	408	207	49							
Volume Left (vph)	22	0	164	149	16							
Volume Right (vph)	0	48	71	52	27							
Hadj (s)	0.50	-0.27	0.01	0.03	-0.19							
Departure Headway (s)	6.2	5.4	4.9	5.4	5.5							
Degree Utilization, x	0.04	0.17	0.56	0.31	0.07							
Capacity (veh/h)	539	619	702	608	572							
Control Delay (s)	8.2	8.3	14.1	10.8	8.9							
Approach Delay (s)	8.3		14.1	10.8	8.9							
Approach LOS	A		B	B	A							
Intersection Summary												
Delay			11.9									
Level of Service			B									
Intersection Capacity Utilization			52.4%		ICU Level of Service		A					
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

7: FM Main Drive Aisle & FM Right-In Driveway

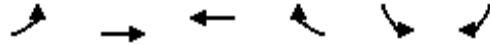
11/21/2014



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Sign Control		Stop	Stop		Stop	
Volume (vph)	0	132	250	0	105	113
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	147	278	0	117	126
Direction, Lane #	EB 1	WB 1	SB 1	SB 2		
Volume Total (vph)	147	278	117	126		
Volume Left (vph)	0	0	117	0		
Volume Right (vph)	0	0	0	126		
Hadj (s)	0.03	0.02	0.50	-0.63		
Departure Headway (s)	4.9	4.7	6.0	4.9		
Degree Utilization, x	0.20	0.37	0.20	0.17		
Capacity (veh/h)	687	725	564	687		
Control Delay (s)	9.2	10.5	9.3	7.7		
Approach Delay (s)	9.2	10.5	8.5			
Approach LOS	A	B	A			
Intersection Summary						
Delay			9.4			
Level of Service			A			
Intersection Capacity Utilization			30.7%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 8: FM Main Drive Aisle & FM RIRO Driveway


















11/21/2014



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↶	↷		↘	
Sign Control		Stop	Stop		Stop	
Volume (vph)	18	122	148	5	42	96
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	20	139	168	6	48	109
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	159	174	157			
Volume Left (vph)	20	0	48			
Volume Right (vph)	0	6	109			
Hadj (s)	0.04	0.01	-0.34			
Departure Headway (s)	4.5	4.5	4.3			
Degree Utilization, x	0.20	0.22	0.19			
Capacity (veh/h)	767	768	778			
Control Delay (s)	8.6	8.7	8.3			
Approach Delay (s)	8.6	8.7	8.3			
Approach LOS	A	A	A			
Intersection Summary						
Delay			8.5			
Level of Service			A			
Intersection Capacity Utilization			39.2%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis
 9: Brutscher Street & FM Main Drive Aisle/Columbia Bank

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	187	4	47	8	14	27	14	116	8	38	129	152
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	203	4	51	9	15	29	15	126	9	41	140	165
Pedestrians		1			1							
Lane Width (ft)		12.0			12.0							
Walking Speed (ft/s)		4.0			4.0							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											342	
pX, platoon unblocked												
vC, conflicting volume	500	473	224	521	551	131	306			136		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	500	473	224	521	551	131	306			136		
tC, single (s)	7.1	6.5	6.2	7.3	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.7	4.0	3.3	2.2			2.2		
p0 queue free %	54	99	94	98	96	97	99			97		
cM capacity (veh/h)	438	472	815	389	426	923	1265			1441		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1							
Volume Total	259	53	15	135	347							
Volume Left	203	9	15	0	41							
Volume Right	51	29	0	9	165							
cSH	483	593	1265	1700	1441							
Volume to Capacity	0.54	0.09	0.01	0.08	0.03							
Queue Length 95th (ft)	78	7	1	0	2							
Control Delay (s)	20.7	11.7	7.9	0.0	1.1							
Lane LOS	C	B	A		A							
Approach Delay (s)	20.7	11.7	0.8		1.1							
Approach LOS	C	B										
Intersection Summary												
Average Delay			8.0									
Intersection Capacity Utilization			56.4%		ICU Level of Service				B			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

12: Springbrook Road & Hayes Street

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	88	81	353	38	69	265
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes	1.00	0.97	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1679	1557	1770		1703	1759
Flt Permitted	0.95	1.00	1.00		0.42	1.00
Satd. Flow (perm)	1679	1557	1770		761	1759
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	92	84	368	40	72	276
RTOR Reduction (vph)	0	74	5	0	0	0
Lane Group Flow (vph)	92	10	403	0	72	276
Confl. Peds. (#/hr)	3	4				
Confl. Bikes (#/hr)				1		
Heavy Vehicles (%)	7%	1%	6%	3%	6%	8%
Turn Type	Perm	Perm	NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8	8			6	
Actuated Green, G (s)	6.9	6.9	32.3		40.1	40.1
Effective Green, g (s)	6.4	6.4	31.8		39.6	39.6
Actuated g/C Ratio	0.12	0.12	0.58		0.72	0.72
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)	195	181	1023		604	1266
v/s Ratio Prot			c0.23		0.01	c0.16
v/s Ratio Perm	c0.05	0.01			0.08	
v/c Ratio	0.47	0.05	0.39		0.12	0.22
Uniform Delay, d1	22.7	21.6	6.3		2.7	2.6
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	1.3	0.1	1.1		0.1	0.4
Delay (s)	24.0	21.7	7.5		2.8	3.0
Level of Service	C	C	A		A	A
Approach Delay (s)	22.9		7.5			2.9
Approach LOS	C		A			A

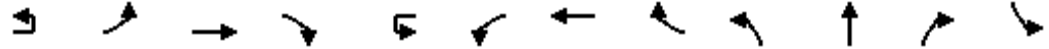
Intersection Summary

HCM 2000 Control Delay	8.7	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.40		
Actuated Cycle Length (s)	55.0	Sum of lost time (s)	13.0
Intersection Capacity Utilization	42.1%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
1: Springbrook Road & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↔	↕	↗		↔	↕	↗	↖	↕	↗	↖
Volume (vph)	4	99	1117	66	11	147	1505	278	357	204	146	353
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5		5.0	5.5	5.5	5.0	5.0	5.0	5.0
Lane Util. Factor		1.00	0.95	1.00		1.00	0.95	1.00	0.97	1.00	1.00	0.97
Frbp, ped/bikes		1.00	1.00	0.98		1.00	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		1787	3406	1438		1719	3505	1519	3433	1863	1455	3433
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		1787	3406	1438		1719	3505	1519	3433	1863	1455	3433
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	4	101	1140	67	11	150	1536	284	364	208	149	360
RTOR Reduction (vph)	0	0	0	38	0	0	0	113	0	0	131	0
Lane Group Flow (vph)	0	105	1140	29	0	161	1536	171	364	208	18	360
Confl. Peds. (#/hr)								1				
Confl. Bikes (#/hr)				1								
Heavy Vehicles (%)	1%	1%	6%	10%	5%	5%	3%	4%	2%	2%	11%	2%
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	5	5	2		1	1	6		3	8		7
Permitted Phases				2				6			8	
Actuated Green, G (s)		11.5	53.0	53.0		18.4	59.9	59.9	16.1	15.1	15.1	15.0
Effective Green, g (s)		11.0	52.5	52.5		17.9	59.4	59.4	15.6	14.6	14.6	14.5
Actuated g/C Ratio		0.09	0.44	0.44		0.15	0.49	0.49	0.13	0.12	0.12	0.12
Clearance Time (s)		4.5	5.0	5.0		4.5	5.0	5.0	4.5	4.5	4.5	4.5
Vehicle Extension (s)		2.3	4.2	4.2		2.5	4.2	4.2	2.3	2.3	2.3	2.3
Lane Grp Cap (vph)		163	1490	629		256	1734	751	446	226	177	414
v/s Ratio Prot		0.06	c0.33			0.09	c0.44		c0.11	c0.11		0.10
v/s Ratio Perm				0.02				0.11			0.01	
v/c Ratio		0.64	0.77	0.05		0.63	0.89	0.23	0.82	0.92	0.10	0.87
Uniform Delay, d1		52.6	28.5	19.4		47.9	27.3	17.2	50.8	52.1	46.9	51.8
Progression Factor		1.00	1.00	1.00		0.87	0.79	0.51	1.00	1.00	1.00	1.00
Incremental Delay, d2		7.0	3.8	0.1		2.8	4.9	0.5	10.6	38.4	0.1	17.1
Delay (s)		59.7	32.3	19.5		44.6	26.3	9.3	61.4	90.6	47.0	68.9
Level of Service		E	C	B		D	C	A	E	F	D	E
Approach Delay (s)			33.9				25.4			66.9		
Approach LOS			C				C			E		
Intersection Summary												
HCM 2000 Control Delay			39.8				HCM 2000 Level of Service				D	
HCM 2000 Volume to Capacity ratio			0.89									
Actuated Cycle Length (s)			120.0				Sum of lost time (s)				20.5	
Intersection Capacity Utilization			87.4%				ICU Level of Service				E	
Analysis Period (min)			15									

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	SBT	SBR
Lane Configurations	↑	↑
Volume (vph)	156	116
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	5.0
Lane Util. Factor	1.00	1.00
Frbp, ped/bikes	1.00	0.98
Flpb, ped/bikes	1.00	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	1727	1515
Flt Permitted	1.00	1.00
Satd. Flow (perm)	1727	1515
Peak-hour factor, PHF	0.98	0.98
Adj. Flow (vph)	159	118
RTOR Reduction (vph)	0	105
Lane Group Flow (vph)	159	13
Confl. Peds. (#/hr)		9
Confl. Bikes (#/hr)		1
Heavy Vehicles (%)	10%	4%
Turn Type	NA	Perm
Protected Phases	4	
Permitted Phases		4
Actuated Green, G (s)	14.0	14.0
Effective Green, g (s)	13.5	13.5
Actuated g/C Ratio	0.11	0.11
Clearance Time (s)	4.5	4.5
Vehicle Extension (s)	2.3	2.3
Lane Grp Cap (vph)	194	170
v/s Ratio Prot	0.09	
v/s Ratio Perm		0.01
v/c Ratio	0.82	0.08
Uniform Delay, d1	52.1	47.7
Progression Factor	1.00	1.00
Incremental Delay, d2	22.2	0.1
Delay (s)	74.2	47.8
Level of Service	E	D
Approach Delay (s)	66.3	
Approach LOS	E	
Intersection Summary		

HCM Unsignalized Intersection Capacity Analysis

2: FM Right-In Driveway & Highway 99W

11/21/2014

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑↑		
Volume (veh/h)	1422	218	0	1969	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1466	225	0	2030	0	0
Pedestrians					1	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	478			898		
pX, platoon unblocked				0.73	0.86	0.73
vC, conflicting volume				1467	2144	734
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				895	122	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	100
cM capacity (veh/h)				558	748	790
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3
Volume Total	733	733	225	677	677	677
Volume Left	0	0	0	0	0	0
Volume Right	0	0	225	0	0	0
cSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.43	0.43	0.13	0.40	0.40	0.40
Queue Length 95th (ft)	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay				0.0		
Intersection Capacity Utilization				43.1%	ICU Level of Service	A
Analysis Period (min)				15		

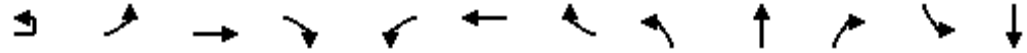
HCM Unsignalized Intersection Capacity Analysis 3: FM RIRO Driveway & Highway 99W

11/21/2014

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Volume (veh/h)	1272	138	0	1957	0	23
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1311	142	0	2018	0	24
Pedestrians						1
Lane Width (ft)						12.0
Walking Speed (ft/s)						4.0
Percent Blockage						0
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	885			491		
pX, platoon unblocked				0.76	0.76	0.76
vC, conflicting volume				1312	2321	657
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				770	652	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	97
cM capacity (veh/h)				645	306	825
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	656	656	142	1009	1009	24
Volume Left	0	0	0	0	0	0
Volume Right	0	0	142	0	0	24
cSH	1700	1700	1700	1700	1700	825
Volume to Capacity	0.39	0.39	0.08	0.59	0.59	0.03
Queue Length 95th (ft)	0	0	0	0	0	2
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.5
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.5
Approach LOS						A
Intersection Summary						
Average Delay				0.1		
Intersection Capacity Utilization				57.8%	ICU Level of Service	B
Analysis Period (min)				15		

HCM Signalized Intersection Capacity Analysis
4: Brutscher Street & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔	↕	↗	↔	↕	↗	↖	↕		↖	↗
Volume (vph)	6	20	1201	75	248	1653	42	215	13	116	14	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5	5.0	5.5	5.5	5.0	5.0		5.0	5.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes		1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.99		1.00	0.99
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Frt		1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.87		1.00	0.88
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)		1805	3406	1462	1752	3471	1582	1752	1542		1805	1588
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00	0.71	1.00		0.58	1.00
Satd. Flow (perm)		1805	3406	1462	1752	3471	1582	1309	1542		1105	1588
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	6	21	1251	78	258	1722	44	224	14	121	15	17
RTOR Reduction (vph)	0	0	0	31	0	0	15	0	99	0	0	46
Lane Group Flow (vph)	0	27	1251	47	258	1722	29	224	36	0	15	27
Confl. Peds. (#/hr)				1						1		
Confl. Bikes (#/hr)							1			1		
Heavy Vehicles (%)	0%	0%	6%	8%	3%	4%	0%	3%	8%	5%	0%	7%
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA
Protected Phases	5	5	2		1	6			8			4
Permitted Phases				2			6	8				4
Actuated Green, G (s)		4.6	64.6	64.6	18.8	78.8	78.8	22.6	22.6		22.6	22.6
Effective Green, g (s)		4.1	64.1	64.1	18.3	78.3	78.3	22.1	22.1		22.1	22.1
Actuated g/C Ratio		0.03	0.53	0.53	0.15	0.65	0.65	0.18	0.18		0.18	0.18
Clearance Time (s)		4.5	5.0	5.0	4.5	5.0	5.0	4.5	4.5		4.5	4.5
Vehicle Extension (s)		2.3	4.8	4.8	2.3	4.8	4.8	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)		61	1819	780	267	2264	1032	241	283		203	292
v/s Ratio Prot		0.01	0.37		c0.15	c0.50			0.02			0.02
v/s Ratio Perm				0.03			0.02	c0.17			0.01	
v/c Ratio		0.44	0.69	0.06	0.97	0.76	0.03	0.93	0.13		0.07	0.09
Uniform Delay, d1		56.8	20.6	13.5	50.5	14.4	7.4	48.2	40.9		40.5	40.6
Progression Factor		0.80	1.04	1.89	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2		2.2	1.6	0.1	45.3	2.5	0.0	38.7	0.1		0.1	0.1
Delay (s)		47.6	22.9	25.6	95.8	16.9	7.4	86.9	41.1		40.6	40.7
Level of Service		D	C	C	F	B	A	F	D		D	D
Approach Delay (s)			23.5			26.7			69.6			40.7
Approach LOS			C			C			E			D

Intersection Summary		
HCM 2000 Control Delay	29.9	HCM 2000 Level of Service C
HCM 2000 Volume to Capacity ratio	0.85	
Actuated Cycle Length (s)	120.0	Sum of lost time (s) 15.5
Intersection Capacity Utilization	80.8%	ICU Level of Service D
Analysis Period (min)	15	

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014

Movement	SBR
Lane Configurations	
Volume (vph)	54
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frbp, ped/bikes	
Flpb, ped/bikes	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.96
Adj. Flow (vph)	56
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Confl. Peds. (#/hr)	2
Confl. Bikes (#/hr)	
Heavy Vehicles (%)	4%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM Unsignalized Intersection Capacity Analysis

5: Springbrook Road & FM Main Drive Aisle

11/21/2014




















Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↘			↕
Volume (veh/h)	0	351	348	114	0	374
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	373	370	121	0	398
Pedestrians			1			1
Lane Width (ft)			12.0			12.0
Walking Speed (ft/s)			4.0			4.0
Percent Blockage			0			0
Right turn flare (veh)						
Median type			None			None
Median storage veh						
Upstream signal (ft)			717			401
pX, platoon unblocked	0.96	0.92			0.92	
vC, conflicting volume	830	432			370	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	603	334			267	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	43			100	
cM capacity (veh/h)	445	650			1199	

Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	373	491	398
Volume Left	0	0	0
Volume Right	373	121	0
cSH	650	1700	1700
Volume to Capacity	0.57	0.29	0.23
Queue Length 95th (ft)	92	0	0
Control Delay (s)	17.7	0.0	0.0
Lane LOS	C		
Approach Delay (s)	17.7	0.0	0.0
Approach LOS	C		

Intersection Summary			
Average Delay		5.2	
Intersection Capacity Utilization		54.6%	ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis
 6: FM Fuel/US Bank & FM Main Drive Aisle

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop				Stop
Volume (vph)	20	62	44	151	159	65	137	5	48	15	5	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	67	48	164	173	71	149	5	52	16	5	27
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	SB 1							
Volume Total (vph)	22	115	408	207	49							
Volume Left (vph)	22	0	164	149	16							
Volume Right (vph)	0	48	71	52	27							
Hadj (s)	0.50	-0.27	0.01	0.03	-0.19							
Departure Headway (s)	6.2	5.4	4.9	5.4	5.5							
Degree Utilization, x	0.04	0.17	0.56	0.31	0.07							
Capacity (veh/h)	539	619	702	608	572							
Control Delay (s)	8.2	8.3	14.1	10.8	8.9							
Approach Delay (s)	8.3		14.1	10.8	8.9							
Approach LOS	A		B	B	A							
Intersection Summary												
Delay			11.9									
Level of Service			B									
Intersection Capacity Utilization			52.4%		ICU Level of Service		A					
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

