

Exhibit H: Preliminary Stormwater Report

## Collina at Springbrook Newberg, Oregon

**Preliminary Stormwater Report** 

Date:	October 2022
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AKS Job Number:	4487-01



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#### Preliminary Stormwater Report Collina at springbrook newberg, oregon

#### **1.0** Purpose of Report

The purpose of this report is to analyze the effects the proposed development will have on the existing stormwater conveyance system; document the criteria, methodology, and informational sources used to design the proposed stormwater system; and present the results of the preliminary hydraulic analysis.

#### 2.0 Project Location/Description

The proposed residential subdivision will be located northeast of the intersection of E Mountainview Drive and N Center Street in Newberg Oregon, encompassing approximately 98.12 acres (Tax Lots 4900, 5000, 5100, 5200, 5300, 5400, 6200, and 6300 Yamhill County Assessor's Map 3 2 08).

The proposed development is a 405-lot residential subdivision with single-family detached homes. The site improvements will include the construction of public streets, underground utilities, stormwater facilities, and open space improvements.

Stormwater management is provided through two regional stormwater facilities. Runoff from the west portion of the site is treated and detained by an extended dry basin. Runoff from the north and east portion of the site will be treated and detained by a vegetated swale and a detention pond.

#### 3.0 Regulatory Design Criteria

Stormwater design criteria is dictated by the City of Newberg August 2015 *Public Works Design and Construction Standards* (CoN PWDCS). Per Figure 4.4, the proposed development will create more than 2,877 square feet of impervious area and will therefore be required to provide treatment and detention for all net new impervious area.



#### 4.6 Water Quantity and Quality Facilities

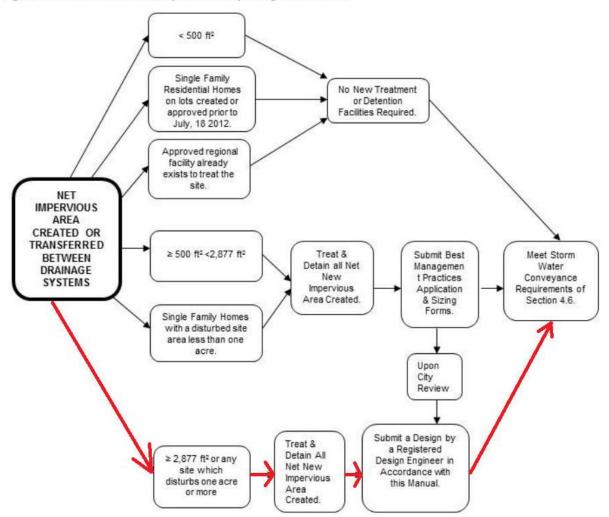


Figure 4.4 Storm water Quality & Quantity Design Flow Chart

#### 3.1. Stormwater Quantity

The CoN PWDCS requires that the post-development runoff rates from the site do not exceed the predevelopment runoff rates. This requirement will be met with detention through two stormwater management facilities.

Section 4.7.1.III Water Quantity Facility Design & Control Standards (CoN PWDCS)

Stormwater quantity on-site detention facilities shall be designed to capture runoff so the post-development runoff rates from the site do not exceed the pre-development runoff rates from the site, based on 24-hour storm events ranging from ½ of the 2-year return storm to the 25-year return storm. Specifically, the ½ of the 2, 2, 10, and 25-year post-development runoff rates will not exceed their respective ½ of the 2, 2, 10, and 25-year pre-development runoff rates...



#### 3.2. Stormwater Quality

The CoN PWDCS, requires that stormwater quality facilities be designed based on the following:

Section 4.8.5 Water Quality Storm (CoN PWDCS)

The storm defines both volume and rate of runoff. The stormwater quality only facilities shall be designed for a dry weather storm event totaling 1.0 inches of precipitation falling in 24 hours with an average storm return period of 96 hours using Figure 4-3, rainfall distribution.

Stormwater quality management for this project will be met using a vegetated swale and an extended dry basin. All facilities will be designed in accordance with City of Newberg Standards during final engineering.

#### 4.0 Design Methodology

The Santa Barbara Urban Hydrograph (SBUH) Method was used to analyze stormwater runoff from the site. This method utilizes the Natural Resource Conservation Service (NRCS) Type 1A 24-hour design storm. HydroCAD 10.0 computer software aided in the analysis. Runoff Curve Numbers (CN), which are representative of existing and developed cover conditions and time of concentration (Tc) values were developed in accordance with the U.S. Department of Agriculture (USDA) – Natural Resource Conservation Service's (NRCS) Technical Release 55 and are included in the Appendix.

#### 5.0 Design Parameters

#### 5.1. Design Storms

Per City of Newberg requirements, the following rainfall intensities and durations were utilized in the analysis of the existing and proposed stormwater facilities:

<b>Recurrence Interval</b>	Rainfall Event	<b>Total Precipitation Depth</b>
(Years)	(Hours)	(Inches)
Water Quality	24.0	1.00
½ of 2	24.0	1.25
2	24.0	2.50
10	24.0	3.50
25	24.0	4.00

#### Table 5-1: Rainfall Intensities

#### 5.2. Pre-Developed Site Conditions

#### 5.2.1. Site Topography

The existing topography divides the subject site into six separate subbasins, as shown on the Pre-Developed Basin Delineation (Figure 2A-2D). The general topography of each basin is as follows:

• Subbasin #1 (Node 1P)

Subbasin grades generally vary from  $\pm 2\%$  to  $\pm 5\%$  towards the Southwest with a high point of  $\pm 271$  feet near the southern stub of N Aldersgate Drive and a low point of  $\pm 230$  feet near the southern property line. The subbasin drains Southwest to an existing storm system in E Mountainview Drive, then South within S Center Street.



• Subbasin #2 (Node 2P)

Subbasin grades generally vary from  $\pm 3\%$  to  $\pm 8\%$  towards the East and South to Hess Creek. Subbasin #2 has a high point of  $\pm 303$  feet near the Northwest property corner and a low point of  $\pm 184$  feet near the Southeast property corner.

• Subbasin #3 (Node 3P)

Subbasin grades generally vary from  $\pm 1\%$  to  $\pm 3\%$ , with the subbasin draining west along E Mountainview Drive. Subbasin runoff enters an existing storm system that discharges to Tax Lot 900 of Tax Map 3.2.18, South of E Mountainvew Drive and West of N Center Street.

• Subbasin #4 (Node 4P)

Subbasin grades generally vary from  $\pm 3\%$  to  $\pm 5\%$ , with the subbasin draining north along N Center Street. Subbasin #4 outfalls into curb inlets of an existing storm system further north along N Center Street. The existing storm system generally conveys flows toward N College Street.

• Subbasin #5 (Node 5P)

Subbasin grades generally vary from  $\pm 2\%$  to  $\pm 5\%$ , with the subbasin draining South along N Aldersgate Drive then West along E Edgewood Drive. Subbasin #5 outfalls into curb inlets of an existing storm system further west along E Edgewood Drive. The existing storm system generally conveys flows toward N College Street.

• Subbasin #6 (Node 6P)

Subbasin grades generally vary from  $\pm 3\%$  to  $\pm 5\%$ , with the subbasin draining South along N Aldersgate Drive. Curb inlets along N Aldersgate Drive convey stormwater west along E Vintage Street outfalling to a stormwater facility within Tax lot 11900 of Tax map 3.2.08CB. Outfalls from the existing stormwater facility generally flows toward N College Street.

Tuble 5 2. Existing impervious Area						
Subbasin	Basin Area (sf)	Impervious Area (ac)				
#1	±29.49	±0.60				
#2	±71.76	±0.91				
#3	±0.58	±0.35				
#4	±0.12	±0.10				
#5	±0.21	±0.18				
#6	±0.06	±0.06				
Total	±102.22	±2.20				

Table 5-2: Existing I	mpervious Area
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#### 5.2.2. Land Use

The existing site consists of a house, hazelwood orchard, farmed fields, and brush/wooded areas.

#### 5.3. Soil Type

The soil beneath the project site and associated drainage basins are listed below, according to the USDA Natural Resources Conservation Service (NRCS) Soil Survey for Yamhill County. The following table outlines the Hydrologic Soil Group rating for the soil type:



Table 5-5. Hydrologic Soli Groupings						
NRCS Map Unit	NRCS Soil Classification	Hydrologic Soil				
Identification		Group Rating				
2013A	Wapato silty clay loam	C/D				
2216C	Chehalem silty clay	C/D				
2300A	Aloha silt loam	C/D				
2301A	Amity silt loam	C/D				
2310C	Woodburn silt loam	С				
2310D	Woodburn silt loam	С				
2310F	Woodburn silt loam	С				
2311C	Helvetia silt loam	С				
2775F	Saum-Ritner complex	С				
2784C	Witzel-Ritner complex	D				
2798D	Witham silty clay loam	D				

#### Table 5-3: Hydrologic Soil Groupings

Further information on this soil type is included in the NRCS Soil Resource Report located in the Appendix of this report.

#### 5.4. Post-Developed Site Conditions

#### 5.4.1. Site Topography

The on-site slopes will be modified with cuts and fills to accommodate the construction of public streets, stormwater facilities, and open space areas. Additionally, sloped residential building pads will be constructed adjacent to the public right-of-way.

General descriptions for subbasins are as follows:

• Subbasin #1 (Node 1S1 and 1S2)

Runoff within this subbasin is directed to Regional Stormwater Facility #1 located at the northeast corner of N Center Street and E Mountainview Drive and discharges into the existing 24" stormwater main flowing south within S Center Street. Post-developed Subbasin #1 was expanded to encompass a portion of Pre-Developed Subbasin #3 along E Mountainview Drive and a portion of Pre-Developed Subbasin #4 along N Center Street.

• Subbasin #2 (Node 2S1)

Runoff within this subbasin is directed to Regional Stormwater Facility #2 located at the northeast corner of N Villa Road and E Mountainview Drive and discharges directly to Hess Creek. Post-developed Subbasin #2 was expanded to encompass a portion of Pre-Developed Subbasin #5 along E Aldersgate Drive and the entirety of Pre-Developed Subbasin #6.

• Subbasin #3 (Node 3S1)

Runoff within this subbasin enters an existing storm system that discharges to Tax Lot 900 of Tax Map 3.2.18, as with the Pre-Developed Site Condition. This subbasin includes a proposed widening of E Mountainview Drive.



• Subbasin #4 (Nodes 4S1)

Runoff within this subbasin discharge to the same curb inlets along N Center Street as with the Pre-Development condition. This subbasin includes a portion of a proposed traffic circle at the intersection of N Center Street and E Henry Road.

• Subbasin #5 (Node 5S1)

Runoff within this subbasin discharge to the same curb inlets along E Edgewood Drive as with the Pre-Development condition. This subbasin includes reconstruction of the intersection of N Aldersgate Drive and E Edgewood Drive.

• Subbasin #6

This subbasin includes a reconstruction of East side of E Vintage Street from a three-quarter street into a full street. With this reconstruction, the East side of E Vintage Street is incorporated into Subbasin #2.

#### 5.4.2. Land Use

The post-developed site land use will consist of a 405-lot, single-family residential subdivision, with associated streets, sidewalks, and underground utilities.

#### 5.4.3. Post-Developed Site Parameters

Appendices A, B, and C provide the HydroCAD reports that were generated for the analyzed storm events. These reports include all the parameters (e.g. impervious/pervious areas, time of concentration, etc.) used to model the site hydrology.

5.4.4. Description of Off-Site Contributing Basins

There are no major off-site contributory basins draining onto the subject site.

#### 6.0 Stormwater Analyses

City of Newberg requires that all net new impervious area be treated and detained. With the proposed stormwater quality control facilities, this site will be able to treat the majority of the new impervious area. A portion of the new impervious area cannot be conveyed to the proposed stormwater facilities due to topographic constraints and conflicts with existing storm systems. These new untreated impervious areas are offset by treating existing untreated impervious area that can be routed to the stormwater facilities. Tabulation and mapping of these areas can be found on Figures 4A-4D, Impervious Area Maps.



#### 6.1. **Proposed Stormwater Quality Control Facilities**

6.1.1. **Regional Stormwater Facility #1** 

At the Southwest corner of the development, an Extended Dry Basin is proposed, to provide water quality treatment for impervious areas. The following contains calculations for the water quality treatment through the basin.