7: FM Main Drive Aisle & FM Right-In Driveway

11/21/2014



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Sign Control		Stop	Stop		Stop	
Volume (vph)	0	133	250	0	105	113
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	148	278	0	117	126
Direction, Lane #	EB 1	WB 1	SB 1	SB 2		
Volume Total (vph)	148	278	117	126		
Volume Left (vph)	0	0	117	0		
Volume Right (vph)	0	0	0	126		
Hadj (s)	0.03	0.02	0.50	-0.63		
Departure Headway (s)	4.9	4.7	6.0	4.9		
Degree Utilization, x	0.20	0.37	0.20	0.17		
Capacity (veh/h)	687	725	564	687		
Control Delay (s)	9.2	10.5	9.3	7.7		
Approach Delay (s)	9.2	10.5	8.5			
Approach LOS	A	B	A			
Intersection Summary						
Delay			9.5			
Level of Service			A			
Intersection Capacity Utilization			30.7%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

8: FM Main Drive Aisle & FM RIRO Driveway

11/21/2014




















Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Volume (vph)	18	122	148	5	42	96
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	20	139	168	6	48	109
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	159	174	157			
Volume Left (vph)	20	0	48			
Volume Right (vph)	0	6	109			
Hadj (s)	0.04	0.01	-0.34			
Departure Headway (s)	4.5	4.5	4.3			
Degree Utilization, x	0.20	0.22	0.19			
Capacity (veh/h)	767	768	778			
Control Delay (s)	8.6	8.7	8.3			
Approach Delay (s)	8.6	8.7	8.3			
Approach LOS	A	A	A			
Intersection Summary						
Delay			8.5			
Level of Service			A			
Intersection Capacity Utilization			39.2%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Brutscher Street & FM Main Drive Aisle/Columbia Bank

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	187	4	51	8	14	27	16	121	8	38	144	152
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	203	4	55	9	15	29	17	132	9	41	157	165
Pedestrians		1			1							
Lane Width (ft)		12.0			12.0							
Walking Speed (ft/s)		4.0			4.0							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											342	
pX, platoon unblocked												
vC, conflicting volume	526	499	240	551	577	137	323			141		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	526	499	240	551	577	137	323			141		
tC, single (s)	7.1	6.5	6.2	7.3	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.7	4.0	3.3	2.2			2.2		
p0 queue free %	52	99	93	98	96	97	99			97		
cM capacity (veh/h)	420	455	798	367	411	916	1247			1434		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1							
Volume Total	263	53	17	140	363							
Volume Left	203	9	17	0	41							
Volume Right	55	29	0	9	165							
cSH	467	574	1247	1700	1434							
Volume to Capacity	0.56	0.09	0.01	0.08	0.03							
Queue Length 95th (ft)	85	8	1	0	2							
Control Delay (s)	22.2	11.9	7.9	0.0	1.1							
Lane LOS	C	B	A		A							
Approach Delay (s)	22.2	11.9	0.9		1.1							
Approach LOS	C	B										
Intersection Summary												
Average Delay			8.4									
Intersection Capacity Utilization			57.7%		ICU Level of Service				B			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

12: Springbrook Road & Hayes Street

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	90	83	367	39	70	284
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes	1.00	0.97	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1679	1557	1770		1703	1759
Flt Permitted	0.95	1.00	1.00		0.41	1.00
Satd. Flow (perm)	1679	1557	1770		741	1759
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	94	86	382	41	73	296
RTOR Reduction (vph)	0	76	5	0	0	0
Lane Group Flow (vph)	94	10	418	0	73	296
Confl. Peds. (#/hr)	3	4				
Confl. Bikes (#/hr)				1		
Heavy Vehicles (%)	7%	1%	6%	3%	6%	8%
Turn Type	Perm	Perm	NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8	8			6	
Actuated Green, G (s)	7.0	7.0	32.2		40.0	40.0
Effective Green, g (s)	6.5	6.5	31.7		39.5	39.5
Actuated g/C Ratio	0.12	0.12	0.58		0.72	0.72
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)	198	184	1020		589	1263
v/s Ratio Prot			c0.24		0.01	c0.17
v/s Ratio Perm	c0.06	0.01			0.08	
v/c Ratio	0.47	0.06	0.41		0.12	0.23
Uniform Delay, d1	22.7	21.5	6.5		2.8	2.6
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	1.3	0.1	1.2		0.1	0.4
Delay (s)	24.0	21.6	7.7		2.9	3.1
Level of Service	C	C	A		A	A
Approach Delay (s)	22.8		7.7			3.0
Approach LOS	C		A			A

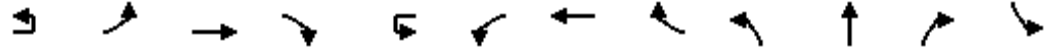
Intersection Summary

HCM 2000 Control Delay	8.7	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.41		
Actuated Cycle Length (s)	55.0	Sum of lost time (s)	13.0
Intersection Capacity Utilization	43.0%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
1: Springbrook Road & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	
Lane Configurations		↔	↕	↗		↔	↕	↗	↖	↕	↗	↖	
Volume (vph)	4	99	1121	66	11	147	1502	277	364	207	147	357	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		5.0	5.5	5.5		5.0	5.5	5.5	5.0	5.0	5.0	5.0	
Lane Util. Factor		1.00	0.95	1.00		1.00	0.95	1.00	0.97	1.00	1.00	0.97	
Frbp, ped/bikes		1.00	1.00	0.98		1.00	1.00	0.98	1.00	1.00	1.00	1.00	
Flpb, ped/bikes		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00	
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Satd. Flow (prot)		1787	3406	1438		1719	3505	1519	3433	1863	1455	3433	
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95	
Satd. Flow (perm)		1787	3406	1438		1719	3505	1519	3433	1863	1455	3433	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	
Adj. Flow (vph)	4	101	1144	67	11	150	1533	283	371	211	150	364	
RTOR Reduction (vph)	0	0	0	38	0	0	0	113	0	0	132	0	
Lane Group Flow (vph)	0	105	1144	29	0	161	1533	170	371	211	18	364	
Confl. Peds. (#/hr)								1					
Confl. Bikes (#/hr)				1									
Heavy Vehicles (%)	1%	1%	6%	10%	5%	5%	3%	4%	2%	2%	11%	2%	
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot	
Protected Phases	5	5	2		1	1	6		3	8		7	
Permitted Phases				2			6				8		
Actuated Green, G (s)		11.5	52.9	52.9		18.3	59.7	59.7	16.3	15.2	15.2	15.1	
Effective Green, g (s)		11.0	52.4	52.4		17.8	59.2	59.2	15.8	14.7	14.7	14.6	
Actuated g/C Ratio		0.09	0.44	0.44		0.15	0.49	0.49	0.13	0.12	0.12	0.12	
Clearance Time (s)		4.5	5.0	5.0		4.5	5.0	5.0	4.5	4.5	4.5	4.5	
Vehicle Extension (s)		2.3	4.2	4.2		2.5	4.2	4.2	2.3	2.3	2.3	2.3	
Lane Grp Cap (vph)		163	1487	627		254	1729	749	452	228	178	417	
v/s Ratio Prot		0.06	c0.34			0.09	c0.44		c0.11	c0.11		0.11	
v/s Ratio Perm				0.02				0.11			0.01		
v/c Ratio		0.64	0.77	0.05		0.63	0.89	0.23	0.82	0.93	0.10	0.87	
Uniform Delay, d1		52.6	28.7	19.4		48.0	27.4	17.3	50.7	52.1	46.8	51.8	
Progression Factor		1.00	1.00	1.00		0.87	0.79	0.52	1.00	1.00	1.00	1.00	
Incremental Delay, d2		7.0	3.9	0.1		3.0	4.9	0.5	11.0	39.1	0.1	17.6	
Delay (s)		59.7	32.6	19.6		45.0	26.5	9.4	61.8	91.3	46.9	69.4	
Level of Service		E	C	B		D	C	A	E	F	D	E	
Approach Delay (s)			34.1			25.6			67.2				
Approach LOS			C			C			E				
Intersection Summary													
HCM 2000 Control Delay			40.1		HCM 2000 Level of Service					D			
HCM 2000 Volume to Capacity ratio			0.90										
Actuated Cycle Length (s)			120.0		Sum of lost time (s)					20.5			
Intersection Capacity Utilization			87.4%		ICU Level of Service					E			
Analysis Period (min)			15										

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	SBT	SBR
Lane Configurations	↑	↑
Volume (vph)	155	116
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	5.0
Lane Util. Factor	1.00	1.00
Frbp, ped/bikes	1.00	0.98
Flpb, ped/bikes	1.00	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	1727	1515
Flt Permitted	1.00	1.00
Satd. Flow (perm)	1727	1515
Peak-hour factor, PHF	0.98	0.98
Adj. Flow (vph)	158	118
RTOR Reduction (vph)	0	105
Lane Group Flow (vph)	158	13
Confl. Peds. (#/hr)		9
Confl. Bikes (#/hr)		1
Heavy Vehicles (%)	10%	4%
Turn Type	NA	Perm
Protected Phases	4	
Permitted Phases		4
Actuated Green, G (s)	14.0	14.0
Effective Green, g (s)	13.5	13.5
Actuated g/C Ratio	0.11	0.11
Clearance Time (s)	4.5	4.5
Vehicle Extension (s)	2.3	2.3
Lane Grp Cap (vph)	194	170
v/s Ratio Prot	0.09	
v/s Ratio Perm		0.01
v/c Ratio	0.81	0.08
Uniform Delay, d1	52.0	47.7
Progression Factor	1.00	1.00
Incremental Delay, d2	21.7	0.1
Delay (s)	73.7	47.8
Level of Service	E	D
Approach Delay (s)	66.5	
Approach LOS	E	
Intersection Summary		

HCM Unsignalized Intersection Capacity Analysis
2: FM Right-In Driveway & Highway 99W

11/21/2014



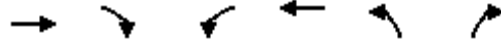
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑↑		
Volume (veh/h)	1419	230	0	1965	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1463	237	0	2026	0	0
Pedestrians					1	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	478			898		
pX, platoon unblocked				0.73	0.86	0.73
vC, conflicting volume				1464	2139	732
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				887	122	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	100
cM capacity (veh/h)				561	747	788

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3
Volume Total	731	731	237	675	675	675
Volume Left	0	0	0	0	0	0
Volume Right	0	0	237	0	0	0
cSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.43	0.43	0.14	0.40	0.40	0.40
Queue Length 95th (ft)	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						

Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	43.0%			ICU Level of Service	A	
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
 3: FM RIRO Driveway & Highway 99W

11/21/2014



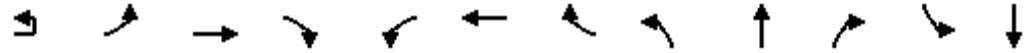
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑		↑
Volume (veh/h)	1269	138	0	1953	0	28
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1308	142	0	2013	0	29
Pedestrians						1
Lane Width (ft)						12.0
Walking Speed (ft/s)						4.0
Percent Blockage						0
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	885			491		
pX, platoon unblocked				0.76	0.76	0.76
vC, conflicting volume				1309	2316	655
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				764	649	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	96
cM capacity (veh/h)				648	308	824

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	654	654	142	1007	1007	29
Volume Left	0	0	0	0	0	0
Volume Right	0	0	142	0	0	29
cSH	1700	1700	1700	1700	1700	824
Volume to Capacity	0.38	0.38	0.08	0.59	0.59	0.04
Queue Length 95th (ft)	0	0	0	0	0	3
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.5
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.5
Approach LOS						A

Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	57.7%			ICU Level of Service	B	
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis
4: Brutscher Street & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔	↕	↗	↔	↕	↗	↖	↕		↖	↗
Volume (vph)	6	20	1202	75	254	1649	42	215	13	116	14	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5	5.0	5.5	5.5	5.0	5.0		5.0	5.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes		1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.99		1.00	0.99
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Frt		1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.87		1.00	0.88
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)		1805	3406	1462	1752	3471	1582	1752	1542		1805	1588
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00	0.71	1.00		0.58	1.00
Satd. Flow (perm)		1805	3406	1462	1752	3471	1582	1309	1542		1105	1588
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	6	21	1252	78	265	1718	44	224	14	121	15	17
RTOR Reduction (vph)	0	0	0	31	0	0	15	0	99	0	0	46
Lane Group Flow (vph)	0	27	1252	47	265	1718	29	224	36	0	15	27
Confl. Peds. (#/hr)				1						1		
Confl. Bikes (#/hr)							1			1		
Heavy Vehicles (%)	0%	0%	6%	8%	3%	4%	0%	3%	8%	5%	0%	7%
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA
Protected Phases	5	5	2		1	6			8			4
Permitted Phases				2			6	8				4
Actuated Green, G (s)		4.6	64.5	64.5	18.9	78.8	78.8	22.6	22.6		22.6	22.6
Effective Green, g (s)		4.1	64.0	64.0	18.4	78.3	78.3	22.1	22.1		22.1	22.1
Actuated g/C Ratio		0.03	0.53	0.53	0.15	0.65	0.65	0.18	0.18		0.18	0.18
Clearance Time (s)		4.5	5.0	5.0	4.5	5.0	5.0	4.5	4.5		4.5	4.5
Vehicle Extension (s)		2.3	4.8	4.8	2.3	4.8	4.8	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)		61	1816	779	268	2264	1032	241	283		203	292
v/s Ratio Prot		0.01	0.37		c0.15	c0.49			0.02			0.02
v/s Ratio Perm				0.03			0.02	c0.17			0.01	
v/c Ratio		0.44	0.69	0.06	0.99	0.76	0.03	0.93	0.13		0.07	0.09
Uniform Delay, d1		56.8	20.7	13.5	50.7	14.4	7.4	48.2	40.9		40.5	40.6
Progression Factor		0.81	1.04	1.90	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2		2.2	1.6	0.1	51.3	2.4	0.0	38.7	0.1		0.1	0.1
Delay (s)		47.9	23.1	25.7	102.0	16.8	7.4	86.9	41.1		40.6	40.7
Level of Service		D	C	C	F	B	A	F	D		D	D
Approach Delay (s)			23.8			27.7			69.6			40.7
Approach LOS			C			C			E			D

Intersection Summary		
HCM 2000 Control Delay	30.6	HCM 2000 Level of Service C
HCM 2000 Volume to Capacity ratio	0.85	
Actuated Cycle Length (s)	120.0	Sum of lost time (s) 15.5
Intersection Capacity Utilization	80.6%	ICU Level of Service D
Analysis Period (min)	15	

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014

Movement	SBR
Lane Configurations	
Volume (vph)	54
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frbp, ped/bikes	
Flpb, ped/bikes	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.96
Adj. Flow (vph)	56
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Confl. Peds. (#/hr)	2
Confl. Bikes (#/hr)	
Heavy Vehicles (%)	4%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM Unsignalized Intersection Capacity Analysis
 5: Springbrook Road & FM Main Drive Aisle

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (veh/h)	0	363	347	117	0	373
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	386	369	124	0	397
Pedestrians			1			1
Lane Width (ft)			12.0			12.0
Walking Speed (ft/s)			4.0			4.0
Percent Blockage			0			0
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (ft)			717			401
pX, platoon unblocked	0.96	0.91			0.91	
vC, conflicting volume	829	432			369	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	602	333			264	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	41			100	
cM capacity (veh/h)	445	650			1200	


















Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	386	494	397
Volume Left	0	0	0
Volume Right	386	124	0
cSH	650	1700	1700
Volume to Capacity	0.59	0.29	0.23
Queue Length 95th (ft)	98	0	0
Control Delay (s)	18.3	0.0	0.0
Lane LOS	C		
Approach Delay (s)	18.3	0.0	0.0
Approach LOS	C		

Intersection Summary			
Average Delay		5.5	
Intersection Capacity Utilization		55.5%	ICU Level of Service B
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

6: FM Fuel/US Bank & FM Main Drive Aisle

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	20	62	47	175	159	65	149	5	64	15	5	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	67	51	190	173	71	162	5	70	16	5	27
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	SB 1							
Volume Total (vph)	22	118	434	237	49							
Volume Left (vph)	22	0	190	162	16							
Volume Right (vph)	0	51	71	70	27							
Hadj (s)	0.50	-0.28	0.02	-0.01	-0.19							
Departure Headway (s)	6.4	5.6	5.1	5.5	5.7							
Degree Utilization, x	0.04	0.18	0.61	0.36	0.08							
Capacity (veh/h)	522	598	685	602	547							
Control Delay (s)	8.4	8.6	15.7	11.5	9.1							
Approach Delay (s)	8.6		15.7	11.5	9.1							
Approach LOS	A		C	B	A							
Intersection Summary												
Delay			13.0									
Level of Service			B									
Intersection Capacity Utilization			55.4%	ICU Level of Service	B							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

7: FM Main Drive Aisle & FM Right-In Driveway

11/21/2014

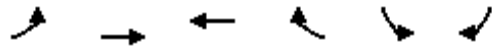


Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Sign Control		Stop	Stop		Stop	
Volume (vph)	0	149	262	0	105	125
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	166	291	0	117	139
Direction, Lane #	EB 1	WB 1	SB 1	SB 2		
Volume Total (vph)	166	291	117	139		
Volume Left (vph)	0	0	117	0		
Volume Right (vph)	0	0	0	139		
Hadj (s)	0.03	0.02	0.50	-0.63		
Departure Headway (s)	5.0	4.8	6.1	5.0		
Degree Utilization, x	0.23	0.39	0.20	0.19		
Capacity (veh/h)	679	715	555	674		
Control Delay (s)	9.5	10.8	9.4	8.0		
Approach Delay (s)	9.5	10.8	8.6			
Approach LOS	A	B	A			
Intersection Summary						
Delay			9.7			
Level of Service			A			
Intersection Capacity Utilization			31.8%		ICU Level of Service	A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

8: FM Main Drive Aisle & FM RIRO Driveway

11/21/2014




















Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↶	↷		↶	
Sign Control		Stop	Stop		Stop	
Volume (vph)	23	127	155	5	42	96
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	26	144	176	6	48	109
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	170	182	157			
Volume Left (vph)	26	0	48			
Volume Right (vph)	0	6	109			
Hadj (s)	0.05	0.01	-0.34			
Departure Headway (s)	4.5	4.5	4.4			
Degree Utilization, x	0.21	0.23	0.19			
Capacity (veh/h)	764	765	768			
Control Delay (s)	8.7	8.8	8.4			
Approach Delay (s)	8.7	8.8	8.4			
Approach LOS	A	A	A			
Intersection Summary						
Delay			8.6			
Level of Service			A			
Intersection Capacity Utilization			40.1%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Brutscher Street & FM Main Drive Aisle/Columbia Bank

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	187	4	56	8	14	27	16	121	8	38	143	159
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	203	4	61	9	15	29	17	132	9	41	155	173
Pedestrians		1			1							
Lane Width (ft)		12.0			12.0							
Walking Speed (ft/s)		4.0			4.0							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											342	
pX, platoon unblocked												
vC, conflicting volume	529	501	243	559	584	137	329			141		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	529	501	243	559	584	137	329			141		
tC, single (s)	7.1	6.5	6.2	7.3	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.7	4.0	3.3	2.2			2.2		
p0 queue free %	51	99	92	98	96	97	99			97		
cM capacity (veh/h)	418	454	795	360	408	916	1241			1434		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1							
Volume Total	268	53	17	140	370							
Volume Left	203	9	17	0	41							
Volume Right	61	29	0	9	173							
cSH	469	569	1241	1700	1434							
Volume to Capacity	0.57	0.09	0.01	0.08	0.03							
Queue Length 95th (ft)	88	8	1	0	2							
Control Delay (s)	22.4	12.0	7.9	0.0	1.1							
Lane LOS	C	B	A		A							
Approach Delay (s)	22.4	12.0	0.9		1.1							
Approach LOS	C	B										
Intersection Summary												
Average Delay			8.5									
Intersection Capacity Utilization			58.4%		ICU Level of Service					B		
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

12: Springbrook Road & Hayes Street

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	93	84	368	39	70	283
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frpb, ped/bikes	1.00	0.97	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1679	1557	1770		1703	1759
Flt Permitted	0.95	1.00	1.00		0.41	1.00
Satd. Flow (perm)	1679	1557	1770		739	1759
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	97	88	383	41	73	295
RTOR Reduction (vph)	0	77	5	0	0	0
Lane Group Flow (vph)	97	11	419	0	73	295
Confl. Peds. (#/hr)	3	4				
Confl. Bikes (#/hr)				1		
Heavy Vehicles (%)	7%	1%	6%	3%	6%	8%
Turn Type	Perm	Perm	NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8	8			6	
Actuated Green, G (s)	7.1	7.1	32.1		39.9	39.9
Effective Green, g (s)	6.6	6.6	31.6		39.4	39.4
Actuated g/C Ratio	0.12	0.12	0.57		0.72	0.72
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)	201	186	1016		587	1260
v/s Ratio Prot			c0.24		0.01	c0.17
v/s Ratio Perm	c0.06	0.01			0.08	
v/c Ratio	0.48	0.06	0.41		0.12	0.23
Uniform Delay, d1	22.6	21.4	6.5		2.8	2.7
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	1.3	0.1	1.2		0.1	0.4
Delay (s)	23.9	21.5	7.8		2.9	3.1
Level of Service	C	C	A		A	A
Approach Delay (s)	22.8		7.8			3.1
Approach LOS	C		A			A

Intersection Summary

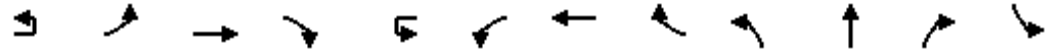
HCM 2000 Control Delay	8.8	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.42		
Actuated Cycle Length (s)	55.0	Sum of lost time (s)	13.0
Intersection Capacity Utilization	43.2%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↔	↕	↗		↔	↕	↗	↖	↕	↗	↖
Volume (vph)	2	106	1265	60	20	154	1318	117	280	134	123	285
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5		5.0	5.5	5.5	5.0	5.0	5.0	5.0
Lane Util. Factor		1.00	0.95	1.00		1.00	0.95	1.00	0.97	1.00	1.00	0.97
Frbp, ped/bikes		1.00	1.00	0.98		1.00	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		1787	3406	1438		1719	3505	1520	3433	1863	1455	3433
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		1787	3406	1438		1719	3505	1520	3433	1863	1455	3433
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	2	108	1291	61	20	157	1345	119	286	137	126	291
RTOR Reduction (vph)	0	0	0	35	0	0	0	62	0	0	112	0
Lane Group Flow (vph)	0	110	1291	26	0	177	1345	57	286	137	14	291
Confl. Peds. (#/hr)								1				
Confl. Bikes (#/hr)				1								
Heavy Vehicles (%)	1%	1%	6%	10%	5%	5%	3%	4%	2%	2%	11%	2%
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	5	5	2		1	1	6		3	8		7
Permitted Phases				2				6			8	
Actuated Green, G (s)		10.4	43.6	43.6		13.5	46.7	46.7	13.3	11.6	11.6	12.8
Effective Green, g (s)		9.9	43.1	43.1		13.0	46.2	46.2	12.8	11.1	11.1	12.3
Actuated g/C Ratio		0.10	0.43	0.43		0.13	0.46	0.46	0.13	0.11	0.11	0.12
Clearance Time (s)		4.5	5.0	5.0		4.5	5.0	5.0	4.5	4.5	4.5	4.5
Vehicle Extension (s)		2.3	4.2	4.2		2.5	4.2	4.2	2.3	2.3	2.3	2.3
Lane Grp Cap (vph)		176	1467	619		223	1619	702	439	206	161	422
v/s Ratio Prot		0.06	c0.38			0.10	c0.38		0.08	c0.07		c0.08
v/s Ratio Perm				0.02				0.04			0.01	
v/c Ratio		0.62	0.88	0.04		0.79	0.83	0.08	0.65	0.67	0.09	0.69
Uniform Delay, d1		43.3	26.1	16.5		42.2	23.5	15.0	41.5	42.7	39.9	42.0
Progression Factor		1.00	1.00	1.00		0.75	0.59	0.27	1.00	1.00	1.00	1.00
Incremental Delay, d2		5.5	7.9	0.1		12.5	3.7	0.2	2.9	6.7	0.1	4.1
Delay (s)		48.7	33.9	16.6		44.1	17.6	4.2	44.4	49.4	40.0	46.1
Level of Service		D	C	B		D	B	A	D	D	D	D
Approach Delay (s)			34.3				19.5			44.6		
Approach LOS			C				B			D		
Intersection Summary												
HCM 2000 Control Delay			31.1				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.83									
Actuated Cycle Length (s)			100.0				Sum of lost time (s)		20.5			
Intersection Capacity Utilization			80.8%				ICU Level of Service		D			
Analysis Period (min)			15									
c	Critical Lane Group											

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	SBT	SBR
Lane Configurations	↑	↑
Volume (vph)	113	89
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	5.0
Lane Util. Factor	1.00	1.00
Frbp, ped/bikes	1.00	0.98
Flpb, ped/bikes	1.00	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	1727	1518
Flt Permitted	1.00	1.00
Satd. Flow (perm)	1727	1518
Peak-hour factor, PHF	0.98	0.98
Adj. Flow (vph)	115	91
RTOR Reduction (vph)	0	81
Lane Group Flow (vph)	115	10
Confl. Peds. (#/hr)		9
Confl. Bikes (#/hr)		1
Heavy Vehicles (%)	10%	4%
Turn Type	NA	Perm
Protected Phases	4	
Permitted Phases		4
Actuated Green, G (s)	11.1	11.1
Effective Green, g (s)	10.6	10.6
Actuated g/C Ratio	0.11	0.11
Clearance Time (s)	4.5	4.5
Vehicle Extension (s)	2.3	2.3
Lane Grp Cap (vph)	183	160
v/s Ratio Prot	0.07	
v/s Ratio Perm		0.01
v/c Ratio	0.63	0.06
Uniform Delay, d1	42.8	40.2
Progression Factor	1.00	1.00
Incremental Delay, d2	5.4	0.1
Delay (s)	48.2	40.3
Level of Service	D	D
Approach Delay (s)	45.5	
Approach LOS	D	
Intersection Summary		

HCM Unsignalized Intersection Capacity Analysis

2: FM Right-In Driveway & Highway 99W

11/21/2014



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑↑		
Volume (veh/h)	1453	251	0	1627	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1498	259	0	1677	0	0
Pedestrians					1	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	478			898		
pX, platoon unblocked				0.67	0.77	0.67
vC, conflicting volume				1499	2058	750
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				747	209	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	100
cM capacity (veh/h)				580	586	722
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3
Volume Total	749	749	259	559	559	559
Volume Left	0	0	0	0	0	0
Volume Right	0	0	259	0	0	0
cSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.44	0.44	0.15	0.33	0.33	0.33
Queue Length 95th (ft)	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	43.9%			ICU Level of Service	A	
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
3: FM RIRO Driveway & Highway 99W

11/21/2014



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Volume (veh/h)	1287	156	0	1646	0	22
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1327	161	0	1697	0	23
Pedestrians					1	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	885			491		
pX, platoon unblocked				0.71	0.84	0.71
vC, conflicting volume				1328	2176	664
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				628	437	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	97
cM capacity (veh/h)				679	464	769

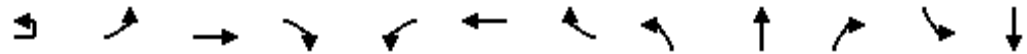
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	663	663	161	848	848	23
Volume Left	0	0	0	0	0	0
Volume Right	0	0	161	0	0	23
cSH	1700	1700	1700	1700	1700	769
Volume to Capacity	0.39	0.39	0.09	0.50	0.50	0.03
Queue Length 95th (ft)	0	0	0	0	0	2
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.8
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.8
Approach LOS						A

Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	49.2%			ICU Level of Service	A	
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔	↕	↗	↔	↕	↗	↖	↕	↖	↖	↖
Volume (vph)	7	24	1213	47	154	1395	50	198	8	137	13	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5	5.0	5.5	5.5	5.0	5.0		5.0	5.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes		1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.99		1.00	0.99
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Frt		1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.86		1.00	0.87
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)		1805	3406	1462	1752	3471	1582	1752	1529		1805	1569
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00	0.72	1.00		0.57	1.00
Satd. Flow (perm)		1805	3406	1462	1752	3471	1582	1325	1529		1086	1569
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	7	25	1264	49	160	1453	52	206	8	143	14	9
RTOR Reduction (vph)	0	0	0	22	0	0	21	0	117	0	0	41
Lane Group Flow (vph)	0	32	1264	27	160	1453	31	206	34	0	14	18
Confl. Peds. (#/hr)				1						1		
Confl. Bikes (#/hr)							1			1		
Heavy Vehicles (%)	0%	0%	6%	8%	3%	4%	0%	3%	8%	5%	0%	7%
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA
Protected Phases	5	5	2		1	6			8			4
Permitted Phases				2			6	8			4	
Actuated Green, G (s)		7.9	55.0	55.0	12.2	59.3	59.3	18.8	18.8		18.8	18.8
Effective Green, g (s)		7.4	54.5	54.5	11.7	58.8	58.8	18.3	18.3		18.3	18.3
Actuated g/C Ratio		0.07	0.54	0.54	0.12	0.59	0.59	0.18	0.18		0.18	0.18
Clearance Time (s)		4.5	5.0	5.0	4.5	5.0	5.0	4.5	4.5		4.5	4.5
Vehicle Extension (s)		2.3	4.8	4.8	2.3	4.8	4.8	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)		133	1856	796	204	2040	930	242	279		198	287
v/s Ratio Prot		0.02	c0.37		0.09	c0.42			0.02			0.01
v/s Ratio Perm				0.02			0.02	c0.16			0.01	
v/c Ratio		0.24	0.68	0.03	0.78	0.71	0.03	0.85	0.12		0.07	0.06
Uniform Delay, d1		43.7	16.5	10.5	42.9	14.6	8.7	39.5	34.1		33.8	33.8
Progression Factor		0.75	0.65	1.26	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2		0.3	1.3	0.0	16.9	2.1	0.1	23.6	0.1		0.1	0.1
Delay (s)		33.0	12.0	13.4	59.8	16.8	8.7	63.1	34.3		33.9	33.8
Level of Service		C	B	B	E	B	A	E	C		C	C
Approach Delay (s)			12.5			20.6			50.9			33.9
Approach LOS			B			C			D			C

Intersection Summary

HCM 2000 Control Delay	20.9	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.76		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	15.5
Intersection Capacity Utilization	72.9%	ICU Level of Service	C
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014

Movement	SBR
Lane Configurations	
Volume (vph)	48
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frbp, ped/bikes	
Flpb, ped/bikes	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.96
Adj. Flow (vph)	50
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Confl. Peds. (#/hr)	2
Confl. Bikes (#/hr)	
Heavy Vehicles (%)	4%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM Unsignalized Intersection Capacity Analysis

5: Springbrook Road & FM Main Drive Aisle

11/21/2014




















Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↗	↘			↕
Volume (veh/h)	0	312	226	110	0	322
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	332	240	117	0	343
Pedestrians			1			1
Lane Width (ft)			12.0			12.0
Walking Speed (ft/s)			4.0			4.0
Percent Blockage			0			0
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (ft)			717			401
pX, platoon unblocked	0.95					
vC, conflicting volume	642	300			240	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	594	300			240	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	55			100	
cM capacity (veh/h)	445	741			1338	

Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	332	357	343
Volume Left	0	0	0
Volume Right	332	117	0
cSH	741	1700	1700
Volume to Capacity	0.45	0.21	0.20
Queue Length 95th (ft)	58	0	0
Control Delay (s)	13.7	0.0	0.0
Lane LOS	B		
Approach Delay (s)	13.7	0.0	0.0
Approach LOS	B		

Intersection Summary			
Average Delay		4.4	
Intersection Capacity Utilization		45.5%	ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis
 6: FM Fuel/US Bank & FM Main Drive Aisle

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop				Stop			Stop				Stop
Volume (vph)	3	75	40	127	194	39	99	0	51	0	0	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	3	82	43	138	211	42	108	0	55	0	0	14
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	SB 1							
Volume Total (vph)	3	125	391	163	14							
Volume Left (vph)	3	0	138	108	0							
Volume Right (vph)	0	43	42	55	14							
Hadj (s)	0.50	-0.22	0.04	-0.04	-0.53							
Departure Headway (s)	5.8	5.1	4.7	5.1	4.9							
Degree Utilization, x	0.01	0.18	0.51	0.23	0.02							
Capacity (veh/h)	588	665	743	645	638							
Control Delay (s)	7.7	8.0	12.4	9.7	8.0							
Approach Delay (s)	8.0		12.4	9.7	8.0							
Approach LOS	A		B	A	A							
Intersection Summary												
Delay			10.9									
Level of Service			B									
Intersection Capacity Utilization			49.2%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 7: FM Main Drive Aisle & FM Right-In Driveway

11/21/2014



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Sign Control		Stop	Stop		Stop	
Volume (vph)	0	122	240	0	139	112
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	136	267	0	154	124
Direction, Lane #	EB 1	WB 1	SB 1	SB 2		
Volume Total (vph)	136	267	154	124		
Volume Left (vph)	0	0	154	0		
Volume Right (vph)	0	0	0	124		
Hadj (s)	0.03	0.02	0.50	-0.63		
Departure Headway (s)	5.0	4.8	6.0	4.9		
Degree Utilization, x	0.19	0.36	0.26	0.17		
Capacity (veh/h)	673	711	571	696		
Control Delay (s)	9.2	10.5	9.9	7.6		
Approach Delay (s)	9.2	10.5	8.9			
Approach LOS	A	B	A			
Intersection Summary						
Delay			9.6			
Level of Service			A			
Intersection Capacity Utilization			30.2%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 8: FM Main Drive Aisle & FM RIRO Driveway

11/21/2014




















Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↖	↗		↘	
Sign Control		Stop	Stop		Stop	
Volume (vph)	17	119	109	5	58	98
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	19	135	124	6	66	111
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	155	130	177			
Volume Left (vph)	19	0	66			
Volume Right (vph)	0	6	111			
Hadj (s)	0.04	0.01	-0.29			
Departure Headway (s)	4.5	4.5	4.3			
Degree Utilization, x	0.19	0.16	0.21			
Capacity (veh/h)	767	758	795			
Control Delay (s)	8.6	8.4	8.4			
Approach Delay (s)	8.6	8.4	8.4			
Approach LOS	A	A	A			
Intersection Summary						
Delay			8.4			
Level of Service			A			
Intersection Capacity Utilization			33.9%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Brutscher Street & FM Main Drive Aisle/Columbia Bank

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	225	2	48	4	1	6	13	106	2	5	72	132
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	245	2	52	4	1	7	14	115	2	5	78	143
Pedestrians		1			1							
Lane Width (ft)		12.0			12.0							
Walking Speed (ft/s)		4.0			4.0							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											342	
pX, platoon unblocked												
vC, conflicting volume	312	309	151	360	379	117	223			118		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	312	309	151	360	379	117	223			118		
tC, single (s)	7.1	6.5	6.2	7.3	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.7	4.0	3.3	2.2			2.2		
p0 queue free %	61	100	94	99	100	99	99			100		
cM capacity (veh/h)	627	599	895	515	547	939	1357			1462		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1							
Volume Total	299	12	14	117	227							
Volume Left	245	4	14	0	5							
Volume Right	52	7	0	2	143							
cSH	661	688	1357	1700	1462							
Volume to Capacity	0.45	0.02	0.01	0.07	0.00							
Queue Length 95th (ft)	59	1	1	0	0							
Control Delay (s)	14.9	10.3	7.7	0.0	0.2							
Lane LOS	B	B	A		A							
Approach Delay (s)	14.9	10.3	0.8		0.2							
Approach LOS	B	B										
Intersection Summary												
Average Delay			7.1									
Intersection Capacity Utilization			46.0%		ICU Level of Service					A		
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