#### WATER QUALITY CALCULATIONS

Regional Stormwater Facility #1 - Extended Dry Basin (Node P1)

Client: Pahlisch Homes Inc. Project: Colina at Springbrook AKS Job No.: 4487-01 Date: 10/7/2022 Done By: GPS Checked By: TS

Total Subbasin Area (Node 1S1):	25.27	acres	
Total Subbasin Area (Node 1S1):	1,100,942	square feet (sf)	

#### **IMPERVIOUS AREA**

Total Impervious Area:	734,364	sf		
Existing Untreated Impervious Area	8,305	sf		22
Proposed Impervious ROW/Open Space Area	323,102	sf		
Total Impervious Lot Area	402,957	sf		
Impervious Lots Area (< 2,877 sf)**	57,717	sf	sf (20 Cluster Lots)	
Impervious Lots Area (> 2,877 sf)*	345,240	sf	sf (249 Lots)	

\*Lots over 2,877 sf in total area are assumed to contain 2,877 sf of impervious area

\*\*Lots under 2,877 sf in total area are assumed to contain 90% of its total area as impervious area

#### WATER QUALITY VOLUME (WQV)

(Per CON 4.8.5)

WQV (Subdivision) =

1.0 inch storm over 24 hrs

61,197 cubic feet

WATER QUALITY FLOW (WQF)

(Per CON Detail 461)

WQV (cf) WQF =-= 0.35 cfs 48\*60\*60



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#### EXTENDED DRY BASIN DESIGN & CALCULATIONS

#### Hydraulic Design Criteria (Per CON Design Standards Section 4.8 & Detail 461)

Permanent Pool Depth: 0.4 ft Permanent Pool covers bottom of basin Water Quality Drawdown Time: 48 hours Maximum Depth of WQ Pool: 4 ft Avoid direct flow across WQ pond to avoid short circuiting

#### Extended Dry Basin Sizing Design:

Bottom Slope	Minimum Bottom Width	Side Slopes	Top of Pond Elev.	Perm. Pool Depth	Pool Bottom Area	Bottom of Pool Elev.
(ft/ft)	(ft)	H:V	(ft)	(ft)	(sf)	(ft)
0.0	75	3.0	230.5	0.4	13,412	224.50

#### Water Quality Flow Hydraulic Calculations:

Q	Pool Elev. at WQV	Orifice CL Height	Calculated Orifice Diameter	Max. Pool Elev., 25-yr Event	Calculated WQV Pool Depth	Calculated WQV
(cfs)	(ft)	(ft)	(in)	(ft)	(ft)	(cf)
0.35	228.30	223.5	2.7	229.3	3.80	62,314

#### Check Against Design Criteria:

	Calculated		Meet CON Criteria?		
Minimum Freeboard:	1.3	feet	Yes more than	1	foot
Minimum Bottom Width:	75.0	feet	Yes greater than	4	feet
Maximum Pool Depth at WQV:	3.8	feet	Yes less than	4	feet
Design Pond WQV:	62,314	cu. ft	Yes greater than	61,197	cu. ft



#### 6.1.2. Regional Stormwater Facility #2

At the Southeast corner of the proposed site, a vegetated swale is proposed to treat the runoff from impervious surfaces. The following contains calculations and sizing for the vegetated swale.



#### WATER QUALITY CALCULATIONS

Regional Stormwater Facility #2 - Vegetated Swale (Node S1)

Client: Pahlisch Homes Inc. Project: Collina at Springbrook AKS Job No.: 4487-01 Date: October 7, 2022 Done By: GPS Checked By: TS

Total Subbasin Area (Node 2S2):	51.91	acres
Total Subbasin Area (Node 2S2):	2,261,386	square feet (sf)

#### **IMPERVIOUS AREA**

Total Impervious Area:	1,454,312	sf	
Ex. Untreated Impervious Area	37,317	sf	
Prop. Imp. ROW/Open Space Area	653,436	sf	
Total Impervious Lot Area	763,559	sf	
Impervious Area of Lots (< 2,877 sf)**	47,186	sf (20 Cluster Lots)	
Impervious Area of Lots (> 2,877 sf)*	716,373	sf (249 Lots)	

\*Lots over 2,877 sf in total area are assumed to contain 2,877 sf of impervious area \*\*Lots under 2,877 sf in total area are assumed to contain 90% of its total area as impervious area

#### WATER QUALITY VOLUME (WQV)

(Per CoN 4.7.2 & 4.8.4-5)

WQV (Subdivision) =  $\frac{1" X \text{ Area (ft)}}{12" \text{ per ft}}$  = **121,193 cubic feet** 

#### WATER QUALITY FLOW (WQF)

(Per CoN 4.7.2 & 4.8.4-5)

 $WQF = \frac{WQV (sf)}{86,400 \text{ seconds}} = 1.40 \text{ cfs}$ 



#### VEGETATED SWALE, WATER QUALITY FLOW DESIGN & CALCULATIONS

#### Hydraulic Design Criteria (Per CoN Standard Drawing 460)

Design Flow: Water Quality Flow Minimum Hydraulic Residence Time: 9 minutes Maximum Water Design Depth: 0.5-ft Minimum Freeboard: 1.0 foot (for facilities not protected from high flows) Manning's "n" Value: 0.24 Maximum Velocity: 2.0 fps based on the 25-YR flow

#### Swale Sizing Assumptions:

Slope	Bottom Width	Manning's #	Side Slope	Depth of Swale	Length
(ft/ft)	(ft)	"n"	H:V	(ft)	(ft)
0.005	10	0.24	4:1	1	132

#### Water Quality Flow Hydraulic Calculations (See Hydraflow Printouts):

[	Q	Flow Depth	Flow Area	Wp	R	Velocity
	(cfs)	(ft)	(sf)	(ft)	(ft)	(fps)
	1.40	0.48	5.77	13.99	0.41	0.24

#### 25-Year Flow Hydraulic Calculations (See HydroCAD Printouts):

Q	Flow Depth	Velocity
(cfs)	(ft)	(fps)
2.47	0.7	0.30

#### Check Against Design Criteria:

	Calculated		CoN Criteria		eria	
Minimum Hydraulic Residence Time:	9.1	minutes	>	9	minutes	Yes
Maximum Water Quality Design Depth:	0.48	feet	<	0.5	feet	Yes
Minimum Length:	132	feet	≥	100	feet	Yes
Maximum Velocity (25 yr):	0.3	fps	<	2	fps	Yes

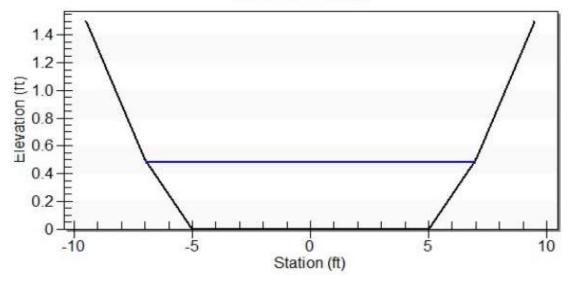
Meet CoN Criteria?



#### SWALE HYDRAULICS ANALYSIS REPORT

Result Parameter	rs	
Flow	1.40	cfs
Depth	0.484	ft
Area of Flow	5.77	sq ft
Wetted Perimeter	13.987	ft
Hydraulic Radius	0.413	ft
Average Velocity	0.243	fps
Top Width (T)	13.868	ft
Froude Number	0.066	
Critical Depth	0.084	ft
Critical Velocity	1.617	fps
Critical Slope	1.94496	ft/ft
Critical Top Width	10.67	ft
Max Shear Stress	0.151	lb/ft^2
Avg Shear Stress	0.129	lb/ft^2
Composite Manning's n Equation L	otter meth	od
Manning's Roughness	0.24	

#### **Cross Section**



#### 6.2. Proposed Stormwater Quantity Control Facilities

Stormwater quantity for this project will be met separately within each basins. The detention ponds within subbasins #1 and #2 will be designed to release the post-developed peak flow at or below the predevelopment rate as required by City of Newberg standards.

Calculations and contributing basins for all stormwater quantity control facilities can be referenced in the Appendix. The table below outlines the pre- and post-development flow rate comparisons for each subbasin:



Recurrence Interval	Peak Pre-Development	Peak Post-Development	Peak Flow Increase or			
(Years)	Flow (cfs) (Node 1P)	Flow (cfs) (Node CS)	(Decrease) – (cfs)			
½ of 2	0.69	0.37	(0.32)			
2	5.09	2.64	(2.45)			
10	9.75	7.86	(1.89)			
25	12.24	11.35	(0.89)			

Table 6-1: Subbasin #1 Peak Pre- and Post-Development Flow Comparisons

#### Table 6-2: Subbasin #2 Peak Pre- and Post-Development Flow Comparisons

Recurrence Interval (Years)	Peak Pre-Development Flow (cfs) (Node 2P)	Peak Post-Development Flow (cfs) (Node HC)	Peak Flow Increase or (Decrease) – (cfs)
½ of 2	0.88	0.54	(0.34)
2	8.78	7.74	(1.04)
10	19.16	15.72	(3.44)
25	24.92	24.60	(0.32)

#### Table 6-3: Subbasin #3 Peak Pre- and Post-Development Flow Comparisons

Recurrence Interval (Years)	Peak Pre-Development Flow (cfs) (Node 3P)	Peak Post-Development Flow (cfs) (Node 3S1)	Peak Flow Increase or (Decrease) – (cfs)
½ of 2	0.10	0.08	(0.02)
2	0.26	0.18	(0.08)
10	0.39	0.27	(0.12)
25	0.49	0.31	(0.18)

#### Table 6-4: Subbasin #4 Peak Pre- and Post-Development Flow Comparisons

Recurrence Interval (Years)	Peak Pre-Development Flow (cfs) (Node 4P)	Peak Post-Development Flow (cfs) (Node 4S1)	Peak Flow Increase or (Decrease) – (cfs)
½ of 2	0.03	0.02	(0.01)
2	0.06	0.06	0.00
10	0.09	0.08	(0.01)
25	0.11	0.10	(0.01)

#### Table 6-5: Subbasin #5 Peak Pre- and Post-Development Flow Comparisons

Recurrence Interval (Years)	Peak Pre-Development Flow (cfs) (Node 5P)	Peak Post-Development Flow (cfs) (Node 5S1)	Peak Flow Increase or (Decrease) – (cfs)
½ of 2	0.05	0.01	(0.04)
2	0.11	0.02	(0.09)
10	0.16	0.04	(0.12)
25	0.18	0.05	(0.13)



Recurrence Interval (Years)	Peak Pre-Development Flow (cfs) (Node 6P)	Peak Post-Development Flow (cfs) (N/A)	Peak Flow Increase or (Decrease) – (cfs)
½ of 2	0.02	0.00	(0.02)
2	0.04	0.00	(0.04)
10	0.05	0.00	(0.05)
25	0.06	0.00	(0.06)

Table 6-6: Subbasin #6 Peak Pre- and Post-Development Flow Comparisons

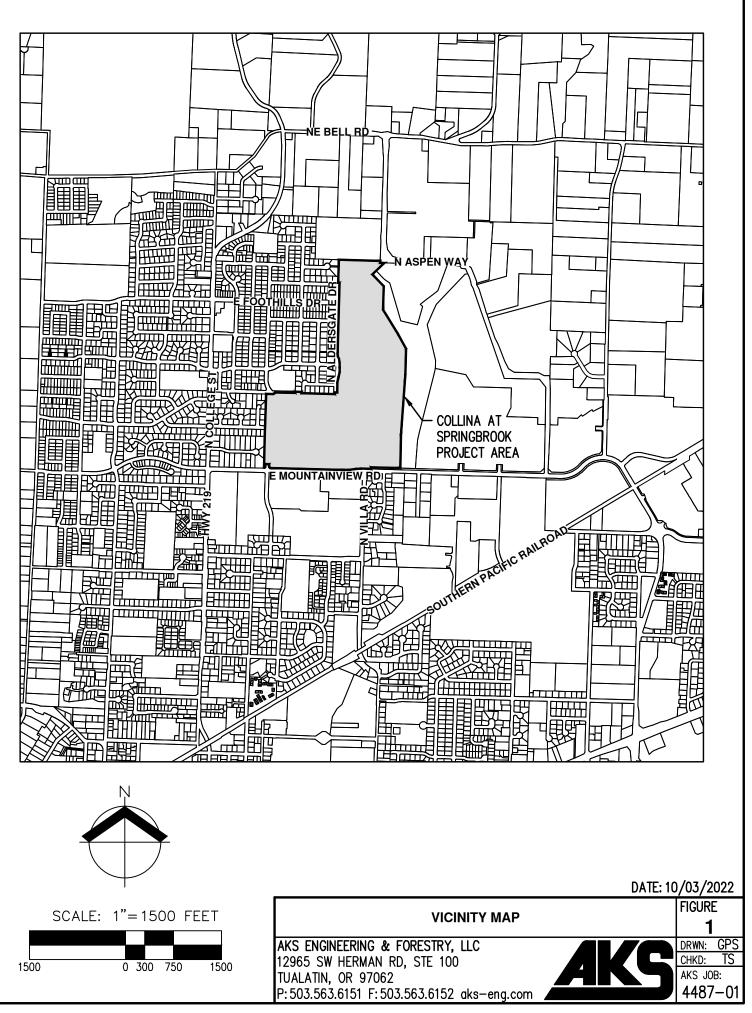
#### 6.3. Downstream Analysis

A visual investigation was performed of the downstream system for each subbasin outfall. The visual investigation did not identify any downstream impacts to the conveyance system.



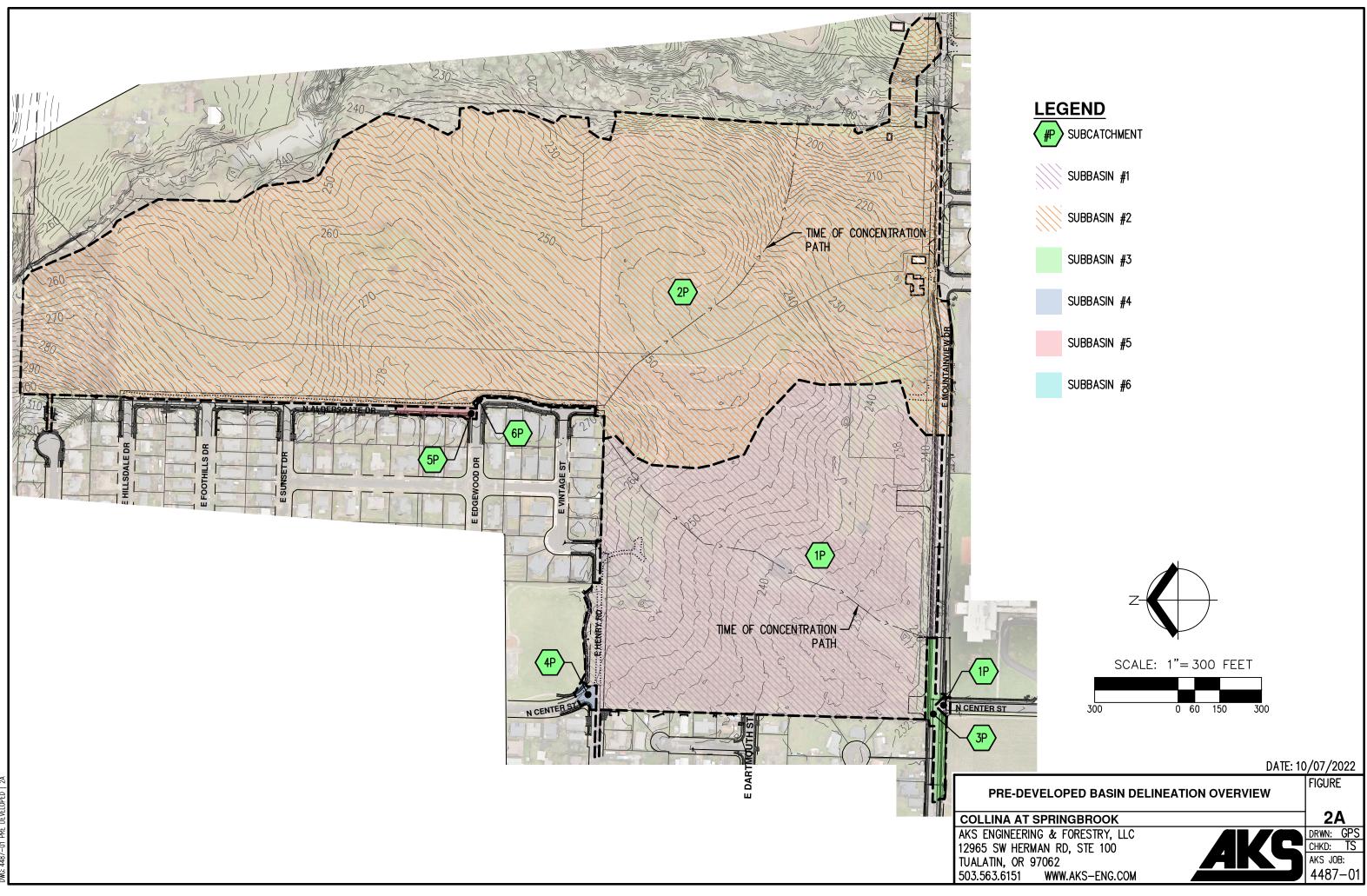


# Figure 1: VICINITY MAP

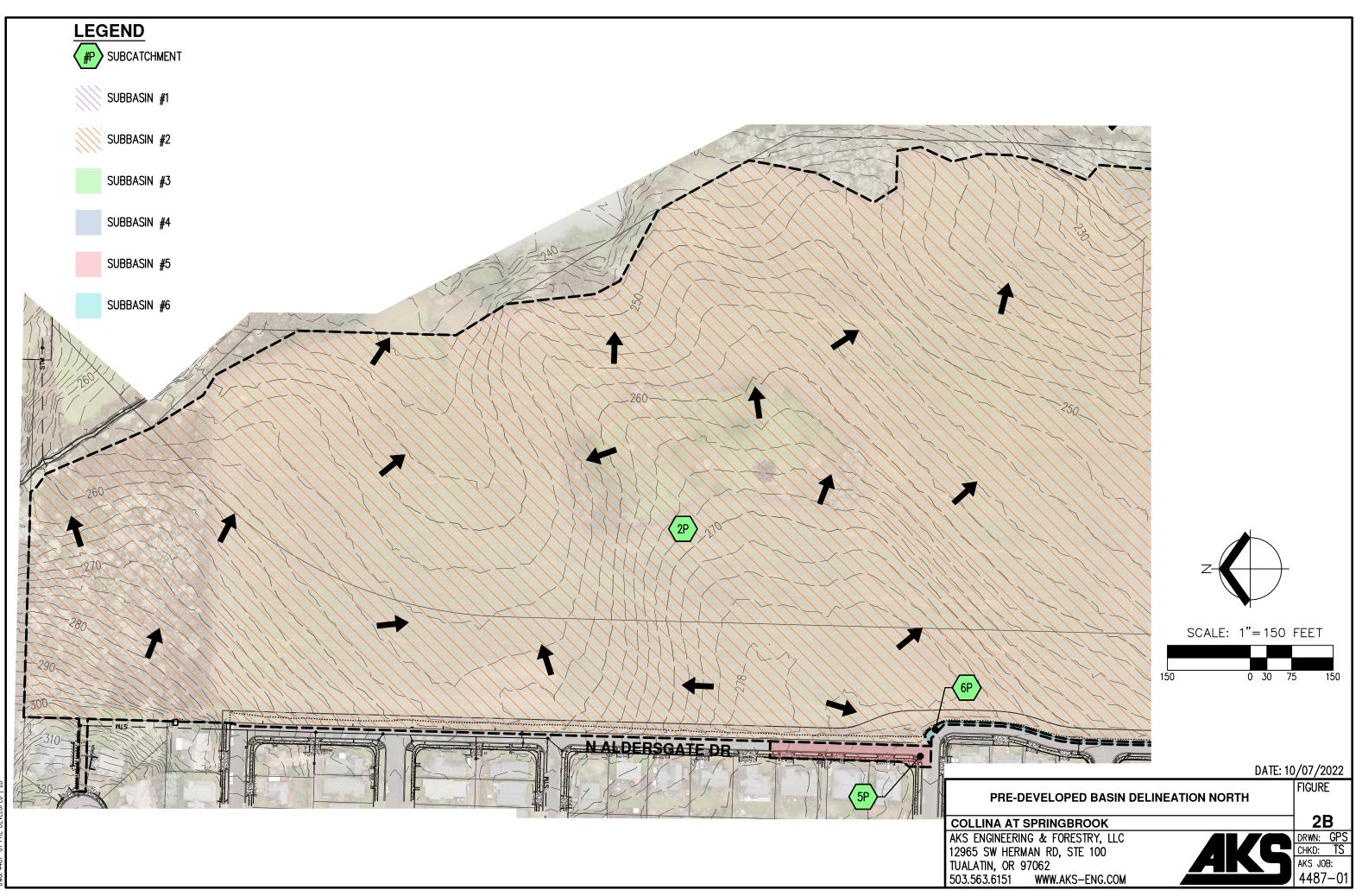


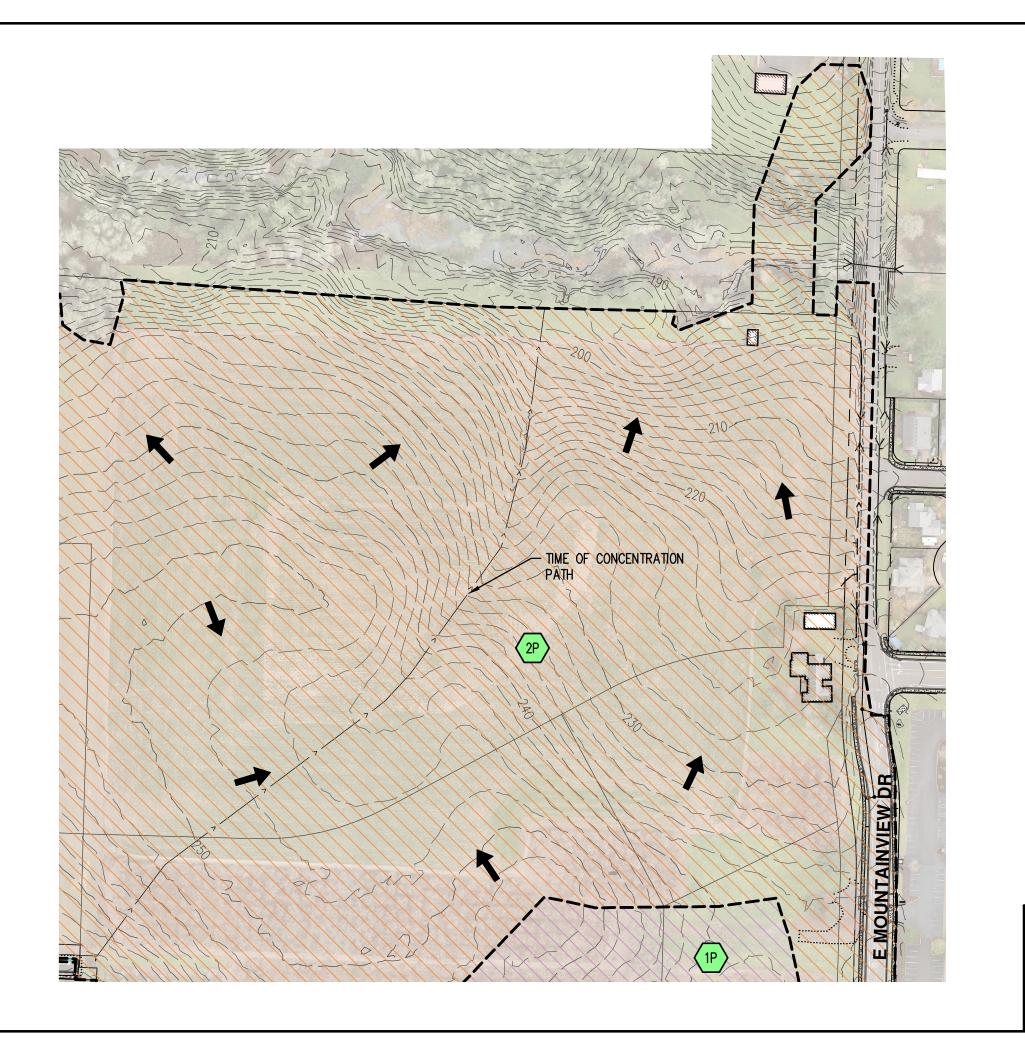


# Figure 2A-2D: PRE-DEVELOPED BASIN DELINEATION



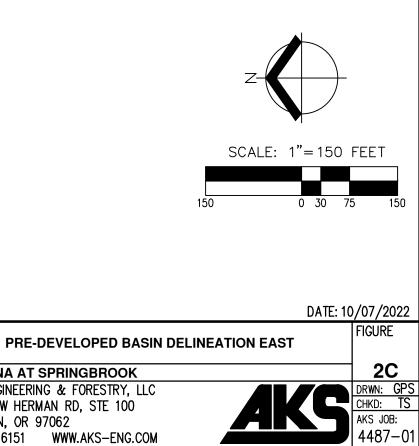




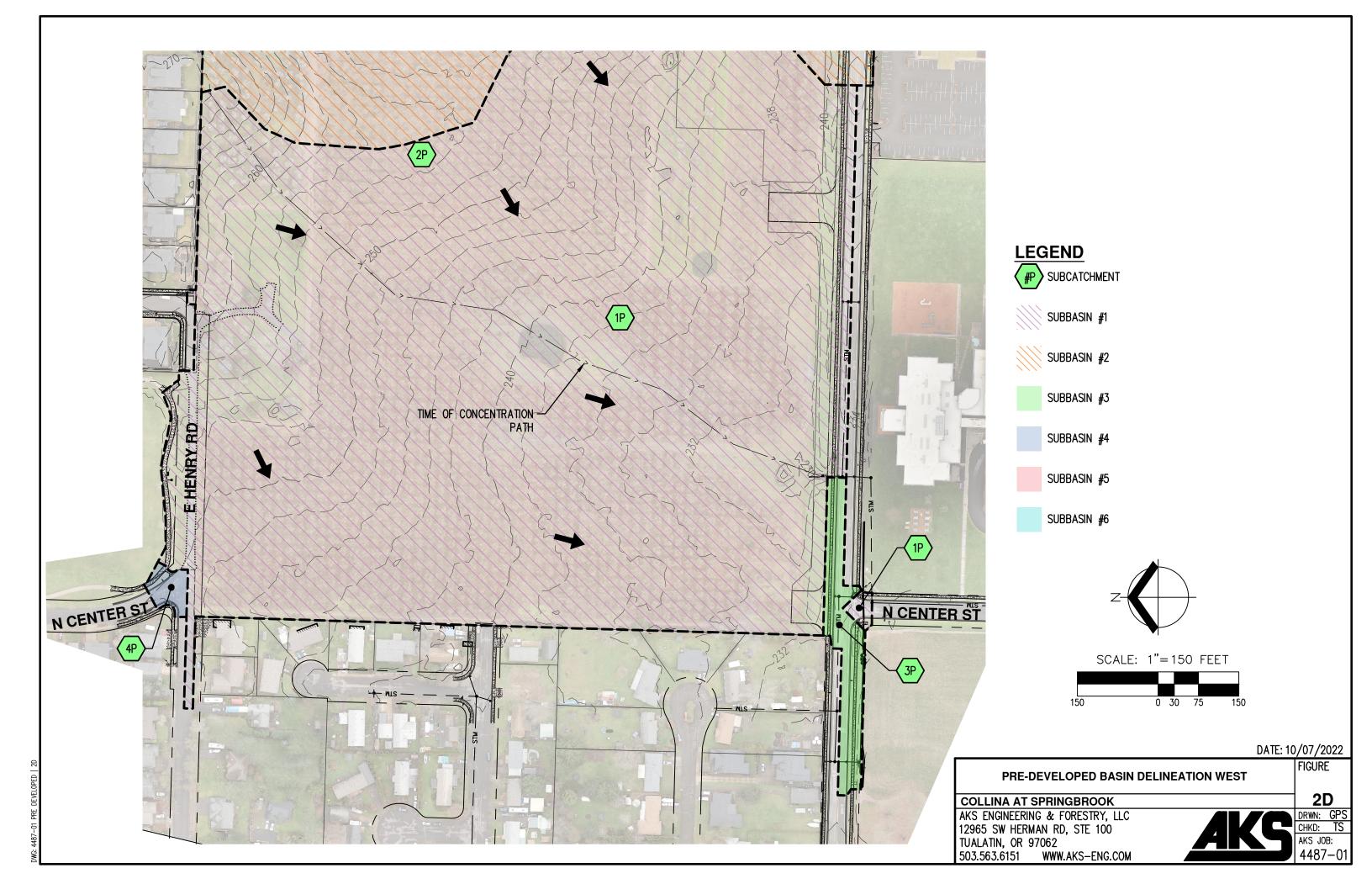


#### **LEGEND**

- #P SUBCATCHMENT