12: Springbrook Road & Hayes Street

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	54	69	224	23	53	242
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frpb, ped/bikes	1.00	0.97	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1679	1557	1771		1703	1759
Flt Permitted	0.95	1.00	1.00		0.53	1.00
Satd. Flow (perm)	1679	1557	1771		947	1759
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	56	72	233	24	55	252
RTOR Reduction (vph)	0	65	4	0	0	0
Lane Group Flow (vph)	56	7	253	0	55	252
Confl. Peds. (#/hr)	3	4				
Confl. Bikes (#/hr)				1		
Heavy Vehicles (%)	7%	1%	6%	3%	6%	8%
Turn Type	Perm	Perm	NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8	8			6	
Actuated Green, G (s)	5.9	5.9	33.6		41.1	41.1
Effective Green, g (s)	5.4	5.4	33.1		40.6	40.6
Actuated g/C Ratio	0.10	0.10	0.60		0.74	0.74
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)	164	152	1065		740	1298
v/s Ratio Prot			c0.14		0.00	c0.14
v/s Ratio Perm	c0.03	0.00			0.05	
v/c Ratio	0.34	0.05	0.24		0.07	0.19
Uniform Delay, d1	23.1	22.5	5.1		2.1	2.2
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	0.9	0.1	0.5		0.0	0.3
Delay (s)	24.0	22.6	5.6		2.2	2.5
Level of Service	C	C	A		A	A
Approach Delay (s)	23.2		5.6			2.5
Approach LOS	C		A			A

Intersection Summary

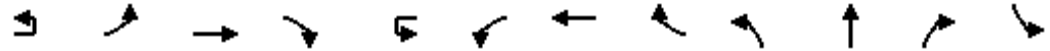
HCM 2000 Control Delay	7.5	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.25		
Actuated Cycle Length (s)	55.0	Sum of lost time (s)	13.0
Intersection Capacity Utilization	32.6%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↔	↕	↗		↔	↕	↗	↖	↕	↗	↖
Volume (vph)	2	108	1290	64	20	157	1344	119	291	140	125	291
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5		5.0	5.5	5.5	5.0	5.0	5.0	5.0
Lane Util. Factor		1.00	0.95	1.00		1.00	0.95	1.00	0.97	1.00	1.00	0.97
Frbp, ped/bikes		1.00	1.00	0.98		1.00	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		1787	3406	1438		1719	3505	1520	3433	1863	1455	3433
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		1787	3406	1438		1719	3505	1520	3433	1863	1455	3433
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	2	110	1316	65	20	160	1371	121	297	143	128	297
RTOR Reduction (vph)	0	0	0	37	0	0	0	62	0	0	113	0
Lane Group Flow (vph)	0	112	1316	28	0	180	1371	59	297	143	15	297
Confl. Peds. (#/hr)								1				
Confl. Bikes (#/hr)				1								
Heavy Vehicles (%)	1%	1%	6%	10%	5%	5%	3%	4%	2%	2%	11%	2%
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	5	5	2		1	1	6		3	8		7
Permitted Phases				2				6			8	
Actuated Green, G (s)		10.5	43.2	43.2		13.5	46.2	46.2	13.4	11.9	11.9	12.9
Effective Green, g (s)		10.0	42.7	42.7		13.0	45.7	45.7	12.9	11.4	11.4	12.4
Actuated g/C Ratio		0.10	0.43	0.43		0.13	0.46	0.46	0.13	0.11	0.11	0.12
Clearance Time (s)		4.5	5.0	5.0		4.5	5.0	5.0	4.5	4.5	4.5	4.5
Vehicle Extension (s)		2.3	4.2	4.2		2.5	4.2	4.2	2.3	2.3	2.3	2.3
Lane Grp Cap (vph)		178	1454	614		223	1601	694	442	212	165	425
v/s Ratio Prot		0.06	c0.39			0.10	c0.39		c0.09	c0.08		0.09
v/s Ratio Perm				0.02				0.04			0.01	
v/c Ratio		0.63	0.91	0.05		0.81	0.86	0.08	0.67	0.67	0.09	0.70
Uniform Delay, d1		43.2	26.8	16.7		42.3	24.2	15.3	41.5	42.5	39.6	42.0
Progression Factor		1.00	1.00	1.00		0.75	0.59	0.26	1.00	1.00	1.00	1.00
Incremental Delay, d2		5.5	9.6	0.1		13.4	4.4	0.2	3.4	7.1	0.1	4.4
Delay (s)		48.7	36.4	16.9		44.9	18.6	4.2	45.0	49.6	39.8	46.4
Level of Service		D	D	B		D	B	A	D	D	D	D
Approach Delay (s)			36.5				20.4			45.0		
Approach LOS			D				C			D		
Intersection Summary												
HCM 2000 Control Delay			32.4				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.85									
Actuated Cycle Length (s)			100.0				Sum of lost time (s)			20.5		
Intersection Capacity Utilization			82.2%				ICU Level of Service			E		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	SBT	SBR
Lane Configurations	↑	↑
Volume (vph)	120	91
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	5.0
Lane Util. Factor	1.00	1.00
Frbp, ped/bikes	1.00	0.98
Flpb, ped/bikes	1.00	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	1727	1518
Flt Permitted	1.00	1.00
Satd. Flow (perm)	1727	1518
Peak-hour factor, PHF	0.98	0.98
Adj. Flow (vph)	122	93
RTOR Reduction (vph)	0	83
Lane Group Flow (vph)	122	10
Confl. Peds. (#/hr)		9
Confl. Bikes (#/hr)		1
Heavy Vehicles (%)	10%	4%
Turn Type	NA	Perm
Protected Phases	4	
Permitted Phases		4
Actuated Green, G (s)	11.4	11.4
Effective Green, g (s)	10.9	10.9
Actuated g/C Ratio	0.11	0.11
Clearance Time (s)	4.5	4.5
Vehicle Extension (s)	2.3	2.3
Lane Grp Cap (vph)	188	165
v/s Ratio Prot	0.07	
v/s Ratio Perm		0.01
v/c Ratio	0.65	0.06
Uniform Delay, d1	42.7	40.0
Progression Factor	1.00	1.00
Incremental Delay, d2	6.3	0.1
Delay (s)	49.0	40.1
Level of Service	D	D
Approach Delay (s)	45.8	
Approach LOS	D	
Intersection Summary		

HCM Unsignalized Intersection Capacity Analysis

2: FM Right-In Driveway & Highway 99W

11/21/2014

	→	↘	↙	←	↖	↗
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑↑		
Volume (veh/h)	1482	251	0	1660	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1528	259	0	1711	0	0
Pedestrians					1	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	478			898		
pX, platoon unblocked				0.65	0.76	0.65
vC, conflicting volume	1529			2099	765	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	744			155	0	
tC, single (s)	4.1			6.8	6.9	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	100			100	100	
cM capacity (veh/h)	569			625	707	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3
Volume Total	764	764	259	570	570	570
Volume Left	0	0	0	0	0	0
Volume Right	0	0	259	0	0	0
cSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.45	0.45	0.15	0.34	0.34	0.34
Queue Length 95th (ft)	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	44.7%			ICU Level of Service	A	
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis 3: FM RIRO Driveway & Highway 99W

11/21/2014

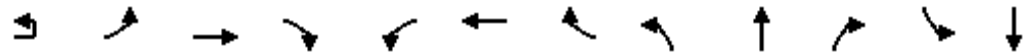


Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑		↑
Volume (veh/h)	1313	156	0	1679	0	22
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1354	161	0	1731	0	23
Pedestrians						1
Lane Width (ft)						12.0
Walking Speed (ft/s)						4.0
Percent Blockage						0
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	885			491		
pX, platoon unblocked				0.69	0.84	0.69
vC, conflicting volume				1355	2220	678
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				617	402	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	97
cM capacity (veh/h)				671	486	753
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	677	677	161	865	865	23
Volume Left	0	0	0	0	0	0
Volume Right	0	0	161	0	0	23
cSH	1700	1700	1700	1700	1700	753
Volume to Capacity	0.40	0.40	0.09	0.51	0.51	0.03
Queue Length 95th (ft)	0	0	0	0	0	2
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.9
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.9
Approach LOS						A
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	50.2%			ICU Level of Service	A	
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔	↕	↗	↔	↕	↗	↖	↕	↖	↖	↕
Volume (vph)	7	24	1237	48	162	1423	51	202	8	146	13	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5	5.0	5.5	5.5	5.0	5.0		5.0	5.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes		1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.99		1.00	0.99
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Frt		1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.86		1.00	0.87
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)		1805	3406	1462	1752	3471	1582	1752	1529		1805	1568
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00	0.72	1.00		0.55	1.00
Satd. Flow (perm)		1805	3406	1462	1752	3471	1582	1324	1529		1038	1568
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	7	25	1289	50	169	1482	53	210	8	152	14	9
RTOR Reduction (vph)	0	0	0	23	0	0	22	0	125	0	0	42
Lane Group Flow (vph)	0	32	1289	27	169	1482	31	210	35	0	14	18
Confl. Peds. (#/hr)				1						1		
Confl. Bikes (#/hr)							1			1		
Heavy Vehicles (%)	0%	0%	6%	8%	3%	4%	0%	3%	8%	5%	0%	7%
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA
Protected Phases	5	5	2		1	6			8			4
Permitted Phases				2			6	8			4	
Actuated Green, G (s)		8.0	55.1	55.1	12.4	59.5	59.5	18.5	18.5		18.5	18.5
Effective Green, g (s)		7.5	54.6	54.6	11.9	59.0	59.0	18.0	18.0		18.0	18.0
Actuated g/C Ratio		0.08	0.55	0.55	0.12	0.59	0.59	0.18	0.18		0.18	0.18
Clearance Time (s)		4.5	5.0	5.0	4.5	5.0	5.0	4.5	4.5		4.5	4.5
Vehicle Extension (s)		2.3	4.8	4.8	2.3	4.8	4.8	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)		135	1859	798	208	2047	933	238	275		186	282
v/s Ratio Prot		0.02	c0.38		0.10	c0.43			0.02			0.01
v/s Ratio Perm				0.02			0.02	c0.16			0.01	
v/c Ratio		0.24	0.69	0.03	0.81	0.72	0.03	0.88	0.13		0.08	0.06
Uniform Delay, d1		43.6	16.6	10.5	43.0	14.7	8.6	40.0	34.4		34.1	34.0
Progression Factor		0.74	0.62	1.27	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2		0.3	1.3	0.0	20.3	2.3	0.1	29.3	0.2		0.1	0.1
Delay (s)		32.7	11.5	13.4	63.2	16.9	8.6	69.3	34.6		34.2	34.1
Level of Service		C	B	B	E	B	A	E	C		C	C
Approach Delay (s)			12.1			21.3			54.3			34.1
Approach LOS			B			C			D			C

Intersection Summary

HCM 2000 Control Delay	21.4	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.77		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	15.5
Intersection Capacity Utilization	74.2%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014

Movement	SBR
Lane Configurations	
Volume (vph)	49
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frbp, ped/bikes	
Flpb, ped/bikes	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.96
Adj. Flow (vph)	51
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Confl. Peds. (#/hr)	2
Confl. Bikes (#/hr)	
Heavy Vehicles (%)	4%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM Unsignalized Intersection Capacity Analysis

5: Springbrook Road & FM Main Drive Aisle

11/21/2014




















Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (veh/h)	0	312	239	111	0	336
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	332	254	118	0	357
Pedestrians			1			1
Lane Width (ft)			12.0			12.0
Walking Speed (ft/s)			4.0			4.0
Percent Blockage			0			0
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (ft)			717			401
pX, platoon unblocked	0.94					
vC, conflicting volume	672	314			254	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	621	314			254	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	54			100	
cM capacity (veh/h)	428	728			1323	

Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	332	372	357
Volume Left	0	0	0
Volume Right	332	118	0
cSH	728	1700	1700
Volume to Capacity	0.46	0.22	0.21
Queue Length 95th (ft)	60	0	0
Control Delay (s)	14.0	0.0	0.0
Lane LOS	B		
Approach Delay (s)	14.0	0.0	0.0
Approach LOS	B		

Intersection Summary			
Average Delay		4.4	
Intersection Capacity Utilization		46.3%	ICU Level of Service
Analysis Period (min)		15	A

HCM Unsignalized Intersection Capacity Analysis
 6: FM Fuel/US Bank & FM Main Drive Aisle

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop				Stop			Stop				Stop
Volume (vph)	3	76	40	127	194	39	99	0	51	0	0	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	3	83	43	138	211	42	108	0	55	0	0	14
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	SB 1							
Volume Total (vph)	3	126	391	163	14							
Volume Left (vph)	3	0	138	108	0							
Volume Right (vph)	0	43	42	55	14							
Hadj (s)	0.50	-0.22	0.04	-0.04	-0.53							
Departure Headway (s)	5.8	5.1	4.7	5.1	4.9							
Degree Utilization, x	0.01	0.18	0.51	0.23	0.02							
Capacity (veh/h)	588	665	742	645	637							
Control Delay (s)	7.7	8.0	12.5	9.7	8.0							
Approach Delay (s)	8.0		12.5	9.7	8.0							
Approach LOS	A		B	A	A							
Intersection Summary												
Delay			10.9									
Level of Service			B									
Intersection Capacity Utilization			49.2%	ICU Level of Service	A							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

7: FM Main Drive Aisle & FM Right-In Driveway

11/21/2014



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Sign Control		Stop	Stop		Stop	
Volume (vph)	0	123	240	0	139	112
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	137	267	0	154	124
Direction, Lane #	EB 1	WB 1	SB 1	SB 2		
Volume Total (vph)	137	267	154	124		
Volume Left (vph)	0	0	154	0		
Volume Right (vph)	0	0	0	124		
Hadj (s)	0.03	0.02	0.50	-0.63		
Departure Headway (s)	5.0	4.8	6.0	4.9		
Degree Utilization, x	0.19	0.36	0.26	0.17		
Capacity (veh/h)	673	710	570	696		
Control Delay (s)	9.2	10.5	9.9	7.7		
Approach Delay (s)	9.2	10.5	8.9			
Approach LOS	A	B	A			
Intersection Summary						
Delay			9.6			
Level of Service			A			
Intersection Capacity Utilization			30.2%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 8: FM Main Drive Aisle & FM RIRO Driveway

11/21/2014




















Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↖	↗		↘	
Sign Control		Stop	Stop		Stop	
Volume (vph)	17	122	111	5	58	98
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	19	139	126	6	66	111
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	158	132	177			
Volume Left (vph)	19	0	66			
Volume Right (vph)	0	6	111			
Hadj (s)	0.04	0.01	-0.29			
Departure Headway (s)	4.5	4.5	4.3			
Degree Utilization, x	0.20	0.16	0.21			
Capacity (veh/h)	766	757	792			
Control Delay (s)	8.6	8.4	8.4			
Approach Delay (s)	8.6	8.4	8.4			
Approach LOS	A	A	A			
Intersection Summary						
Delay			8.5			
Level of Service			A			
Intersection Capacity Utilization			34.1%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Brutscher Street & FM Main Drive Aisle/Columbia Bank

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	225	2	51	4	1	6	15	114	2	5	78	132
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	245	2	55	4	1	7	16	124	2	5	85	143
Pedestrians		1			1							
Lane Width (ft)		12.0			12.0							
Walking Speed (ft/s)		4.0			4.0							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											342	
pX, platoon unblocked												
vC, conflicting volume	332	328	158	383	399	126	229			127		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	332	328	158	383	399	126	229			127		
tC, single (s)	7.1	6.5	6.2	7.3	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.7	4.0	3.3	2.2			2.2		
p0 queue free %	60	100	94	99	100	99	99			100		
cM capacity (veh/h)	607	584	887	494	533	929	1350			1452		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1							
Volume Total	302	12	16	126	234							
Volume Left	245	4	16	0	5							
Volume Right	55	7	0	2	143							
cSH	645	669	1350	1700	1452							
Volume to Capacity	0.47	0.02	0.01	0.07	0.00							
Queue Length 95th (ft)	63	1	1	0	0							
Control Delay (s)	15.4	10.5	7.7	0.0	0.2							
Lane LOS	C	B	A		A							
Approach Delay (s)	15.4	10.5	0.9		0.2							
Approach LOS	C	B										
Intersection Summary												
Average Delay			7.2									
Intersection Capacity Utilization			46.5%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

12: Springbrook Road & Hayes Street

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	55	70	237	23	54	255
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes	1.00	0.97	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1679	1557	1772		1703	1759
Flt Permitted	0.95	1.00	1.00		0.52	1.00
Satd. Flow (perm)	1679	1557	1772		935	1759
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	57	73	247	24	56	266
RTOR Reduction (vph)	0	66	4	0	0	0
Lane Group Flow (vph)	57	7	267	0	56	266
Confl. Peds. (#/hr)	3	4				
Confl. Bikes (#/hr)				1		
Heavy Vehicles (%)	7%	1%	6%	3%	6%	8%
Turn Type	Perm	Perm	NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8	8			6	
Actuated Green, G (s)	5.9	5.9	33.5		41.1	41.1
Effective Green, g (s)	5.4	5.4	33.0		40.6	40.6
Actuated g/C Ratio	0.10	0.10	0.60		0.74	0.74
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)	164	152	1063		733	1298
v/s Ratio Prot			c0.15		0.00	c0.15
v/s Ratio Perm	c0.03	0.00			0.05	
v/c Ratio	0.35	0.05	0.25		0.08	0.20
Uniform Delay, d1	23.2	22.5	5.2		2.1	2.2
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	0.9	0.1	0.6		0.0	0.4
Delay (s)	24.1	22.6	5.7		2.2	2.6
Level of Service	C	C	A		A	A
Approach Delay (s)	23.2		5.7			2.5
Approach LOS	C		A			A