  - SUBBASIN #1
  - SUBBASIN #2
  - SUBBASIN #3
  - SUBBASIN #4
  - SUBBASIN #5
  - SUBBASIN #6

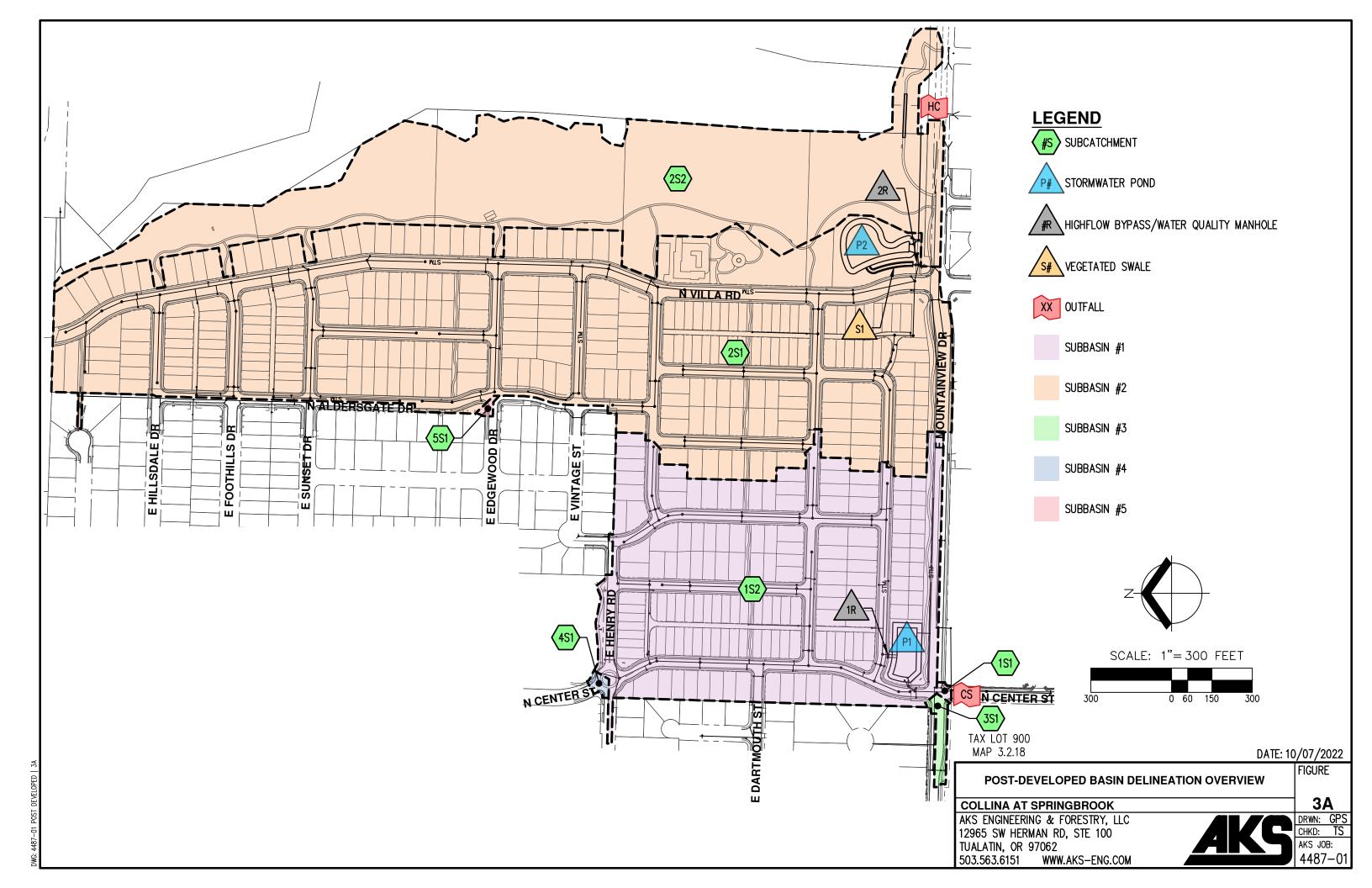


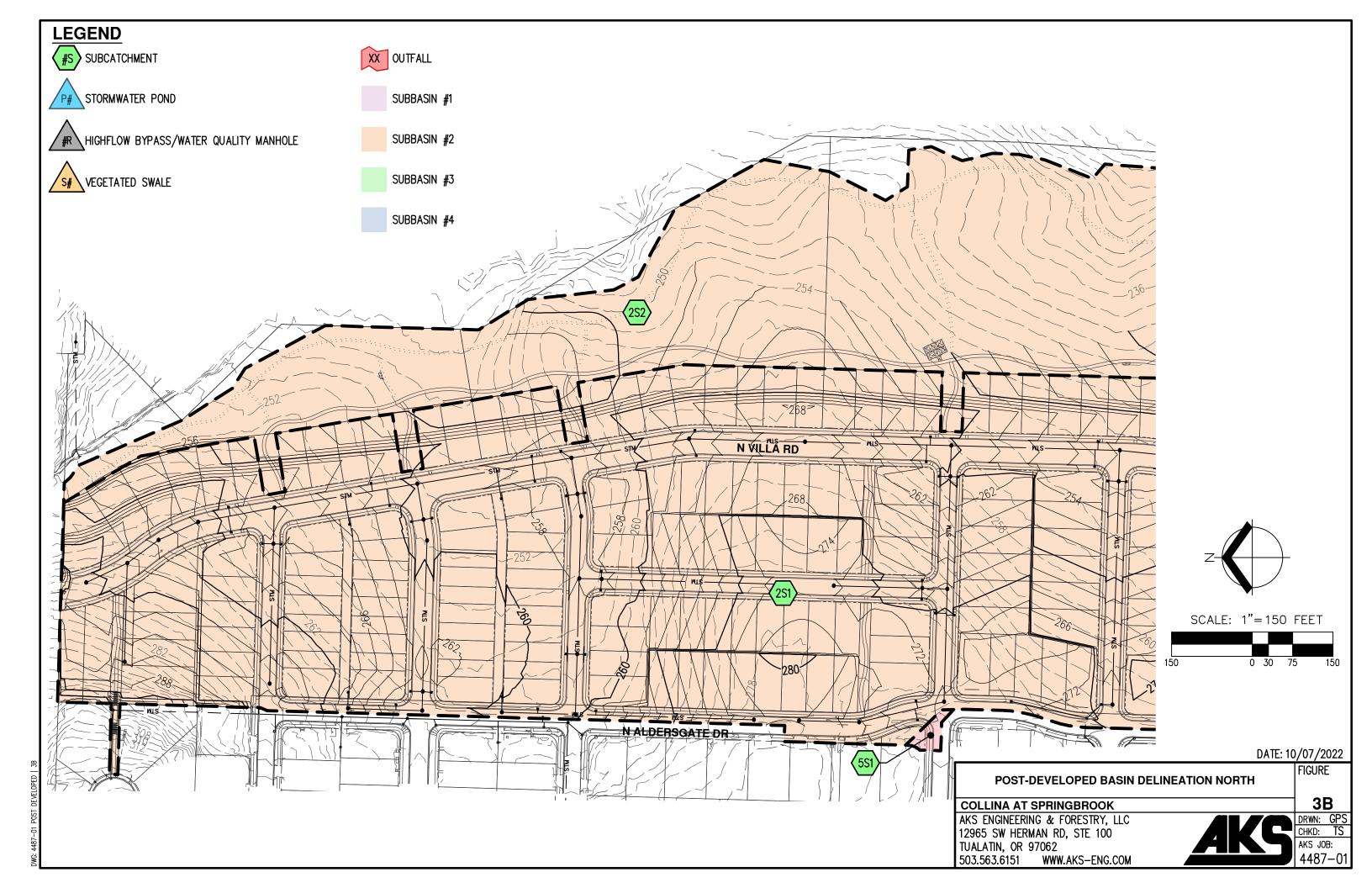
# COLLINA AT SPRINGBROOK AKS ENGINEERING & FORESTRY, LLC 12965 SW HERMAN RD, STE 100 TUALATIN, OR 97062 503.563.6151 WWW.AKS-ENG.COM