Intersection Summary

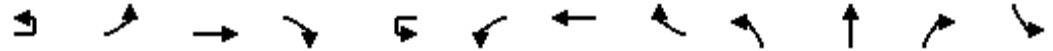
HCM 2000 Control Delay	7.4	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.27		
Actuated Cycle Length (s)	55.0	Sum of lost time (s)	13.0
Intersection Capacity Utilization	33.2%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↔	↕	↗		↔	↕	↗	↖	↑	↗	↖
Volume (vph)	2	108	1294	64	20	156	1340	119	299	143	126	295
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5		5.0	5.5	5.5	5.0	5.0	5.0	5.0
Lane Util. Factor		1.00	0.95	1.00		1.00	0.95	1.00	0.97	1.00	1.00	0.97
Frbp, ped/bikes		1.00	1.00	0.98		1.00	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		1787	3406	1438		1719	3505	1520	3433	1863	1455	3433
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		1787	3406	1438		1719	3505	1520	3433	1863	1455	3433
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	2	110	1320	65	20	159	1367	121	305	146	129	301
RTOR Reduction (vph)	0	0	0	37	0	0	0	63	0	0	114	0
Lane Group Flow (vph)	0	112	1320	28	0	179	1367	58	305	146	15	301
Confl. Peds. (#/hr)								1				
Confl. Bikes (#/hr)				1								
Heavy Vehicles (%)	1%	1%	6%	10%	5%	5%	3%	4%	2%	2%	11%	2%
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	5	5	2		1	1	6		3	8		7
Permitted Phases				2				6			8	
Actuated Green, G (s)		10.5	43.0	43.0		13.5	46.0	46.0	13.7	12.0	12.0	13.0
Effective Green, g (s)		10.0	42.5	42.5		13.0	45.5	45.5	13.2	11.5	11.5	12.5
Actuated g/C Ratio		0.10	0.42	0.42		0.13	0.46	0.46	0.13	0.12	0.12	0.12
Clearance Time (s)		4.5	5.0	5.0		4.5	5.0	5.0	4.5	4.5	4.5	4.5
Vehicle Extension (s)		2.3	4.2	4.2		2.5	4.2	4.2	2.3	2.3	2.3	2.3
Lane Grp Cap (vph)		178	1447	611		223	1594	691	453	214	167	429
v/s Ratio Prot		0.06	c0.39			0.10	c0.39		c0.09	c0.08		0.09
v/s Ratio Perm				0.02				0.04			0.01	
v/c Ratio		0.63	0.91	0.05		0.80	0.86	0.08	0.67	0.68	0.09	0.70
Uniform Delay, d1		43.2	27.0	16.9		42.3	24.4	15.4	41.3	42.5	39.6	42.0
Progression Factor		1.00	1.00	1.00		0.75	0.59	0.27	1.00	1.00	1.00	1.00
Incremental Delay, d2		5.5	10.3	0.1		13.2	4.4	0.2	3.4	7.5	0.1	4.5
Delay (s)		48.7	37.3	17.0		44.7	18.9	4.3	44.7	50.0	39.7	46.5
Level of Service		D	D	B		D	B	A	D	D	D	D
Approach Delay (s)			37.2				20.6			44.9		
Approach LOS			D				C			D		
Intersection Summary												
HCM 2000 Control Delay			32.8				HCM 2000 Level of Service			C		
HCM 2000 Volume to Capacity ratio			0.86									
Actuated Cycle Length (s)			100.0				Sum of lost time (s)			20.5		
Intersection Capacity Utilization			82.5%				ICU Level of Service			E		
Analysis Period (min)			15									
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W







11/21/2014



Movement	SBT	SBR
Lane Configurations	↑	↑
Volume (vph)	119	91
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	5.0
Lane Util. Factor	1.00	1.00
Frbp, ped/bikes	1.00	0.98
Flpb, ped/bikes	1.00	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	1727	1518
Flt Permitted	1.00	1.00
Satd. Flow (perm)	1727	1518
Peak-hour factor, PHF	0.98	0.98
Adj. Flow (vph)	121	93
RTOR Reduction (vph)	0	83
Lane Group Flow (vph)	121	10
Confl. Peds. (#/hr)		9
Confl. Bikes (#/hr)		1
Heavy Vehicles (%)	10%	4%
Turn Type	NA	Perm
Protected Phases	4	
Permitted Phases		4
Actuated Green, G (s)	11.3	11.3
Effective Green, g (s)	10.8	10.8
Actuated g/C Ratio	0.11	0.11
Clearance Time (s)	4.5	4.5
Vehicle Extension (s)	2.3	2.3
Lane Grp Cap (vph)	186	163
v/s Ratio Prot	0.07	
v/s Ratio Perm		0.01
v/c Ratio	0.65	0.06
Uniform Delay, d1	42.8	40.0
Progression Factor	1.00	1.00
Incremental Delay, d2	6.7	0.1
Delay (s)	49.5	40.1
Level of Service	D	D
Approach Delay (s)	46.0	
Approach LOS	D	
Intersection Summary		

HCM Unsignalized Intersection Capacity Analysis 2: FM Right-In Driveway & Highway 99W

11/21/2014

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑↑		
Volume (veh/h)	1478	264	0	1655	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1524	272	0	1706	0	0
Pedestrians					1	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	478			898		
pX, platoon unblocked				0.65	0.75	0.65
vC, conflicting volume				1525	2093	763
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				730	155	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	100
cM capacity (veh/h)				574	622	705
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3
Volume Total	762	762	272	569	569	569
Volume Left	0	0	0	0	0	0
Volume Right	0	0	272	0	0	0
cSH	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.45	0.45	0.16	0.33	0.33	0.33
Queue Length 95th (ft)	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS						
Approach Delay (s)	0.0			0.0		
Approach LOS						
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	44.6%			ICU Level of Service	A	
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
 3: FM RIRO Driveway & Highway 99W

11/21/2014



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗		↑↑		↗
Volume (veh/h)	1309	156	0	1674	0	28
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1349	161	0	1726	0	29
Pedestrians					1	
Lane Width (ft)					12.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	885			491		
pX, platoon unblocked				0.69	0.84	0.69
vC, conflicting volume				1350	2213	676
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				608	400	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	96
cM capacity (veh/h)				675	489	752

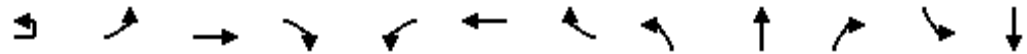
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	675	675	161	863	863	29
Volume Left	0	0	0	0	0	0
Volume Right	0	0	161	0	0	29
cSH	1700	1700	1700	1700	1700	752
Volume to Capacity	0.40	0.40	0.09	0.51	0.51	0.04
Queue Length 95th (ft)	0	0	0	0	0	3
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	10.0
Lane LOS						A
Approach Delay (s)	0.0			0.0		10.0
Approach LOS						A

Intersection Summary						
Average Delay			0.1			
Intersection Capacity Utilization			50.0%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↵	↕	↗	↵	↕	↗	↗	↕	↗	↗	↗
Volume (vph)	7	24	1239	48	169	1418	51	202	8	146	13	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5	5.0	5.5	5.5	5.0	5.0		5.0	5.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes		1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.99		1.00	0.99
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Frt		1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.86		1.00	0.87
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)		1805	3406	1462	1752	3471	1582	1752	1529		1805	1568
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00	0.72	1.00		0.55	1.00
Satd. Flow (perm)		1805	3406	1462	1752	3471	1582	1324	1529		1036	1568
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	7	25	1291	50	176	1477	53	210	8	152	14	9
RTOR Reduction (vph)	0	0	0	23	0	0	22	0	125	0	0	42
Lane Group Flow (vph)	0	32	1291	27	176	1477	31	210	35	0	14	18
Confl. Peds. (#/hr)				1						1		
Confl. Bikes (#/hr)							1			1		
Heavy Vehicles (%)	0%	0%	6%	8%	3%	4%	0%	3%	8%	5%	0%	7%
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA
Protected Phases	5	5	2		1	6			8			4
Permitted Phases				2			6	8				4
Actuated Green, G (s)		8.0	55.0	55.0	12.6	59.6	59.6	18.4	18.4		18.4	18.4
Effective Green, g (s)		7.5	54.5	54.5	12.1	59.1	59.1	17.9	17.9		17.9	17.9
Actuated g/C Ratio		0.08	0.54	0.54	0.12	0.59	0.59	0.18	0.18		0.18	0.18
Clearance Time (s)		4.5	5.0	5.0	4.5	5.0	5.0	4.5	4.5		4.5	4.5
Vehicle Extension (s)		2.3	4.8	4.8	2.3	4.8	4.8	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)		135	1856	796	211	2051	934	236	273		185	280
v/s Ratio Prot		0.02	c0.38		0.10	c0.43			0.02			0.01
v/s Ratio Perm				0.02			0.02	c0.16			0.01	
v/c Ratio		0.24	0.70	0.03	0.83	0.72	0.03	0.89	0.13		0.08	0.06
Uniform Delay, d1		43.6	16.7	10.5	43.0	14.6	8.5	40.1	34.5		34.2	34.1
Progression Factor		0.75	0.62	1.28	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2		0.3	1.3	0.0	23.1	2.2	0.1	30.6	0.2		0.1	0.1
Delay (s)		33.0	11.6	13.5	66.1	16.8	8.6	70.7	34.7		34.3	34.2
Level of Service		C	B	B	E	B	A	E	C		C	C
Approach Delay (s)			12.2			21.6			55.1			34.2
Approach LOS			B			C			E			C

Intersection Summary

HCM 2000 Control Delay	21.7	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.77		
Actuated Cycle Length (s)	100.0	Sum of lost time (s)	15.5
Intersection Capacity Utilization	74.6%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/21/2014

Movement	SBR
Lane Configurations	
Volume (vph)	49
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frbp, ped/bikes	
Flpb, ped/bikes	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.96
Adj. Flow (vph)	51
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Confl. Peds. (#/hr)	2
Confl. Bikes (#/hr)	
Heavy Vehicles (%)	4%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM Unsignalized Intersection Capacity Analysis

5: Springbrook Road & FM Main Drive Aisle

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↖	↗			↕
Volume (veh/h)	0	326	238	115	0	334
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	347	253	122	0	355
Pedestrians			1			1
Lane Width (ft)			12.0			12.0
Walking Speed (ft/s)			4.0			4.0
Percent Blockage			0			0
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (ft)			717			401
pX, platoon unblocked	0.94					
vC, conflicting volume	671	315			253	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	620	315			253	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	52			100	
cM capacity (veh/h)	428	727			1324	


















Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	347	376	355
Volume Left	0	0	0
Volume Right	347	122	0
cSH	727	1700	1700
Volume to Capacity	0.48	0.22	0.21
Queue Length 95th (ft)	65	0	0
Control Delay (s)	14.4	0.0	0.0
Lane LOS	B		
Approach Delay (s)	14.4	0.0	0.0
Approach LOS	B		

Intersection Summary			
Average Delay		4.6	
Intersection Capacity Utilization		47.3%	ICU Level of Service A
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

6: FM Fuel/US Bank & FM Main Drive Aisle

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop				Stop
Volume (vph)	3	76	44	155	194	39	113	0	69	0	0	13
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	3	83	48	168	211	42	123	0	75	0	0	14
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	SB 1							
Volume Total (vph)	3	130	422	198	14							
Volume Left (vph)	3	0	168	123	0							
Volume Right (vph)	0	48	42	75	14							
Hadj (s)	0.50	-0.24	0.05	-0.07	-0.53							
Departure Headway (s)	6.0	5.3	4.8	5.2	5.1							
Degree Utilization, x	0.01	0.19	0.57	0.29	0.02							
Capacity (veh/h)	559	641	722	626	603							
Control Delay (s)	7.9	8.3	14.0	10.3	8.2							
Approach Delay (s)	8.3		14.0	10.3	8.2							
Approach LOS	A		B	B	A							
Intersection Summary												
Delay			11.9									
Level of Service			B									
Intersection Capacity Utilization			52.6%		ICU Level of Service		A					
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis

7: FM Main Drive Aisle & FM Right-In Driveway

11/21/2014



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Sign Control		Stop	Stop		Stop	
Volume (vph)	0	141	255	0	139	125
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	157	283	0	154	139
Direction, Lane #	EB 1	WB 1	SB 1	SB 2		
Volume Total (vph)	157	283	154	139		
Volume Left (vph)	0	0	154	0		
Volume Right (vph)	0	0	0	139		
Hadj (s)	0.03	0.02	0.50	-0.63		
Departure Headway (s)	5.1	4.9	6.1	5.0		
Degree Utilization, x	0.22	0.39	0.26	0.19		
Capacity (veh/h)	664	700	560	681		
Control Delay (s)	9.5	10.9	10.1	7.9		
Approach Delay (s)	9.5	10.9	9.1			
Approach LOS	A	B	A			
Intersection Summary						
Delay			9.9			
Level of Service			A			
Intersection Capacity Utilization			31.5%	ICU Level of Service		A
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

8: FM Main Drive Aisle & FM RIRO Driveway

11/21/2014




















Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Volume (vph)	23	128	119	5	58	98
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	26	145	135	6	66	111
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	172	141	177			
Volume Left (vph)	26	0	66			
Volume Right (vph)	0	6	111			
Hadj (s)	0.04	0.01	-0.29			
Departure Headway (s)	4.5	4.5	4.3			
Degree Utilization, x	0.22	0.18	0.21			
Capacity (veh/h)	763	753	780			
Control Delay (s)	8.8	8.5	8.5			
Approach Delay (s)	8.8	8.5	8.5			
Approach LOS	A	A	A			
Intersection Summary						
Delay			8.6			
Level of Service			A			
Intersection Capacity Utilization			38.6%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Brutscher Street & FM Main Drive Aisle/Columbia Bank

11/21/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	225	2	57	4	1	6	15	114	2	5	77	140
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	245	2	62	4	1	7	16	124	2	5	84	152
Pedestrians		1			1							
Lane Width (ft)		12.0			12.0							
Walking Speed (ft/s)		4.0			4.0							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											342	
pX, platoon unblocked												
vC, conflicting volume	335	331	161	392	406	126	237			127		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	335	331	161	392	406	126	237			127		
tC, single (s)	7.1	6.5	6.2	7.3	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.7	4.0	3.3	2.2			2.2		
p0 queue free %	60	100	93	99	100	99	99			100		
cM capacity (veh/h)	604	581	883	482	528	929	1341			1452		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1							
Volume Total	309	12	16	126	241							
Volume Left	245	4	16	0	5							
Volume Right	62	7	0	2	152							
cSH	645	661	1341	1700	1452							
Volume to Capacity	0.48	0.02	0.01	0.07	0.00							
Queue Length 95th (ft)	65	1	1	0	0							
Control Delay (s)	15.6	10.5	7.7	0.0	0.2							
Lane LOS	C	B	A		A							
Approach Delay (s)	15.6	10.5	0.9		0.2							
Approach LOS	C	B										
Intersection Summary												
Average Delay			7.3									
Intersection Capacity Utilization			47.3%		ICU Level of Service				A			
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

12: Springbrook Road & Hayes Street

11/21/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	59	71	239	23	54	253
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes	1.00	0.97	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1679	1557	1772		1703	1759
Flt Permitted	0.95	1.00	1.00		0.52	1.00
Satd. Flow (perm)	1679	1557	1772		933	1759
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	61	74	249	24	56	264
RTOR Reduction (vph)	0	67	4	0	0	0
Lane Group Flow (vph)	61	7	269	0	56	264
Confl. Peds. (#/hr)	3	4				
Confl. Bikes (#/hr)				1		
Heavy Vehicles (%)	7%	1%	6%	3%	6%	8%
Turn Type	Perm	Perm	NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8	8			6	
Actuated Green, G (s)	6.0	6.0	33.4		41.0	41.0
Effective Green, g (s)	5.5	5.5	32.9		40.5	40.5
Actuated g/C Ratio	0.10	0.10	0.60		0.74	0.74
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)	167	155	1059		730	1295
v/s Ratio Prot			c0.15		0.00	c0.15
v/s Ratio Perm	c0.04	0.00			0.05	
v/c Ratio	0.37	0.05	0.25		0.08	0.20
Uniform Delay, d1	23.1	22.4	5.2		2.2	2.2
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	1.0	0.1	0.6		0.0	0.4
Delay (s)	24.1	22.5	5.8		2.2	2.6
Level of Service	C	C	A		A	A
Approach Delay (s)	23.2		5.8			2.5
Approach LOS	C		A			A

Intersection Summary

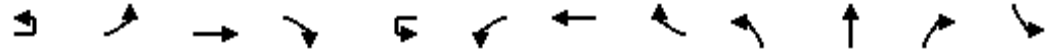
HCM 2000 Control Delay	7.6	HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.27		
Actuated Cycle Length (s)	55.0	Sum of lost time (s)	13.0
Intersection Capacity Utilization	33.3%	ICU Level of Service	A
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/24/2014



Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL
Lane Configurations		↔	↕	↗		↖	↕	↗	↖	↕	↗	↖
Volume (vph)	4	124	1071	121	11	462	1227	297	439	252	252	367
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5		5.0	5.5	5.5	5.0	5.0	5.0	5.0
Lane Util. Factor		1.00	0.95	1.00		0.97	0.95	1.00	0.97	1.00	1.00	0.97
Frbp, ped/bikes		1.00	1.00	0.99		1.00	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes		1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		1.00	1.00	0.85		1.00	1.00	0.85	1.00	1.00	0.85	1.00
Flt Protected		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)		1787	3406	1450		3335	3505	1519	3433	1863	1455	3433
Flt Permitted		0.95	1.00	1.00		0.95	1.00	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)		1787	3406	1450		3335	3505	1519	3433	1863	1455	3433
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	4	127	1093	123	11	471	1252	303	448	257	257	374
RTOR Reduction (vph)	0	0	0	66	0	0	0	155	0	0	213	0
Lane Group Flow (vph)	0	131	1093	57	0	482	1252	148	448	257	44	374
Confl. Peds. (#/hr)								1				
Confl. Bikes (#/hr)				1								
Heavy Vehicles (%)	1%	1%	6%	10%	5%	5%	3%	4%	2%	2%	11%	2%
Turn Type	Prot	Prot	NA	Perm	Prot	Prot	NA	Perm	Prot	NA	Perm	Prot
Protected Phases	5	5	2		1	1	6		3	8		7
Permitted Phases				2			6				8	
Actuated Green, G (s)		12.7	41.7	41.7		19.0	48.0	48.0	22.7	21.1	21.1	19.7
Effective Green, g (s)		12.2	41.2	41.2		18.5	47.5	47.5	22.2	20.6	20.6	19.2
Actuated g/C Ratio		0.10	0.34	0.34		0.15	0.40	0.40	0.18	0.17	0.17	0.16
Clearance Time (s)		4.5	5.0	5.0		4.5	5.0	5.0	4.5	4.5	4.5	4.5
Vehicle Extension (s)		2.3	4.2	4.2		2.5	4.2	4.2	2.3	2.3	2.3	2.3
Lane Grp Cap (vph)		181	1169	497		514	1387	601	635	319	249	549
v/s Ratio Prot		0.07	c0.32			0.14	c0.36		c0.13	c0.14		0.11
v/s Ratio Perm				0.04				0.10			0.03	
v/c Ratio		0.72	0.93	0.12		0.94	0.90	0.25	0.71	0.81	0.18	0.68
Uniform Delay, d1		52.3	38.1	26.9		50.2	34.1	24.3	45.8	47.8	42.5	47.5
Progression Factor		1.00	1.00	1.00		0.91	0.87	0.91	1.00	1.00	1.00	1.00
Incremental Delay, d2		12.1	14.7	0.5		18.5	6.8	0.6	3.2	13.2	0.2	3.0
Delay (s)		64.4	52.8	27.4		64.2	36.5	22.8	49.0	61.0	42.7	50.5
Level of Service		E	D	C		E	D	C	D	E	D	D
Approach Delay (s)			51.6				41.0			50.5		
Approach LOS			D				D			D		
Intersection Summary												
HCM 2000 Control Delay			47.2				HCM 2000 Level of Service			D		
HCM 2000 Volume to Capacity ratio			0.89									
Actuated Cycle Length (s)			120.0				Sum of lost time (s)		20.5			
Intersection Capacity Utilization			86.8%				ICU Level of Service		E			
Analysis Period (min)			15									

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

1: Springbrook Road & Highway 99W

11/24/2014



Movement	SBT	SBR
Lane Configurations	↑	↑
Volume (vph)	190	126
Ideal Flow (vphpl)	1900	1900
Total Lost time (s)	5.0	5.0
Lane Util. Factor	1.00	1.00
Frbp, ped/bikes	1.00	0.98
Flpb, ped/bikes	1.00	1.00
Frt	1.00	0.85
Flt Protected	1.00	1.00
Satd. Flow (prot)	1727	1516
Flt Permitted	1.00	1.00
Satd. Flow (perm)	1727	1516
Peak-hour factor, PHF	0.98	0.98
Adj. Flow (vph)	194	129
RTOR Reduction (vph)	0	110
Lane Group Flow (vph)	194	19
Confl. Peds. (#/hr)		9
Confl. Bikes (#/hr)		1
Heavy Vehicles (%)	10%	4%
Turn Type	NA	Perm
Protected Phases	4	
Permitted Phases		4
Actuated Green, G (s)	18.1	18.1
Effective Green, g (s)	17.6	17.6
Actuated g/C Ratio	0.15	0.15
Clearance Time (s)	4.5	4.5
Vehicle Extension (s)	2.3	2.3
Lane Grp Cap (vph)	253	222
v/s Ratio Prot	0.11	
v/s Ratio Perm		0.01
v/c Ratio	0.77	0.09
Uniform Delay, d1	49.2	44.2
Progression Factor	1.00	1.00
Incremental Delay, d2	12.2	0.1
Delay (s)	61.4	44.3
Level of Service	E	D
Approach Delay (s)	52.4	
Approach LOS	D	
Intersection Summary		

HCM Unsignalized Intersection Capacity Analysis
2: FM Right-In Driveway & Highway 99W

11/24/2014









Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑↑		
Volume (veh/h)	1484	230	0	2025	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1530	237	0	2088	0	0
Pedestrians					1	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					4.0	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	478			898		
pX, platoon unblocked				0.70	0.70	0.70
vC, conflicting volume				1531	2053	766
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				906	1650	0
tC, single (s)				4.1	6.8	6.9
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	100
cM capacity (veh/h)				533	64	761

Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	WB 4
Volume Total	765	765	237	522	522	522	522
Volume Left	0	0	0	0	0	0	0
Volume Right	0	0	237	0	0	0	0
cSH	1700	1700	1700	1700	1700	1700	1700
Volume to Capacity	0.45	0.45	0.14	0.31	0.31	0.31	0.31
Queue Length 95th (ft)	0	0	0	0	0	0	0
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Lane LOS							
Approach Delay (s)	0.0			0.0			
Approach LOS							

Intersection Summary			
Average Delay	0.0		
Intersection Capacity Utilization	44.8%	ICU Level of Service	A
Analysis Period (min)	15		

HCM Unsignalized Intersection Capacity Analysis 3: FM RIRO Driveway & Highway 99W

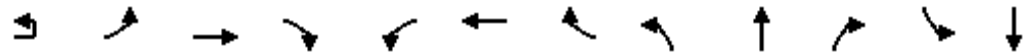
11/24/2014

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↑↑		↑
Volume (veh/h)	1334	138	0	2013	0	28
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Hourly flow rate (vph)	1375	142	0	2075	0	29
Pedestrians						1
Lane Width (ft)						12.0
Walking Speed (ft/s)						4.0
Percent Blockage						0
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	885			491		
pX, platoon unblocked				0.73	0.73	0.73
vC, conflicting volume	1376			2414	689	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	773			524	0	
tC, single (s)	4.1			6.8	6.9	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	100			100	96	
cM capacity (veh/h)	620			355	795	
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	NB 1
Volume Total	688	688	142	1038	1038	29
Volume Left	0	0	0	0	0	0
Volume Right	0	0	142	0	0	29
cSH	1700	1700	1700	1700	1700	795
Volume to Capacity	0.40	0.40	0.08	0.61	0.61	0.04
Queue Length 95th (ft)	0	0	0	0	0	3
Control Delay (s)	0.0	0.0	0.0	0.0	0.0	9.7
Lane LOS						A
Approach Delay (s)	0.0			0.0		9.7
Approach LOS						A
Intersection Summary						
Average Delay	0.1					
Intersection Capacity Utilization	59.4%			ICU Level of Service		B
Analysis Period (min)	15					

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/24/2014



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations		↔	↕	↗	↔	↕	↗	↖	↕		↖	↗
Volume (vph)	6	20	1267	75	254	1709	42	215	13	116	14	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.5	5.5	5.0	5.5	5.5	5.0	5.0		5.0	5.0
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes		1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.99		1.00	0.99
Flpb, ped/bikes		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Frt		1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.87		1.00	0.88
Flt Protected		0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)		1805	3406	1462	1752	3471	1582	1752	1542		1805	1588
Flt Permitted		0.95	1.00	1.00	0.95	1.00	1.00	0.71	1.00		0.59	1.00
Satd. Flow (perm)		1805	3406	1462	1752	3471	1582	1309	1542		1130	1588
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	6	21	1320	78	265	1780	44	224	14	121	15	17
RTOR Reduction (vph)	0	0	0	40	0	0	16	0	96	0	0	45
Lane Group Flow (vph)	0	27	1320	38	265	1780	28	224	39	0	15	28
Confl. Peds. (#/hr)				1						1		
Confl. Bikes (#/hr)							1			1		
Heavy Vehicles (%)	0%	0%	6%	8%	3%	4%	0%	3%	8%	5%	0%	7%
Turn Type	Prot	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA
Protected Phases	5	5	2		1	6			8			4
Permitted Phases				2			6	8				4
Actuated Green, G (s)		3.7	59.6	59.6	21.6	77.5	77.5	24.8	24.8		24.8	24.8
Effective Green, g (s)		3.2	59.1	59.1	21.1	77.0	77.0	24.3	24.3		24.3	24.3
Actuated g/C Ratio		0.03	0.49	0.49	0.18	0.64	0.64	0.20	0.20		0.20	0.20
Clearance Time (s)		4.5	5.0	5.0	4.5	5.0	5.0	4.5	4.5		4.5	4.5
Vehicle Extension (s)		2.3	4.8	4.8	2.3	4.8	4.8	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)		48	1677	720	308	2227	1015	265	312		228	321
v/s Ratio Prot		0.01	0.39		c0.15	c0.51			0.02			0.02
v/s Ratio Perm				0.03			0.02	c0.17			0.01	
v/c Ratio		0.56	0.79	0.05	0.86	0.80	0.03	0.85	0.12		0.07	0.09
Uniform Delay, d1		57.7	25.2	15.9	48.0	15.8	7.8	46.0	39.1		38.7	38.9
Progression Factor		0.68	1.48	4.72	1.00	1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2		6.7	2.4	0.1	20.6	3.1	0.1	20.9	0.1		0.1	0.1
Delay (s)		46.0	39.7	75.0	68.6	18.9	7.9	66.9	39.3		38.8	38.9
Level of Service		D	D	E	E	B	A	E	D		D	D
Approach Delay (s)			41.7			25.0			56.5			38.9
Approach LOS			D			C			E			D

Intersection Summary

HCM 2000 Control Delay	34.2	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.84		
Actuated Cycle Length (s)	120.0	Sum of lost time (s)	15.5
Intersection Capacity Utilization	82.3%	ICU Level of Service	E
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

4: Brutscher Street & Highway 99W

11/24/2014

Movement	SBR
Lane Configurations	
Volume (vph)	54
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frbp, ped/bikes	
Flpb, ped/bikes	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.96
Adj. Flow (vph)	56
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Confl. Peds. (#/hr)	2
Confl. Bikes (#/hr)	
Heavy Vehicles (%)	4%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM Unsignalized Intersection Capacity Analysis
5: Springbrook Road & FM Main Drive Aisle

11/24/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (veh/h)	0	363	572	117	0	778
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	386	609	124	0	828
Pedestrians			1			1
Lane Width (ft)			12.0			12.0
Walking Speed (ft/s)			4.0			4.0
Percent Blockage			0			0
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (ft)			717			401
pX, platoon unblocked	0.75	0.75			0.75	
vC, conflicting volume	1086	672			609	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	945	391			306	
tC, single (s)	6.8	6.9			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	15			100	
cM capacity (veh/h)	197	456			945	


















Direction, Lane #	WB 1	NB 1	SB 1	SB 2
Volume Total	386	733	414	414
Volume Left	0	0	0	0
Volume Right	386	124	0	0
cSH	456	1700	1700	1700
Volume to Capacity	0.85	0.43	0.24	0.24
Queue Length 95th (ft)	211	0	0	0
Control Delay (s)	43.3	0.0	0.0	0.0
Lane LOS	E			
Approach Delay (s)	43.3	0.0	0.0	
Approach LOS	E			

Intersection Summary			
Average Delay		8.6	
Intersection Capacity Utilization		67.3%	ICU Level of Service C
Analysis Period (min)		15	

HCM Unsignalized Intersection Capacity Analysis

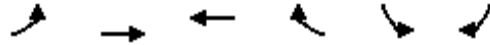
6: FM Fuel/US Bank & FM Main Drive Aisle

11/24/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control		Stop			Stop			Stop				Stop
Volume (vph)	20	62	47	175	159	65	149	5	64	15	5	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	67	51	190	173	71	162	5	70	16	5	27
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	SB 1							
Volume Total (vph)	22	118	434	237	49							
Volume Left (vph)	22	0	190	162	16							
Volume Right (vph)	0	51	71	70	27							
Hadj (s)	0.50	-0.28	0.02	-0.01	-0.19							
Departure Headway (s)	6.4	5.6	5.1	5.5	5.7							
Degree Utilization, x	0.04	0.18	0.61	0.36	0.08							
Capacity (veh/h)	522	598	685	602	547							
Control Delay (s)	8.4	8.6	15.7	11.5	9.1							
Approach Delay (s)	8.6		15.7	11.5	9.1							
Approach LOS	A		C	B	A							
Intersection Summary												
Delay			13.0									
Level of Service			B									
Intersection Capacity Utilization			55.4%	ICU Level of Service	B							
Analysis Period (min)			15									

HCM Unsignalized Intersection Capacity Analysis
 7: FM Main Drive Aisle & FM Right-In Driveway

11/24/2014



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↑	↑
Sign Control		Stop	Stop		Stop	
Volume (vph)	0	149	262	0	105	125
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	0	166	291	0	117	139
Direction, Lane #	EB 1	WB 1	SB 1	SB 2		
Volume Total (vph)	166	291	117	139		
Volume Left (vph)	0	0	117	0		
Volume Right (vph)	0	0	0	139		
Hadj (s)	0.03	0.02	0.50	-0.63		
Departure Headway (s)	5.0	4.8	6.1	5.0		
Degree Utilization, x	0.23	0.39	0.20	0.19		
Capacity (veh/h)	679	715	555	674		
Control Delay (s)	9.5	10.8	9.4	8.0		
Approach Delay (s)	9.5	10.8	8.6			
Approach LOS	A	B	A			
Intersection Summary						
Delay			9.7			
Level of Service			A			
Intersection Capacity Utilization			31.8%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

8: FM Main Drive Aisle & FM RIRO Driveway

11/24/2014




















Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Sign Control		Stop	Stop		Stop	
Volume (vph)	23	127	155	5	42	96
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Hourly flow rate (vph)	26	144	176	6	48	109
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total (vph)	170	182	157			
Volume Left (vph)	26	0	48			
Volume Right (vph)	0	6	109			
Hadj (s)	0.05	0.01	-0.34			
Departure Headway (s)	4.5	4.5	4.4			
Degree Utilization, x	0.21	0.23	0.19			
Capacity (veh/h)	764	765	768			
Control Delay (s)	8.7	8.8	8.4			
Approach Delay (s)	8.7	8.8	8.4			
Approach LOS	A	A	A			
Intersection Summary						
Delay			8.6			
Level of Service			A			
Intersection Capacity Utilization			40.1%	ICU Level of Service	A	
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis

9: Brutscher Street & FM Main Drive Aisle/Columbia Bank

11/24/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (veh/h)	187	4	56	8	14	27	16	121	8	38	143	159
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	203	4	61	9	15	29	17	132	9	41	155	173
Pedestrians		1			1							
Lane Width (ft)		12.0			12.0							
Walking Speed (ft/s)		4.0			4.0							
Percent Blockage		0			0							
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (ft)											342	
pX, platoon unblocked												
vC, conflicting volume	529	501	243	559	584	137	329			141		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	529	501	243	559	584	137	329			141		
tC, single (s)	7.1	6.5	6.2	7.3	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.7	4.0	3.3	2.2			2.2		
p0 queue free %	51	99	92	98	96	97	99			97		
cM capacity (veh/h)	418	454	795	360	408	916	1241			1434		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1							
Volume Total	268	53	17	140	370							
Volume Left	203	9	17	0	41							
Volume Right	61	29	0	9	173							
cSH	469	569	1241	1700	1434							
Volume to Capacity	0.57	0.09	0.01	0.08	0.03							
Queue Length 95th (ft)	88	8	1	0	2							
Control Delay (s)	22.4	12.0	7.9	0.0	1.1							
Lane LOS	C	B	A		A							
Approach Delay (s)	22.4	12.0	0.9		1.1							
Approach LOS	C	B										
Intersection Summary												
Average Delay			8.5									
Intersection Capacity Utilization			58.4%		ICU Level of Service					B		
Analysis Period (min)			15									

HCM Signalized Intersection Capacity Analysis

12: Springbrook Road & Hayes Street

11/24/2014



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (vph)	93	84	593	39	70	688
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.5	4.5	4.5		4.5	4.5
Lane Util. Factor	1.00	1.00	1.00		1.00	1.00
Frbp, ped/bikes	1.00	0.97	1.00		1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00
Frt	1.00	0.85	0.99		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1679	1557	1778		1703	1759
Flt Permitted	0.95	1.00	1.00		0.26	1.00
Satd. Flow (perm)	1679	1557	1778		465	1759
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	97	88	618	41	73	717
RTOR Reduction (vph)	0	77	3	0	0	0
Lane Group Flow (vph)	97	11	656	0	73	717
Confl. Peds. (#/hr)	3	4				
Confl. Bikes (#/hr)				1		
Heavy Vehicles (%)	7%	1%	6%	3%	6%	8%
Turn Type	Perm	Perm	NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8	8			6	
Actuated Green, G (s)	7.1	7.1	32.1		39.9	39.9
Effective Green, g (s)	6.6	6.6	31.6		39.4	39.4
Actuated g/C Ratio	0.12	0.12	0.57		0.72	0.72
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	2.5	2.5	2.5		2.5	2.5
Lane Grp Cap (vph)	201	186	1021		407	1260
v/s Ratio Prot			c0.37		0.01	c0.41
v/s Ratio Perm	c0.06	0.01			0.12	
v/c Ratio	0.48	0.06	0.64		0.18	0.57
Uniform Delay, d1	22.6	21.4	7.9		4.2	3.7
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	1.3	0.1	3.1		0.2	1.9
Delay (s)	23.9	21.5	11.0		4.3	5.6
Level of Service	C	C	B		A	A
Approach Delay (s)	22.8		11.0			5.5
Approach LOS	C		B			A
Intersection Summary						
HCM 2000 Control Delay			9.7		HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio			0.63			
Actuated Cycle Length (s)			55.0		Sum of lost time (s)	13.0
Intersection Capacity Utilization			55.1%		ICU Level of Service	B
Analysis Period (min)			15			
c Critical Lane Group						