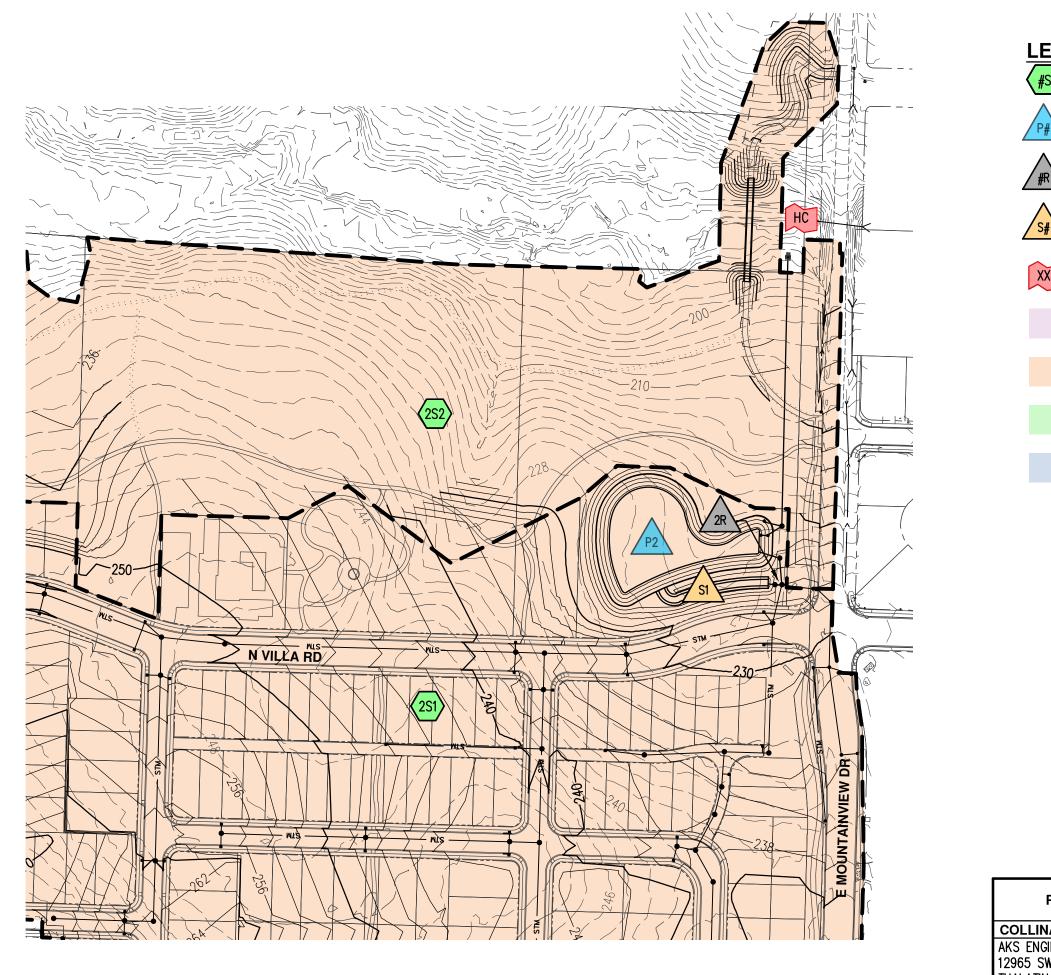




# Figure 3A-3D: POST-DEVELOPED BASIN DELINEATION







### LEGEND

#S SUBCATCHMENT

P# STORMWATER POND

#R HIGHFLOW BYPASS/WATER QUALITY MANHOLE

S# VEGETATED SWALE

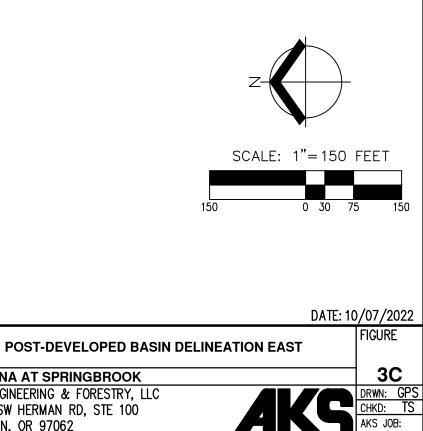
XX OUTFALL

SUBBASIN #1

SUBBASIN #2

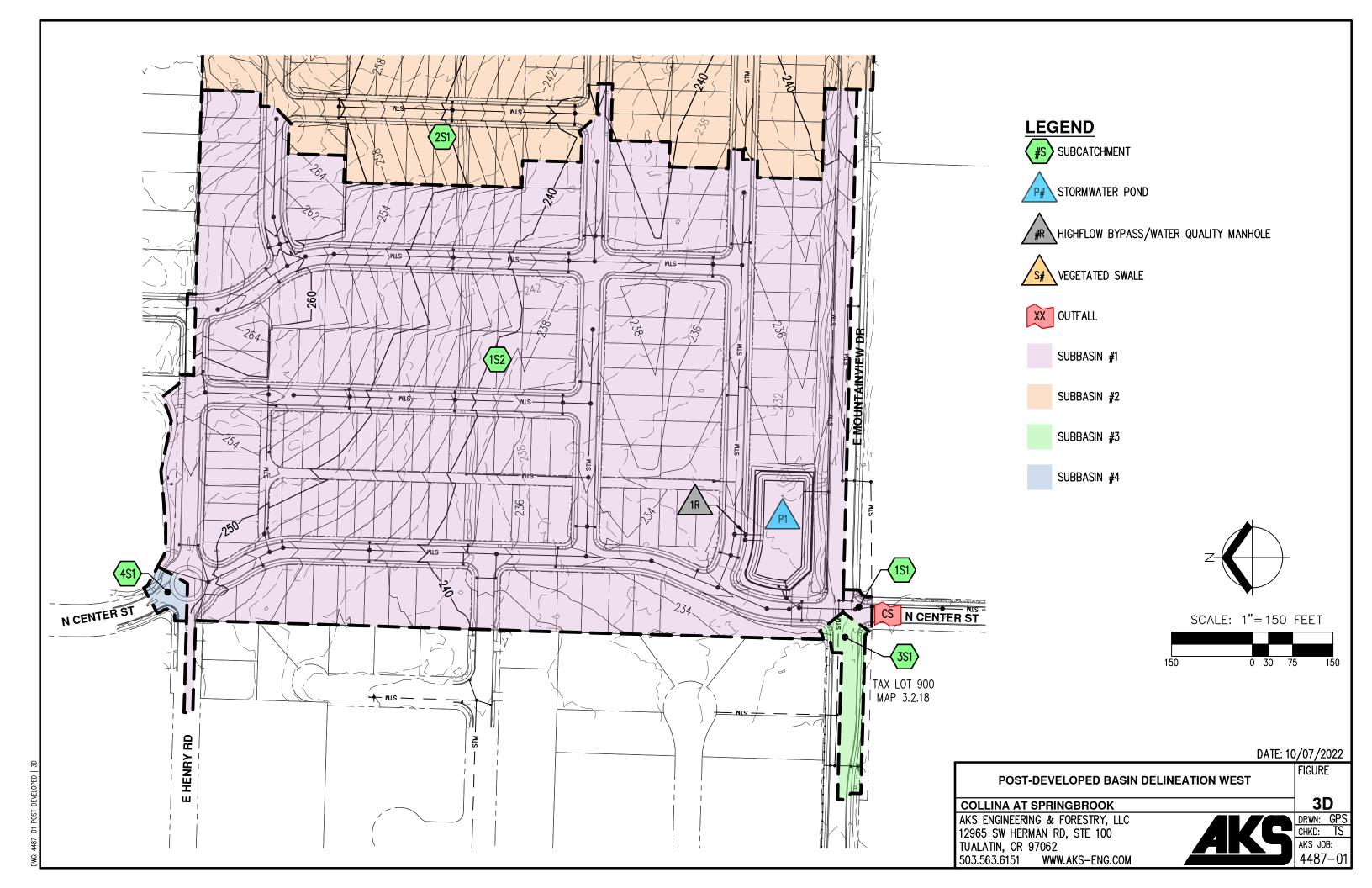
SUBBASIN #3

SUBBASIN #4



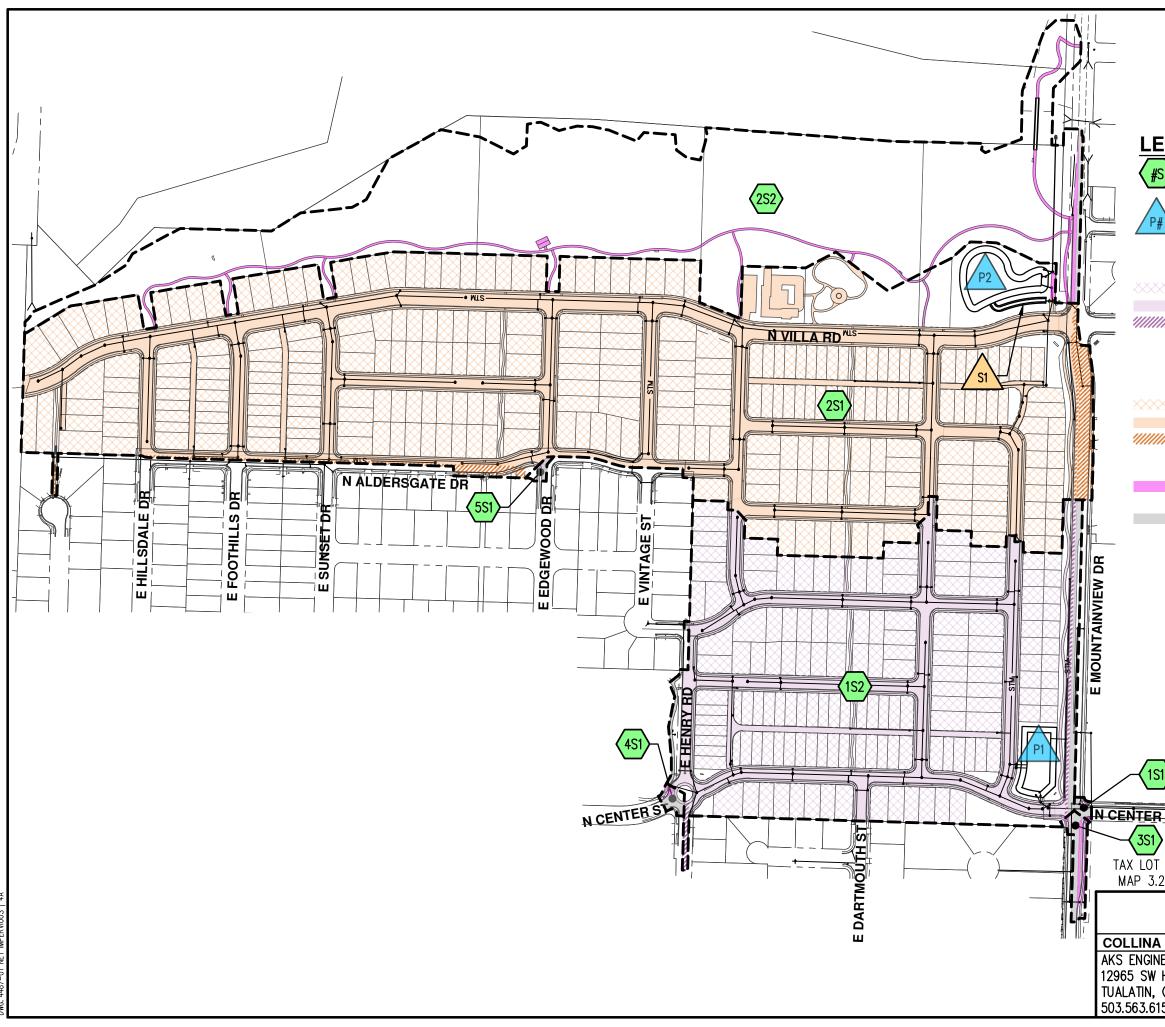
4487-01

COLLINA AT SPRINGBROOK AKS ENGINEERING & FORESTRY, LLC 12965 SW HERMAN RD, STE 100 TUALATIN, OR 97062 503.563.6151 WWW.AKS-ENG.COM

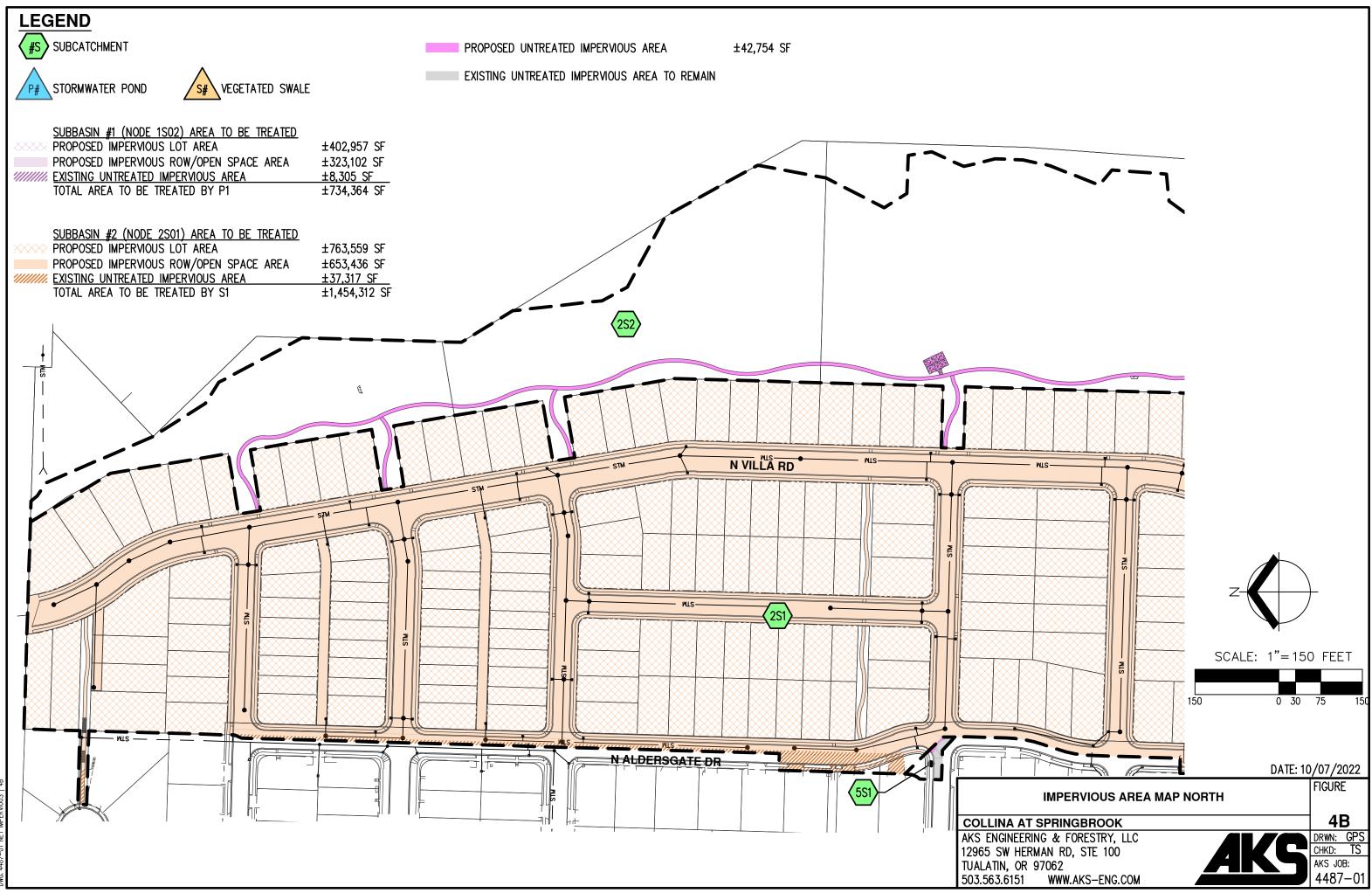




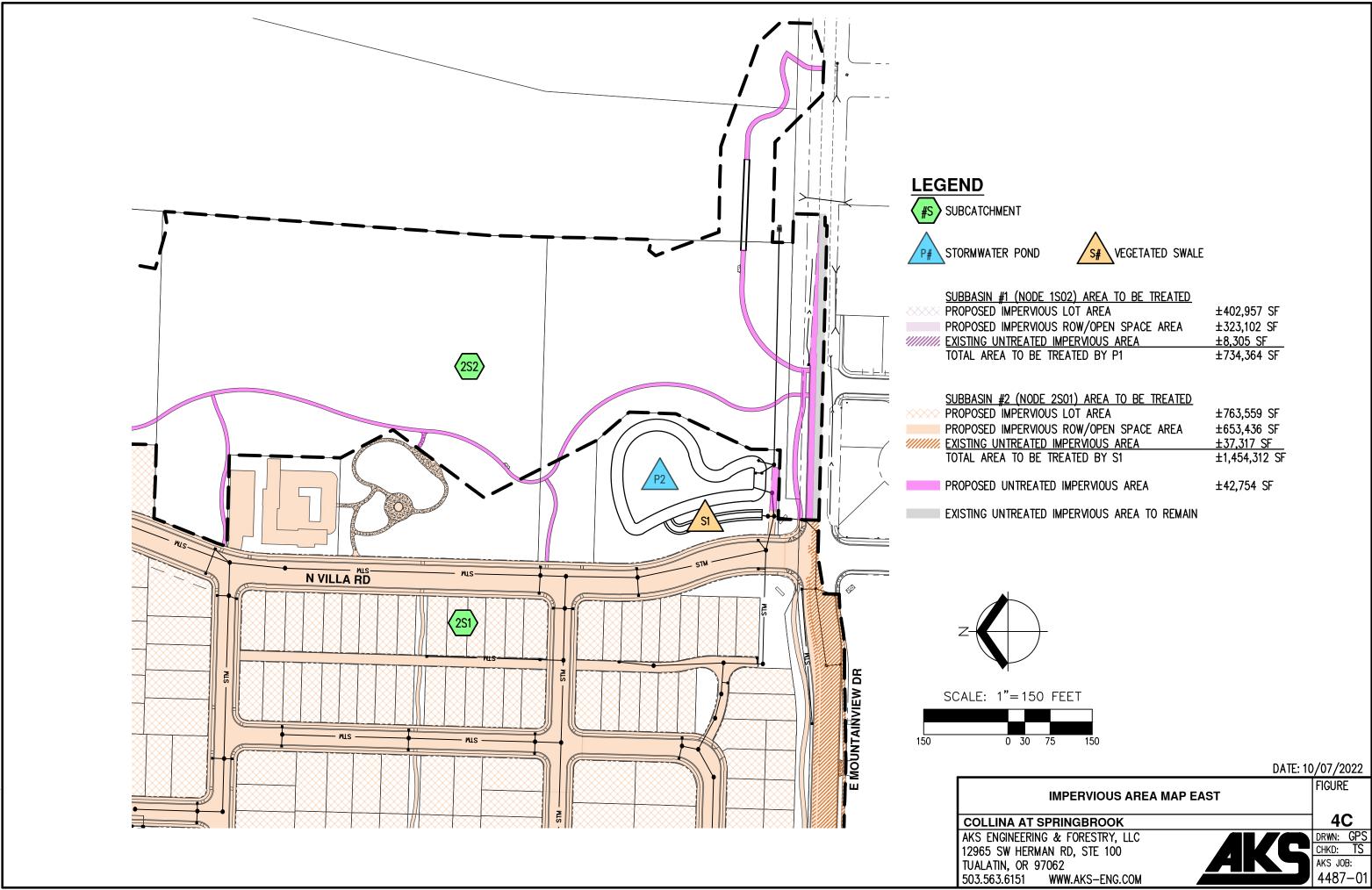
# Figure 4A-4D: IMPERVIOUS AREA MAPS



EGEND	
SUBCATCHMENT	
STORMWATER POND	
SUBBASIN #1 (NODE 1SO2) AREA TO BE TREATED PROPOSED IMPERVIOUS LOT AREA PROPOSED IMPERVIOUS ROW/OPEN SPACE AREA EXISTING UNTREATED IMPERVIOUS AREA TOTAL AREA TO BE TREATED BY P1	±402,957 SF ±323,102 SF ±8,305 SF ±734,364 SF
SUBBASIN #2 (NODE 2S01) AREA TO BE TREATED PROPOSED IMPERVIOUS LOT AREA PROPOSED IMPERVIOUS ROW/OPEN SPACE AREA <u>EXISTING UNTREATED IMPERVIOUS AREA</u> TOTAL AREA TO BE TREATED BY S1	±763,559 SF ±653,436 SF <u>±37,317 SF</u> ±1,454,312 SF
PROPOSED UNTREATED IMPERVIOUS AREA	±42,754 SF
z	
SCALE: 1"= 300 FEET	
T 900 5.2.18 DAT	E: 10/07/2022
IMPERVIOUS AREA MAP OVERVIEW	FIGURE
A AT SPRINGBROOK NEERING & FORESTRY, LLC W HERMAN RD, STE 100 , OR 97062 S151 WWW.AKS-ENG.COM	<b>4A</b> DRWN: GPS CHKD: TS AKS JOB: 4487-01

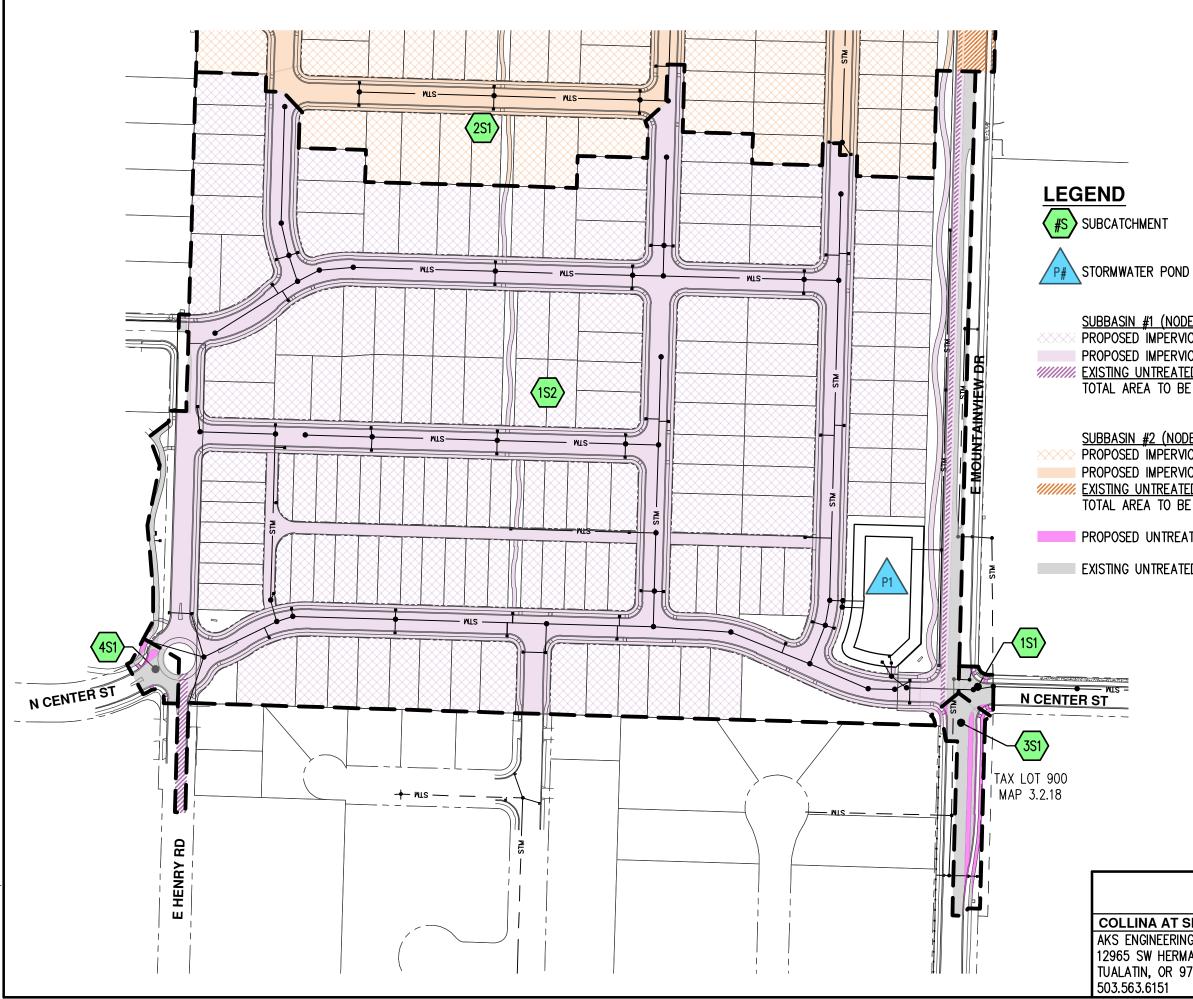


% 4487-01 NET IMPERVIOU





(NODE 1SO2) AREA TO BE TREATED	
PERVIOUS LOT AREA	±402,957 SF
PERVIOUS ROW/OPEN SPACE AREA	±323,102 SF
REATED IMPERVIOUS AREA	±8,305 SF
TO BE TREATED BY P1	±734,364 SF
(NODE 2SO1) AREA TO BE TREATED	
PERVIOUS LOT AREA	±763,559 SF
PERVIOUS ROW/OPEN SPACE AREA	±653,436 SF
REATED IMPERVIOUS AREA	±37,317 SF
TO BE TREATED BY SI	+1.454.312 SE



S# VEGETATED SWALE

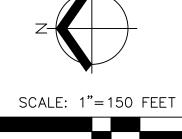
(NODE 1SO2) AREA TO BE TREATED	
IPERVIOUS LOT AREA	±402,957 SF
IPERVIOUS ROW/OPEN SPACE AREA	±323,102 SF
TREATED IMPERVIOUS AREA	<u>±8,305 SF</u>
TO BE TREATED BY P1	±734,364 SF

SUBBASIN #2 (NODE 2SO1) AREA TO BE TREATED	
PROPOSED IMPERVIOUS LOT AREA	±763,559 SF
PROPOSED IMPERVIOUS ROW/OPEN SPACE AREA	±653,436 SF
EXISTING UNTREATED IMPERVIOUS AREA	<u>±37,317 SF</u>
TOTAL AREA TO BE TREATED BY S1	±1,454,312 SF