APPENDIX I
**SIMTRAFFIC
QUEUING REPORTS**

Queuing and Blocking Report
2015 Pre-Development Conditions - Weekday PM Peak Hour

11/21/2014

Intersection: 1: Springbrook Road & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB
Directions Served	UL	T	T	R	UL	T	T	R	L	L	T	R
Maximum Queue (ft)	217	377	393	78	213	462	468	394	182	262	636	259
Average Queue (ft)	112	287	259	16	141	371	373	205	138	189	523	178
95th Queue (ft)	238	475	482	142	302	572	568	529	229	323	1062	356
Link Distance (ft)		742	742		394	394	394					1026
Upstream Blk Time (%)					0	12	10	1				5
Queuing Penalty (veh)					0	79	66	0				22
Storage Bay Dist (ft)	225			240				265	190	190		190
Storage Blk Time (%)	3	18	14				20		4	6	53	5
Queuing Penalty (veh)	15	19	9				55		13	20	268	28

Intersection: 1: Springbrook Road & Highway 99W

Movement	SB	SB	SB	SB
Directions Served	L	L	T	R
Maximum Queue (ft)	225	285	293	138
Average Queue (ft)	188	245	230	76
95th Queue (ft)	255	350	489	175
Link Distance (ft)			505	
Upstream Blk Time (%)			9	
Queuing Penalty (veh)			0	
Storage Bay Dist (ft)	125	125		125
Storage Blk Time (%)	23	61	24	7
Queuing Penalty (veh)	63	167	110	35

Intersection: 4: Brutscher Street & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	SB	SB
Directions Served	UL	T	T	R	UL	T	T	R	L	TR	L	TR
Maximum Queue (ft)	44	351	398	214	346	700	663	107	305	132	26	69
Average Queue (ft)	24	201	221	60	303	529	487	24	242	67	11	34
95th Queue (ft)	56	414	447	226	422	1020	994	137	365	181	32	75
Link Distance (ft)		444	444			929	929			621		352
Upstream Blk Time (%)		0	1			19	9					
Queuing Penalty (veh)		0	8			0	0					
Storage Bay Dist (ft)	215			185	250			70	350		75	
Storage Blk Time (%)		10	16		49	8	22		1	0		1
Queuing Penalty (veh)		3	12		403	19	9		1	1		0

Queuing and Blocking Report
 2015 Pre-Development Conditions - Weekday PM Peak Hour

11/21/2014

Intersection: 7: FM Main Drive Aisle & FM Right-In Driveway

Movement	EB	WB	SB	SB
Directions Served	T	T	L	R
Maximum Queue (ft)	54	105	40	52
Average Queue (ft)	38	59	24	33
95th Queue (ft)	57	118	50	61
Link Distance (ft)	54	370		269
Upstream Blk Time (%)	1			
Queuing Penalty (veh)	1			
Storage Bay Dist (ft)			80	
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 8: FM Main Drive Aisle & FM RIRO Driveway

Movement	EB	WB	SB
Directions Served	LT	TR	LR
Maximum Queue (ft)	50	94	40
Average Queue (ft)	39	57	24
95th Queue (ft)	56	105	49
Link Distance (ft)	370	197	264
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Zone Summary

Zone wide Queuing Penalty: 1425

Queuing and Blocking Report
2015 Post-Development Conditions - Weekday PM Peak Hour

11/26/2014

Intersection: 1: Springbrook Road & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB
Directions Served	UL	T	T	R	UL	T	T	R	L	L	T	R
Maximum Queue (ft)	302	388	439	78	240	381	384	158	213	290	842	290
Average Queue (ft)	155	306	316	16	172	304	312	78	143	246	689	208
95th Queue (ft)	331	465	527	142	366	528	533	336	278	392	1041	369
Link Distance (ft)		742	742		394	394	394					1026
Upstream Blk Time (%)					9	7	6	0				6
Queuing Penalty (veh)					61	43	39	0				27
Storage Bay Dist (ft)	225			240				265	190	190		190
Storage Blk Time (%)	4	20	17				14		12	17	69	7
Queuing Penalty (veh)	22	21	11				39		42	62	354	41

Intersection: 1: Springbrook Road & Highway 99W

Movement	SB	SB	SB	SB
Directions Served	L	L	T	R
Maximum Queue (ft)	226	328	458	221
Average Queue (ft)	170	275	294	106
95th Queue (ft)	272	381	526	262
Link Distance (ft)			505	
Upstream Blk Time (%)			6	
Queuing Penalty (veh)			0	
Storage Bay Dist (ft)	125	125		125
Storage Blk Time (%)	16	65	41	6
Queuing Penalty (veh)	44	177	193	33

Intersection: 4: Brutscher Street & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	SB	SB
Directions Served	UL	T	T	R	UL	T	T	R	L	TR	L	TR
Maximum Queue (ft)	112	459	453	210	320	380	315	12	195	105	15	60
Average Queue (ft)	34	277	292	68	223	215	162	3	138	67	5	40
95th Queue (ft)	161	473	473	255	373	453	372	18	222	123	22	75
Link Distance (ft)		444	444			929	929			621		352
Upstream Blk Time (%)		3	3									
Queuing Penalty (veh)		18	19									
Storage Bay Dist (ft)	215			185	250			70	350		75	
Storage Blk Time (%)		16	23		17	1	13					1
Queuing Penalty (veh)		4	17		140	3	6					0

Queuing and Blocking Report
2015 Post-Development Conditions - Weekday PM Peak Hour

11/26/2014

Intersection: 7: FM Main Drive Aisle & FM Right-In Driveway

Movement	EB	WB	SB	SB
Directions Served	T	T	L	R
Maximum Queue (ft)	52	92	49	58
Average Queue (ft)	39	60	32	34
95th Queue (ft)	59	106	57	73
Link Distance (ft)	54	370		269
Upstream Blk Time (%)	1			
Queuing Penalty (veh)	1			
Storage Bay Dist (ft)			80	
Storage Blk Time (%)			0	0
Queuing Penalty (veh)			0	0

Intersection: 8: FM Main Drive Aisle & FM RIRO Driveway

Movement	EB	WB	SB
Directions Served	LT	TR	LR
Maximum Queue (ft)	59	65	36
Average Queue (ft)	45	47	26
95th Queue (ft)	66	73	47
Link Distance (ft)	370	197	264
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Zone Summary

Zone wide Queuing Penalty: 1419

Queuing and Blocking Report
2015 Pre-Development Conditions - Saturday Mid-day

11/21/2014

Intersection: 1: Springbrook Road & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB
Directions Served	UL	T	T	R	UL	T	T	R	L	L	T	R
Maximum Queue (ft)	298	543	510	156	358	380	404	157	160	195	209	115
Average Queue (ft)	164	377	362	31	286	223	241	31	112	142	142	61
95th Queue (ft)	328	589	567	206	486	429	451	208	191	215	239	143
Link Distance (ft)		742	742		394	394	394					1026
Upstream Blk Time (%)					13	2	3	0				
Queuing Penalty (veh)					72	14	14	0				
Storage Bay Dist (ft)	225			240				265	190	190		190
Storage Blk Time (%)	2	21	20				8		2	4	5	0
Queuing Penalty (veh)	11	23	13				9		6	11	22	2

Intersection: 1: Springbrook Road & Highway 99W

Movement	SB	SB	SB	SB
Directions Served	L	L	T	R
Maximum Queue (ft)	199	286	233	90
Average Queue (ft)	113	192	147	51
95th Queue (ft)	250	320	268	101
Link Distance (ft)			505	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	125	125		125
Storage Blk Time (%)	6	38	18	0
Queuing Penalty (veh)	12	80	67	1

Intersection: 4: Brutscher Street & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	SB	SB
Directions Served	UL	T	T	R	UL	T	T	R	L	TR	L	TR
Maximum Queue (ft)	52	356	379	101	244	373	327	107	200	99	19	42
Average Queue (ft)	20	203	225	26	137	215	189	27	152	59	10	21
95th Queue (ft)	63	402	416	131	287	392	363	139	233	106	24	55
Link Distance (ft)		444	444			929	929			621		352
Upstream Blk Time (%)				0								
Queuing Penalty (veh)				0								
Storage Bay Dist (ft)	215			185	250			70	350		75	
Storage Blk Time (%)		6	10		0	7	20					3
Queuing Penalty (veh)		2	5		3	11	10					0

Queuing and Blocking Report
2015 Pre-Development Conditions - Saturday Mid-day

11/21/2014

Intersection: 7: FM Main Drive Aisle & FM Right-In Driveway

Movement	EB	WB	SB	SB
Directions Served	T	T	L	R
Maximum Queue (ft)	49	123	48	59
Average Queue (ft)	37	68	30	34
95th Queue (ft)	52	123	60	67
Link Distance (ft)	54	370		269
Upstream Blk Time (%)	1			
Queuing Penalty (veh)	1			
Storage Bay Dist (ft)			80	
Storage Blk Time (%)				0
Queuing Penalty (veh)				0

Intersection: 8: FM Main Drive Aisle & FM RIRO Driveway

Movement	EB	WB	SB
Directions Served	LT	TR	LR
Maximum Queue (ft)	50	65	50
Average Queue (ft)	37	43	28
95th Queue (ft)	53	74	56
Link Distance (ft)	370	197	264
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Zone Summary

Zone wide Queuing Penalty: 389

Queuing and Blocking Report
2015 Post-Development Conditions - Saturday Mid-day

11/21/2014

Intersection: 1: Springbrook Road & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	NB	NB
Directions Served	UL	T	T	R	UL	T	T	R	L	L	T	R
Maximum Queue (ft)	290	564	605	233	328	394	413	158	182	228	225	196
Average Queue (ft)	162	453	456	62	223	265	282	62	108	152	150	102
95th Queue (ft)	360	716	741	297	408	465	477	299	201	269	297	210
Link Distance (ft)		742	742		394	394	394					1026
Upstream Blk Time (%)		2	1		3	3	4	0				
Queuing Penalty (veh)		0	0		16	17	23	0				
Storage Bay Dist (ft)	225			240				265	190	190		190
Storage Blk Time (%)		35	32				11		1	5		7
Queuing Penalty (veh)		38	20				13		3	13		30

Intersection: 1: Springbrook Road & Highway 99W

Movement	SB	SB	SB	SB
Directions Served	L	L	T	R
Maximum Queue (ft)	187	242	162	67
Average Queue (ft)	123	184	97	44
95th Queue (ft)	243	287	220	85
Link Distance (ft)			505	
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	125	125		125
Storage Blk Time (%)	7	38	13	
Queuing Penalty (veh)	15	79	49	

Intersection: 4: Brutscher Street & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	SB	SB
Directions Served	UL	T	T	R	UL	T	T	R	L	TR	L	TR
Maximum Queue (ft)	43	360	371	87	210	340	307	23	238	136	24	52
Average Queue (ft)	15	189	203	29	132	198	168	10	158	84	9	26
95th Queue (ft)	48	386	405	133	253	371	333	33	266	153	29	62
Link Distance (ft)		444	444			929	929			621		352
Upstream Blk Time (%)			1									
Queuing Penalty (veh)			6									
Storage Bay Dist (ft)	215			185	250			70	350		75	
Storage Blk Time (%)		7	13		6	3	18					1
Queuing Penalty (veh)		2	6		40	6	9					0

Queuing and Blocking Report
2015 Post-Development Conditions - Saturday Mid-day

11/21/2014

Intersection: 7: FM Main Drive Aisle & FM Right-In Driveway

Movement	EB	WB	SB	SB
Directions Served	T	T	L	R
Maximum Queue (ft)	62	76	44	49
Average Queue (ft)	42	54	34	31
95th Queue (ft)	66	83	57	59
Link Distance (ft)	54	370		269
Upstream Blk Time (%)	2			
Queuing Penalty (veh)	2			
Storage Bay Dist (ft)			80	
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 8: FM Main Drive Aisle & FM RIRO Driveway

Movement	EB	WB	SB
Directions Served	LT	TR	LR
Maximum Queue (ft)	58	53	36
Average Queue (ft)	43	38	19
95th Queue (ft)	71	66	45
Link Distance (ft)	370	197	264
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Zone Summary

Zone wide Queuing Penalty: 389

Queuing and Blocking Report

2017 Post-Development Conditions - Weekday PM Peak Hour

11/26/2014

Intersection: 1: Springbrook Road & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	NB	NB	NB
Directions Served	UL	T	T	R	UL	L	T	T	R	L	L	T
Maximum Queue (ft)	278	472	442	156	261	295	319	360	78	174	171	178
Average Queue (ft)	179	261	240	31	213	237	203	230	16	116	99	170
95th Queue (ft)	340	494	462	205	376	439	391	438	142	231	221	186
Link Distance (ft)		729	729		392	392	392	392				
Upstream Blk Time (%)					5	15	1	1				
Queuing Penalty (veh)					25	78	3	7				
Storage Bay Dist (ft)	225			240					265	190	190	
Storage Blk Time (%)	8	17	11					4		0	0	2
Queuing Penalty (veh)	41	21	14					13		1	0	13

Intersection: 1: Springbrook Road & Highway 99W

Movement	NB	SB	SB	SB	SB
Directions Served	R	L	L	T	R
Maximum Queue (ft)	186	226	319	397	209
Average Queue (ft)	144	179	265	295	117
95th Queue (ft)	221	280	388	517	306
Link Distance (ft)				503	
Upstream Blk Time (%)				6	
Queuing Penalty (veh)				0	
Storage Bay Dist (ft)	190	125	125		125
Storage Blk Time (%)	5	23	66	44	0
Queuing Penalty (veh)	37	72	209	216	0

Intersection: 4: Brutscher Street & Highway 99W

Movement	EB	EB	EB	EB	WB	WB	WB	WB	NB	NB	SB	SB
Directions Served	UL	T	T	R	UL	T	T	R	L	TR	L	TR
Maximum Queue (ft)	108	451	460	224	364	837	799	69	326	252	15	51
Average Queue (ft)	30	243	260	62	339	602	538	19	242	96	5	29
95th Queue (ft)	154	495	524	227	411	1094	1094	104	389	310	18	65
Link Distance (ft)		444	444			929	929			621		352
Upstream Blk Time (%)		2	6			15	7					
Queuing Penalty (veh)		16	44			0	0					
Storage Bay Dist (ft)	215			185	250			70	350		75	
Storage Blk Time (%)		16	19		55	12	20	0	10	1		1
Queuing Penalty (veh)		4	14		471	31	9	0	13	2		0

Queuing and Blocking Report
2017 Post-Development Conditions - Weekday PM Peak Hour

11/26/2014

Intersection: 7: FM Main Drive Aisle & FM Right-In Driveway

Movement	EB	WB	SB	SB
Directions Served	T	T	L	R
Maximum Queue (ft)	65	101	49	64
Average Queue (ft)	40	69	27	43
95th Queue (ft)	68	118	55	79
Link Distance (ft)	54	370		269
Upstream Blk Time (%)	2			
Queuing Penalty (veh)	3			
Storage Bay Dist (ft)			80	
Storage Blk Time (%)				0
Queuing Penalty (veh)				0

Intersection: 8: FM Main Drive Aisle & FM RIRO Driveway

Movement	EB	WB	SB
Directions Served	LT	TR	LR
Maximum Queue (ft)	46	70	40
Average Queue (ft)	38	48	21
95th Queue (ft)	55	74	54
Link Distance (ft)	370	197	264
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Zone Summary

Zone wide Queuing Penalty: 1359

APPENDIX J
**TRANSIT BUS LINE
SCHEDULES**

Newberg Weekdays



Yamhill County Transit Area
 535 NE 5th Street
 McMinnville, OR 97128
 phone: 503.474.4900 or 503.538.7433
 web: www.yctransitarea.org
 email: saunderst@co.yamhill.or.us

Fixed Routes – Flag Stops are allowed on Rts. 5&7

How to read this schedule:

Find the stop where you will board the bus.
 Read top to bottom to find scheduled departure times at the listed stops.
 Read from left to right to find how long it takes to travel between stops.

Place correct fare in farebox; exact fare only – drivers do not carry change.
 You may flag down a bus along any city fixed route. Schedules are subject to change without notice. For the most current schedule check the YCTA website.

Holiday Schedule:

YCTA buses do not operate on the following holidays: New Year’s Day, Memorial Day, Fourth of July, Labor Day, Thanksgiving Day, and Christmas Day.

Pets:

Service animals are always welcome. Pets may travel in a pet carrier as long as they present no health or safety risk.

Key:

AM schedule in regular print.
PM schedule in **bold** print.