PROPOSED UNTREATED IMPERVIOUS AREA

±42,754 SF

#### EXISTING UNTREATED IMPERVIOUS AREA TO REMAIN

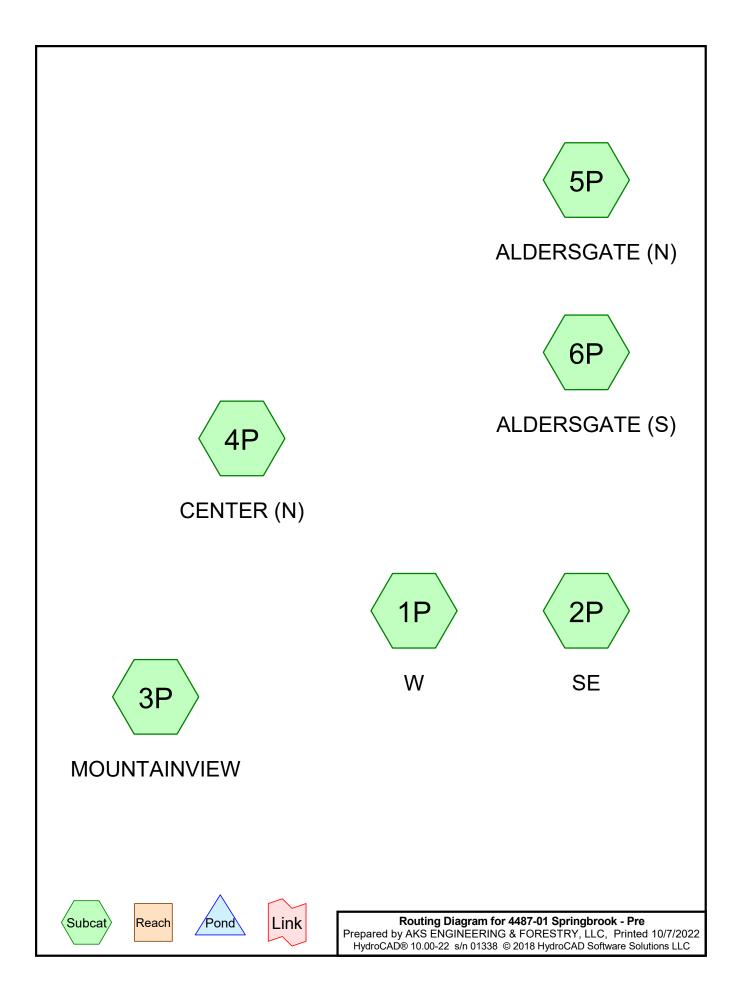




#### DATE: 10/07/2022 FIGURE **IMPERVIOUS AREA MAP WEST** 4D COLLINA AT SPRINGBROOK DRWN: GPS CHKD: TS AKS ENGINEERING & FORESTRY, LLC 12965 SW HERMAN RD, STE 100 TUALATIN, OR 97062 AKS JOB: 4487-01 503.563.6151 WWW.AKS-ENG.COM



## **Appendix A:** HYDROCAD REPORTS FOR PRE-DEVELOPED CONDITION STORM EVENTS



4487-01 Springbrook - Pre	Type IA 24-hr	1/2 2 YEAR Rain	fall=1.25"
Prepared by AKS ENGINEERING & FORESTRY, LLC		Printed	10/7/2022
HvdroCAD® 10.00-22 s/n 01338 © 2018 HvdroCAD Software So	lutions LLC		Page 2

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SBUH method, Split Pervious/Imperv. Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1P: W	Runoff Area=1,284,759 sf 2.03% Impervious Runoff Depth=0.29" Flow Length=1,389' Tc=29.8 min CN=84/98 Runoff=0.69 cfs 0.707 af
Subcatchment 2P: SE	Runoff Area=3,125,647 sf 1.30% Impervious Runoff Depth=0.18" Flow Length=2,065' Tc=27.1 min CN=80/98 Runoff=0.88 cfs 1.102 af
Subcatchment 3P: MOUNTAINVIEW	Runoff Area=25,396 sf 60.03% Impervious Runoff Depth=0.73" Tc=5.0 min CN=84/98 Runoff=0.10 cfs 0.035 af
Subcatchment 4P: CENTER (N)	Runoff Area=5,401 sf 80.65% Impervious Runoff Depth=0.87" Tc=5.0 min CN=80/98 Runoff=0.03 cfs 0.009 af
Subcatchment 5P: ALDERSGATE (N	N) Runoff Area=9,103 sf 85.65% Impervious Runoff Depth=0.90" Tc=5.0 min CN=77/98 Runoff=0.05 cfs 0.016 af
Subcatchment 6P: ALDERSGATE (S	Runoff Area=0.062 ac 100.00% Impervious Runoff Depth=1.03" Tc=5.0 min CN=0/98 Runoff=0.02 cfs 0.005 af
Total Runoff Area = 10	02.227 ac Runoff Volume = 1.875 af Average Runoff Depth = 0.22"

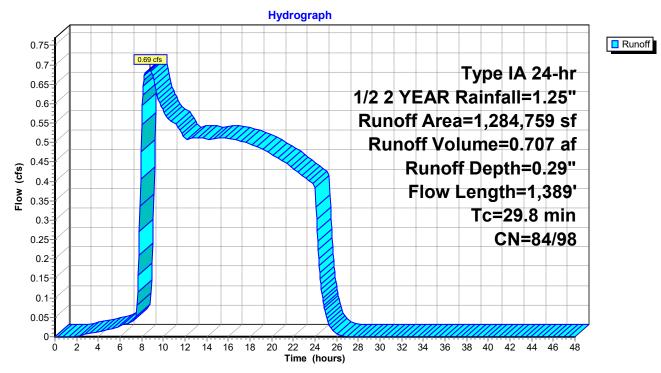
97.83% Pervious = 100.004 ac 2.17% Impervious = 2.223 ac

# Summary for Subcatchment 1P: W

Runoff = 0.69 cfs @ 8.75 hrs, Volume= 0.707 af, Depth= 0.29"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

Ale	ea (sf)	CN E	escription											
69	6,873	82 V	Woods/grass comb., Poor, HSG C											
53	6,093	86 V	Woods/grass comb., Poor, HSG D											
2	6,049		Paved roads w/curbs & sewers, HSG C											
	9,932		Gravel surface, HSG C											
	1,238			,	ood, HSG C									
	4,574	80 >	75% Gras	s cover, Go	ood, HSG D									
1,28	4,759		Veighted A											
,	8,710		-	vious Area										
2	6,049	98 2	.03% Impe	ervious Are	а									
_														
	1	01	\/_l!	0	Description									
	Length	Slope	Velocity	Capacity	Description									
(min)	(feet)	(ft/ft)	(ft/sec)	Capacity (cfs)										
	0			•	Sheet Flow, Sheet-Orchard									
<u>(min)</u> 9.8	(feet) 50	(ft/ft) 0.0480	(ft/sec) 0.08	•	Sheet Flow, Sheet-Orchard Woods: Light underbrush n= 0.400 P2= 2.50"									
(min)	(feet)	(ft/ft)	(ft/sec)	•	Sheet Flow, Sheet-Orchard Woods: Light underbrush n= 0.400 P2= 2.50" Shallow Concentrated Flow, Shallow									
<u>(min)</u> 9.8 7.9	(feet) 50 658	(ft/ft) 0.0480 0.0392	(ft/sec) 0.08 1.39	•	Sheet Flow, Sheet-Orchard Woods: Light underbrush n= 0.400 P2= 2.50" Shallow Concentrated Flow, Shallow Short Grass Pasture Kv= 7.0 fps									
<u>(min)</u> 9.8	(feet) 50	(ft/ft) 0.0480	(ft/sec) 0.08	•	Sheet Flow, Sheet-Orchard Woods: Light underbrush n= 0.400 P2= 2.50" Shallow Concentrated Flow, Shallow Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow, Shallow									
<u>(min)</u> 9.8 7.9	(feet) 50 658	(ft/ft) 0.0480 0.0392	(ft/sec) 0.08 1.39	•	Sheet Flow, Sheet-Orchard Woods: Light underbrush n= 0.400 P2= 2.50" Shallow Concentrated Flow, Shallow Short Grass Pasture Kv= 7.0 fps									



## Subcatchment 1P: W

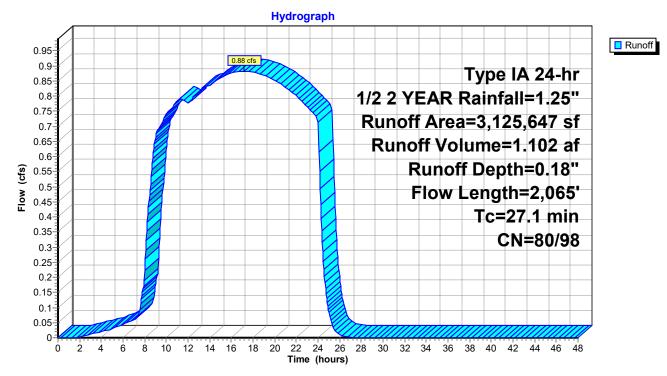
## Summary for Subcatchment 2P: SE

Runoff = 0.88 cfs @ 17.20 hrs, Volume= 1.102 af, Depth= 0.18"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

_	A	rea (sf)	CN	Description											
		38,333	82	Farmsteads, HSG C											
	2,0	22,883	79	Pasture/grassland/range, Fair, HSG C											
		95,843		Pasture/grassland/range, Fair, HSG D											
		15,113				Poor, HSG C									
		39,683				& sewers, HSG C									
		15,812		Gravel surfa											
		3,615			· ·	Fair, HSG C									
		71,757		Brush, Goo											
÷		21,606			s cover, Go	bod, HSG C									
_		1,002		Stream											
		25,647		Weighted A	0										
	,	84,962		98.70% Per											
		40,685	98	1.30% Impe	ervious Are	a									
	Тс	Length	Slope	Velocity	Capacity	Description									
	(min)	(feet)	(ft/ft)		(cfs)	Decemption									
_	1.3	16	0.1182	, ,		Sheet Flow,									
			002			Grass: Short n= 0.150 P2= 2.50"									
	3.1	268	0.0208	1.44		Shallow Concentrated Flow,									
						Nearly Bare & Untilled Kv= 10.0 fps									
	7.4	493	0.0124	1.11		Shallow Concentrated Flow,									
						Nearly Bare & Untilled Kv= 10.0 fps									
	1.5	178	0.0822	2.01		Shallow Concentrated Flow,									
						Short Grass Pasture Kv= 7.0 fps									
	8.2	486	0.0200	0.99		Shallow Concentrated Flow,									
						Short Grass Pasture Kv= 7.0 fps									
	5.6	624	0.0709	1.86		Shallow Concentrated Flow,									
_						Short Grass Pasture Kv= 7.0 fps									
	27.1	2 065	Total												

27.1 2,065 Total



## Subcatchment 2P: SE

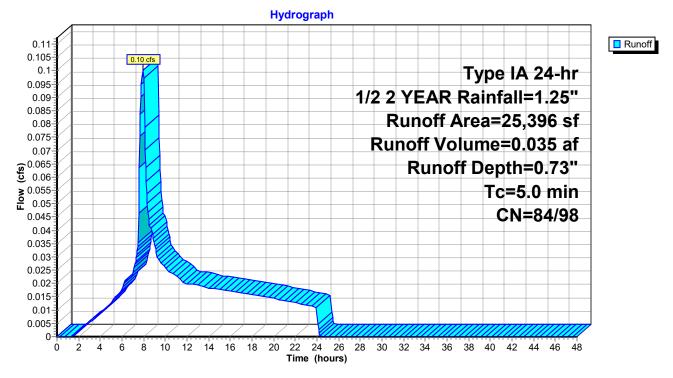
#### Summary for Subcatchment 3P: MOUNTAINVIEW

Runoff = 0.10 cfs @ 7.95 hrs, Volume= 0.035 af, Depth= 0.73"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

Area (s	f) CN	Description
6,44	7 86	Woods/grass comb., Poor, HSG D
15,24	6 98	Paved roads w/curbs & sewers, HSG C
3,70	)3 80	>75% Grass cover, Good, HSG D
25,39	96 92	Weighted Average
10,15	50 84	39.97% Pervious Area
15,24	6 98	60.03% Impervious Area
Tc Leną (min) (fe		
5.0		Direct Entry,

#### Subcatchment 3P: MOUNTAINVIEW



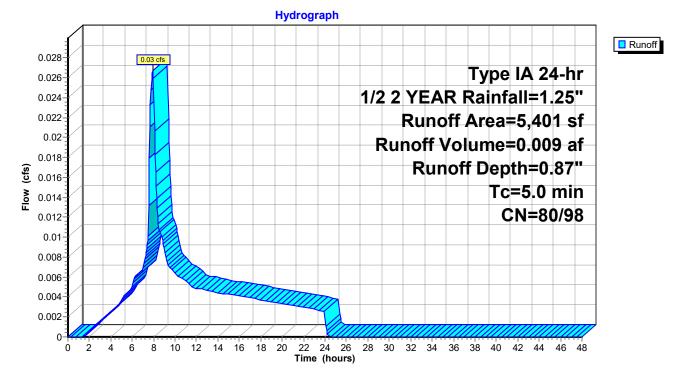
## Summary for Subcatchment 4P: CENTER (N)

Runoff = 0.03 cfs @ 7.91 hrs, Volume= 0.009 af, Depth= 0.87"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