Newberg Foothills Drive, Fixed Route 5 – Weekdays

Naps Thriftway	George Fox (Meridian St)	The Oaks	Senior Center	Main and Columbia	Woodview Village	Naps Thriftway
1	2	3	4	5	6	7
7:30	7:33	7:34	7:39	7:42	7:50	7:55
8:30	8:33	8:34	8:39	8:42	8:50	8:55
9:30	9:33	9:34	9:39	9:42	9:50	9:55
10:30	10:33	10:34	10:39	10:42	10:50	10:55
11:30	11:33	11:34	11:39	11:42	11:50	11:55
12:30	12:33	12:34	12:39	12:42	12:50	12:55
1:30	1:33	1:34	1:39	1:42	1:50	1:55
2:30	2:33	2:34	2:39	2:42	2:50	2:55
3:30	3:33	3:34	3:39	3:42	3:50	3:55
4:30	4:33	4:34	4:39	4:42	4:50	4:55
5:30	5:33	5:34	5:39	5:42	5:50	5:55

Newberg Providence, Fixed Route 7 - Weekdays

Naps Thriftway	Colonial Village	CPRD/FISH (Elliott Rd)	Fred Meyer	PCC	Providence Hospital	Safeway	Naps Thriftway
1	2	3	4	5	6	7	8
7:00	7:04	7:09	7:13	7:15	7:19	7:23	7:29
8:00	8:04	8:09	8:13	8:15	8:19	8:23	8:29
9:00	9:04	9:09	9:13	9:15	9:19	9:23	9:29
11:00	11:04	11:09	11:13	11:15	11:19	11:23	11:29
12:00	12:04	12:09	12:13	12:15	12:19	12:23	12:29
1:00	1:04	1:09	1:13	1:15	1:19	1:23	1:29
2:00	2:04	2:09	2:13	2:15	2:19	2:23	2:29
3:00	3:04	3:09	3:13	3:15	3:19	3:23	3:29
4:00	4:04	4:09	4:13	4:15	4:19	4:23	4:29
5:00	5:04	5:09	5:13	5:15	5:19	5:23	5:29
6:00	6:04	6:09	6:13	6:15	6:19	6:23	6:29

McMinnville – 99w



Yamhill County Transit Area

535 NE 5th Street
 McMinnville, OR 97128
 phone: 503.474.4900 or 503.538.7433
 web: www.yctransitarea.org
 email: saunderst@co.yamhill.or.us

Key:

AM schedule in regular print

PM schedule in **bold** print

Please call ahead to request stops at the Dayton RV Park and Providence Hospital in Newberg.

How to read schedule:

Find the stop where you will board the bus.

Read top to bottom to find scheduled departure times at the listed stops.

Read from left to right to find travel time between stops.

Exact fare only – drivers do not carry change.

Schedules are subject to change without notice. For the most current schedule check the YCTA website: www.yctransitarea.org

Holiday Schedule: YCTA buses do not operate on the following holidays: New Year's Day, Memorial Day, Fourth of July, Labor Day, Thanksgiving Day, and Christmas Day.

McMinnville to Tigard Transit Center, Route 46s – Saturday

Transit Center	99w by Safeway	Lafayette (by Post Office)	Dayton (by City Hall)	Dundee (by Fire Dept)	Newberg (by Naps Thriftway)	Newberg (by Walgreens)	Newberg (Brutscher St by Today's Vision)	Sherwood (Langer Dr by Shari's)	Tigard Transit Center
8:00	8:04	8:10	8:17	8:29	8:32	8:34	8:37	8:52	9:08
10:40	10:44	10:50	10:57	11:09	11:12	11:14	11:17	11:32	11:48
2:15	2:19	2:25	2:32	2:44	2:47	2:49	2:52	3:07	3:23
4:55	4:59	5:05	5:12	5:24	5:27	5:29	5:32	5:47	6:03

Tigard Transit Center to McMinnville, Route 46s – Saturday

Tigard Transit Center	Sherwood (Langer Dr by Shari's)	Newberg (by Shari's)	Newberg (by J's Restaurant)	Newberg (by Naps Thriftway)	Dundee (by Women's Club)	Dayton (by City Hall)	Lafayette (by City Hall)	McMinnville Town Center	Transit Center
9:23	9:38	9:51	9:54	10:00	10:05	10:14	10:21	10:30	10:35
12:03	12:18	12:31	12:34	12:40	12:45	12:54	1:01	1:10	1:15
3:38	3:53	4:06	4:09	4:15	4:20	4:29	4:36	4:45	4:50
6:18	6:33	6:46	6:49	6:55	7:00	7:09	7:16	7:25	7:30

McMinnville to Tigard Transit Center, Route 44 – Weekdays

Transit Center	Lafayette Ave & Riverside Dr	Lafayette Ave by Orchards	Lafayette Ave by Redwood Commons	Lafayette Ave & 99w by Mazatlan	Lafayette (by Post Office)	Dayton (by City Hall)	Dundee (by Fire Dept)	Newberg (by Naps Thriftway)	Newberg (by Walgreens)	Newberg (Brutscher St by Today's Vision)	Sherwood (Langer Dr by Shari's)	99w & 124th	Tigard Transit Center
5:10	5:11	5:12	5:13	5:14	5:22	5:29	5:41	5:44	5:46	5:49	6:04	6:11	6:22
6:25	6:26	6:27	6:28	6:29	6:37	6:44	6:56	6:59	7:01	7:04	7:19	7:26	7:37
7:25	7:26	7:27	7:28	7:29	7:37	7:44	7:56	7:59	8:01	8:04	8:19	8:24	8:37
10:35	10:36	10:37	10:38	10:39	10:47	10:54	11:06	11:09	11:11	11:14	11:29	11:36	11:47
12:15	12:16	12:17	12:18	12:19	12:27	12:34	12:46	12:49	12:51	12:54	1:09	1:14	1:27
1:15	1:16	1:17	1:18	1:19	1:27	1:34	1:46	1:49	1:51	1:54	2:09	2:14	2:25
3:20	3:21	3:22	3:23	3:22	3:30	3:37	3:52	3:55	3:57	4:00	4:18	4:25	4:37
5:40	5:40	5:41	5:41	5:42	5:50	5:57	6:12	6:15	6:17	6:20	6:32	6:39	6:51
6:12	6:11	6:12	6:13	6:14	6:22	6:29	6:44	6:47	6:49	6:52	7:10	7:17	7:29

McMinnville to Tigard Transit Center, Express Route 45x – PM Weekdays

Transit Center	Baker St by Oregon Mutual	Linfield Ave & Blaine St	Willamette Valley Med Ctr	Newberg (by Naps Thriftway)	Sherwood (Langer Dr by Shari's)	99w & 124th	Tigard Transit Center
5:05	5:07	5:12	5:14	5:30	5:50	5:57	6:06

Tigard Transit Center to McMinnville, Route 44 – Weekdays

Tigard Transit Center	99w & 124th	Sherwood (Langer Dr by Shari's)	Newberg (by Shari's)	Newberg (by J's Restaurant)	Newberg (by Naps Thriftway)	Dundee (by Women's Club)	Dayton (by City Hall)	Lafayette (by City Hall)	Lafayette Ave by Safeway	Redwood Commons	Lafayette Ave by Orchards	Lafayette Ave & Riverside Dr	Transit Center
-	-	-	-	-	6:45	6:50	6:59	7:06	7:13	7:14	7:15	7:16	7:20
7:48	7:53	8:03	8:16	8:19	8:25	8:30	8:39	8:46	8:53	8:54	8:55	8:56	9:00
8:48	8:58	9:03	9:16	9:19	9:25	9:30	9:39	9:46	9:53	9:54	9:55	9:56	10:00
11:58	12:08	12:13	12:26	12:29	12:35	12:40	12:49	12:56	1:03	1:04	1:05	1:06	1:10
1:38	1:48	1:53	2:06	2:09	2:24	2:32	2:44	2:51	2:58	2:59	3:00	3:01	3:05
2:38	2:48	2:53	3:06	3:09	3:24	3:32	3:44	3:51	3:58	3:59	4:00	4:01	4:05
4:47	4:57	5:04	5:17	5:20	5:26	5:32	5:41	5:48	5:55	5:56	5:57	5:58	6:02
6:16	6:26	6:35	6:54	6:57	7:03	7:11	7:23	7:33	7:43	7:44	7:45	7:46	7:50
7:01	7:11	7:20	7:33	7:36	7:42	-	-	-	-	-	-	-	-
7:39	7:49	7:54	8:07	8:10	8:16	8:24	8:36	8:46	8:56	8:57	8:58	8:59	9:03

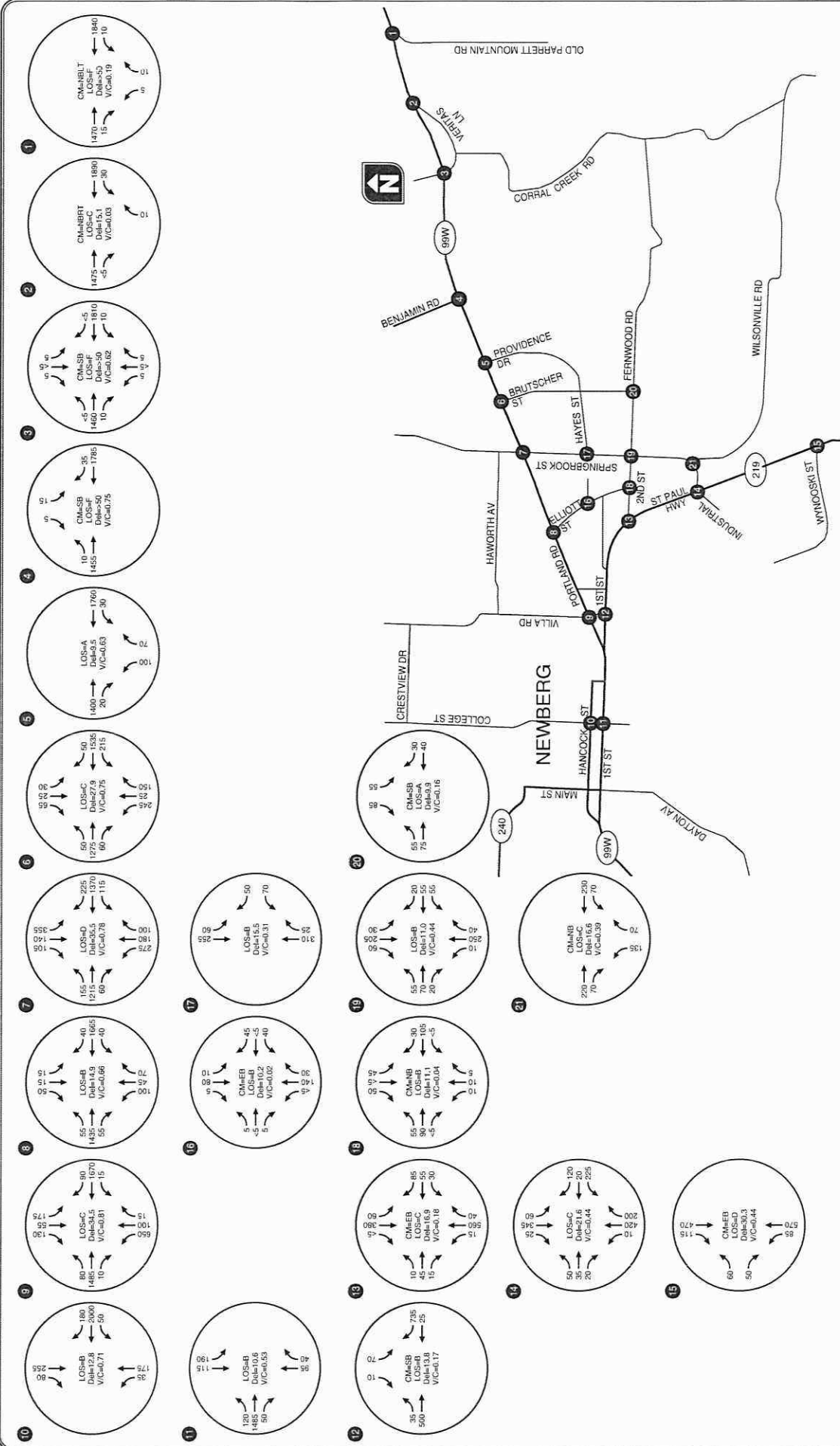
Tigard Transit Center to McMinnville, Express Route 45x – AM Weekdays

Tigard Transit Center	99w & 124th	Sherwood (Langer Dr by Shari's)	Newberg (by Naps Thriftway)	Willamette Valley Med Ctr	Linfield Ave & Blaine St	Baker St by Oregon Mutual	Transit Center
6:42	6:56	6:58	7:20	7:43	7:45	7:48	7:50

APPENDIX K
**NEWBERG DUNDEE
BYPASS PROJECT
VOLUMES**

September 2011

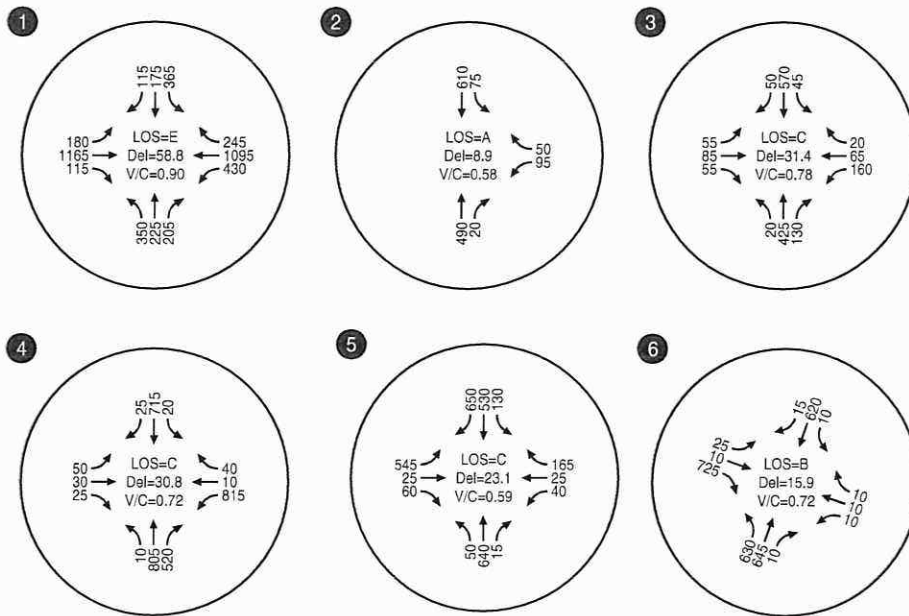
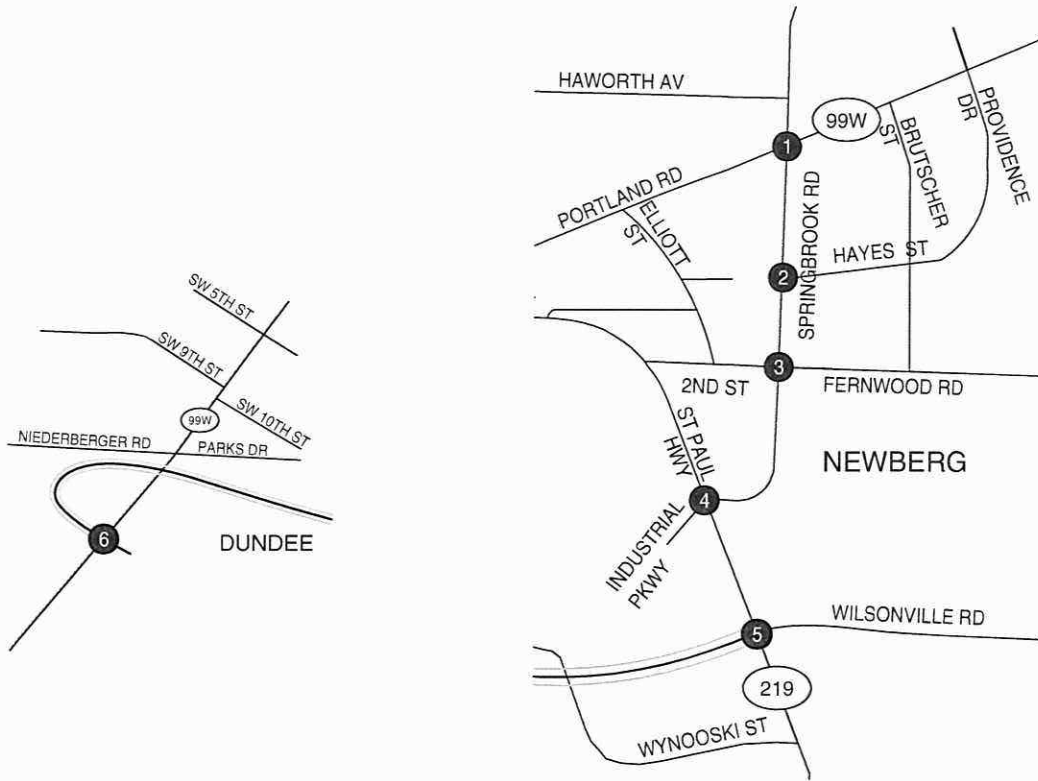
NDTIP



2011 EXISTING TRAFFIC CONDITIONS NEWBERG, OREGON

FIGURE 3A

H:\proj\18372pdx\4848\2011\Figs\Fig For Trans Tech Report\372_2016_2035_V\4848 Sep 16, 2011 - 9:31am - P:\18372pdx\4848\2011\Figs\Fig For Trans Tech Report\372_2016_2035_V\4848



LEGEND

- LOS = INTERSECTION LEVEL OF SERVICE
- Del = INTERSECTION AVERAGE CONTROL DELAY
- V/C = CRITICAL VOLUME-TO-CAPACITY RATIO

**YEAR 2016 PHASE 1 SCENARIO
DESIGN HOUR TRAFFIC CONDITIONS
NEWBERG, OREGON**

**FIGURE
7**

H:\profile\9372 - Newberg-Dundee Bypass\2012 July Analysis TSD\dwg\9372_Fig_2012_July.dwg Jul 25, 2012 - 11:39am - darguea Layout Tab: 01