A	rea (sf)	CN	Description		
	4,356	98	Paved road	s w/curbs &	& sewers, HSG C
	1,045	80	>75% Gras	s cover, Go	ood, HSG D
	5,401	95	Weighted A	verage	
	1,045	80	19.35% Per	vious Area	3
	4,356	98	80.65% Imp	pervious Are	rea
Tc	Length	Slope	,	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft	) (ft/sec)	(cfs)	
5.0					Direct Entry,

## Subcatchment 4P: CENTER (N)



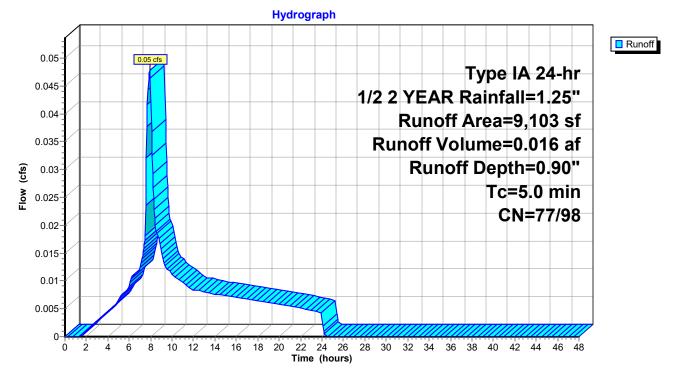
# Summary for Subcatchment 5P: ALDERSGATE (N)

Runoff = 0.05 cfs @ 7.91 hrs, Volume= 0.016 af, Depth= 0.90"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

A	rea (sf)	CN	Description
	7,797	98	Paved roads w/curbs & sewers, HSG C
	653	80	>75% Grass cover, Good, HSG D
	653	74	>75% Grass cover, Good, HSG C
	9,103	95	Weighted Average
	1,306	77	14.35% Pervious Area
	7,797	98	85.65% Impervious Area
Tc (min)	Length (feet)	Slop (ft/1	
5.0			Direct Entry,

# Subcatchment 5P: ALDERSGATE (N)



# Summary for Subcatchment 6P: ALDERSGATE (S)

Runoff 0.02 cfs @ 7.91 hrs, Volume= 0.005 af, Depth= 1.03" =

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

Area 0.	062	<u>CN</u> 98		escr avec	-		v/cı	urbs	s & s	sew	ers,	HS	SG	С										
0.	062	98	5 1(	00.0	0%	Impe	ervi	ous	Are	a														
Tc (min)	Leng (fee		Slop (ft/i			ocity sec)	C	-	acity cfs)		)esc	rip	tior	ו										
5.0										D	)ire	ct E	Int	ry,										
					ę	Sub	cat	chi	mei	nt 6	SP:	AL	D	ER	SG	АТ	Έ(	S)						
									Hydr								•							_
0.018	3																							Runo
0.017			0.02	cfs														Т	ne	IA	2	<b>4</b> _ł	١r	
0.016 0.015													1/2	2	YF	FΔ	R I		1	all	1	1		
0.014			]															-		=0.0		-		
0.013 0.012	-												F							=0.				
0.01 <sup>7</sup>																				th				
0.0 (cts)	-																			=5				
₩ 0.008	=																			CN	1=1	0/9	8	
0.007																								
0.005	3																							
0.004 0.003	E A			~	Ű		$\overline{m}$																	-
0.002																								-
0.00 <sup>2</sup> (				<u></u>			~	_		,				///										ļ
	0 2	4	6 8	3 10	12	14	16	18		22 2 'ime			28	30	32	34	36	38	40	42	44	46	48	

4487-01 Springbrook - Pre	Type IA 24-hr 2 YEAR Rainfall=2.50"
Prepared by AKS ENGINEERING & FORESTRY, LLC	Printed 10/7/2022
HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solution	ns LLC Page 11

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SBUH method, Split Pervious/Imperv. Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1P: W	Runoff Area=1,284,759 sf 2.03% Impervious Runoff Depth=1.14" Flow Length=1,389' Tc=29.8 min CN=84/98 Runoff=5.09 cfs 2.800 af
Subcatchment 2P: SE	Runoff Area=3,125,647 sf 1.30% Impervious Runoff Depth=0.91" Flow Length=2,065' Tc=27.1 min CN=80/98 Runoff=8.78 cfs 5.423 af
Subcatchment 3P: MOUNTAINVIEW	Runoff Area=25,396 sf 60.03% Impervious Runoff Depth=1.81" Tc=5.0 min CN=84/98 Runoff=0.26 cfs 0.088 af
Subcatchment 4P: CENTER (N)	Runoff Area=5,401 sf 80.65% Impervious Runoff Depth=2.00" Tc=5.0 min CN=80/98 Runoff=0.06 cfs 0.021 af
Subcatchment 5P: ALDERSGATE (N	N Runoff Area=9,103 sf 85.65% Impervious Runoff Depth=2.05" Tc=5.0 min CN=77/98 Runoff=0.11 cfs 0.036 af
Subcatchment 6P: ALDERSGATE (S	Runoff Area=0.062 ac 100.00% Impervious Runoff Depth=2.27" Tc=5.0 min CN=0/98 Runoff=0.04 cfs 0.012 af
Total Runoff Area = 10	2.227 ac Runoff Volume = 8.379 af Average Runoff Depth = 0.98"

97.83% Pervious = 100.004 ac 2.17% Impervious = 2.223 ac

# Summary for Subcatchment 1P: W

Runoff = 5.09 cfs @ 8.07 hrs, Volume= 2.800 af, Depth= 1.14"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

Ar	ea (sf)	CN E	Description											
6	96,873	82 V	Woods/grass comb., Poor, HSG C											
5	36,093	86 V	Woods/grass comb., Poor, HSG D											
:	26,049	98 F	Paved roads w/curbs & sewers, HSG C											
	9,932		Gravel surface, HSG C											
	11,238			,	ood, HSG C									
	4,574	80 >	75% Gras	s cover, Go	ood, HSG D									
,	84,759		Veighted A	•										
,	58,710		-	vious Area										
	26,049	98 2	03% Impe	ervious Area	а									
Тс	Length	Slope	Velocity	Capacity	Description									
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description									
9.8	50	0.0480	0.08	(013)	Shoot Flow, Shoot Orchard									
9.0	50	0.0400	0.00		Sheet Flow, Sheet-Orchard Woods: Light underbrush n= 0.400 P2= 2.50"									
7.9	658	0.0392	1.39		Shallow Concentrated Flow, Shallow									
1.5	000	0.0032	1.00		Short Grass Pasture Kv= 7.0 fps									
12.1	681	0.0181	0.94		Shallow Concentrated Flow, Shallow									
12.1	001	0.0101	0.01		Short Grass Pasture Kv= 7.0 fps									
29.8														

Hydrograph Runoff 5.09 cfs Type IA 24-hr 5-2 YEAR Rainfall=2.50" Runoff Area=1,284,759 sf 4-Runoff Volume=2.800 af Runoff Depth=1.14" Flow (cfs) 3-Flow Length=1,389' Tc=29.8 min 2-CN=84/98 1. 0-2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ó Time (hours)

## Subcatchment 1P: W

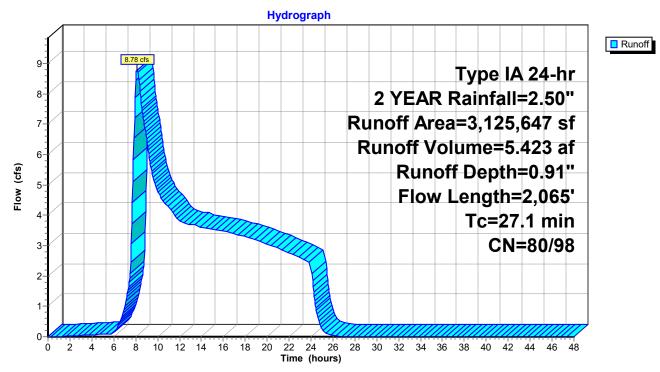
## Summary for Subcatchment 2P: SE

Runoff = 8.78 cfs @ 8.11 hrs, Volume= 5.423 af, Depth= 0.91"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description												
	38,333	82	Farmsteads	Farmsteads, HSG C											
2,0	)22,883	79	Pasture/grassland/range, Fair, HSG C												
	195,843		Pasture/grassland/range, Fair, HSG D												
3	315,113	82	Woods/gras	Noods/grass comb., Poor, HSG C											
	39,683	98	Paved road	s w/curbs &	& sewers, HSG C										
	15,812	96	Gravel surfa	ace, HSG (	C										
	3,615	79	50-75% Gra	ass cover, l	Fair, HSG C										
1	171,757		Brush, Goo	,											
	21,606			s cover, Go	ood, HSG C										
*	1,002	100	Stream												
,	25,647		Weighted A	verage											
3,0	)84,962		98.70% Pei												
	40,685	98	1.30% Impe	ervious Are	a										
т.	المربية مرالي	01.000	Valasita.	0	Description										
Tc	Length	Slope		Capacity	Description										
(min)	(feet)	(ft/ft)		(cfs)											
1.3	16	0.1182	2 0.21		Sheet Flow,										
0.4	000	0.0000			Grass: Short n= 0.150 P2= 2.50"										
3.1	268	0.0208	3 1.44		Shallow Concentrated Flow,										
7 4	400	0.0404			Nearly Bare & Untilled Kv= 10.0 fps										
7.4	493	0.0124	1.11		Shallow Concentrated Flow,										
1 5	170	0 0000	0 0 0 1		Nearly Bare & Untilled Kv= 10.0 fps										
1.5	178	0.0822	2 2.01		Shallow Concentrated Flow,										
8.2	486	0.0200	0.00		Short Grass Pasture Kv= 7.0 fps										
0.2	400	0.0200	0.99		Shallow Concentrated Flow,										
5.6	624	0.0709	1.86		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,										
5.0	024	0.0708	0.00		Short Grass Pasture Kv= 7.0 fps										
	2 065	Total													

27.1 2,065 Total



# Subcatchment 2P: SE

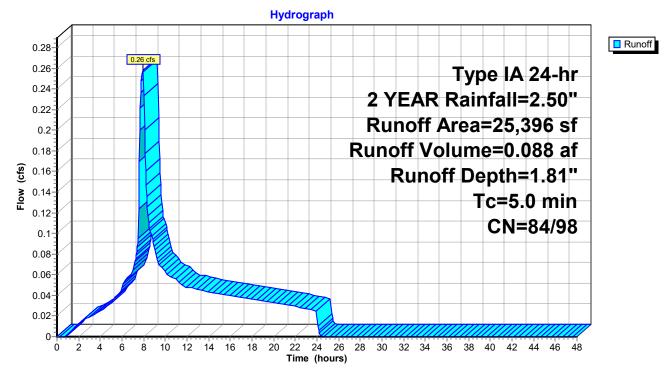
# Summary for Subcatchment 3P: MOUNTAINVIEW

Runoff = 0.26 cfs @ 7.93 hrs, Volume= 0.088 af, Depth= 1.81"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description		
	6,447	86	Woods/gras	ss comb., F	Poor, HSG D
	15,246	98	Paved road	s w/curbs &	& sewers, HSG C
	3,703	80	>75% Gras	s cover, Go	bod, HSG D
	25,396	92	Weighted A	verage	
	10,150	84	39.97% Per	vious Area	l
	15,246	98	60.03% Imp	pervious Ar	ea
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description
5.0					Direct Entry,

#### Subcatchment 3P: MOUNTAINVIEW



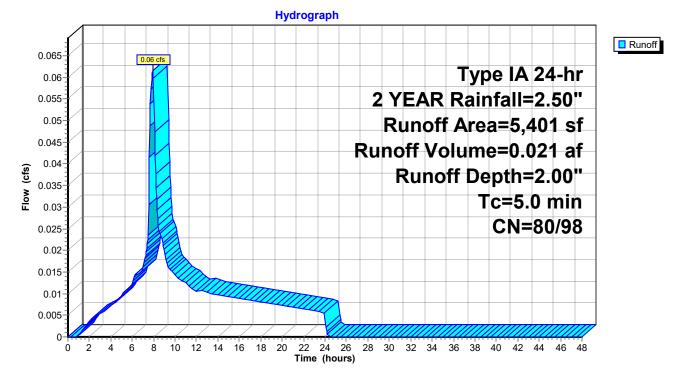
## Summary for Subcatchment 4P: CENTER (N)

Runoff = 0.06 cfs @ 7.91 hrs, Volume= 0.021 af, Depth= 2.00"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description		
	4,356	98	Paved road	s w/curbs &	& sewers, HSG C
	1,045	80	>75% Gras	s cover, Go	bod, HSG D
	5,401	95	Weighted A	verage	
	1,045	80	19.35% Per	vious Area	l
	4,356	98	80.65% Imp	pervious Ar	ea
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
5.0					Direct Entry,

## Subcatchment 4P: CENTER (N)



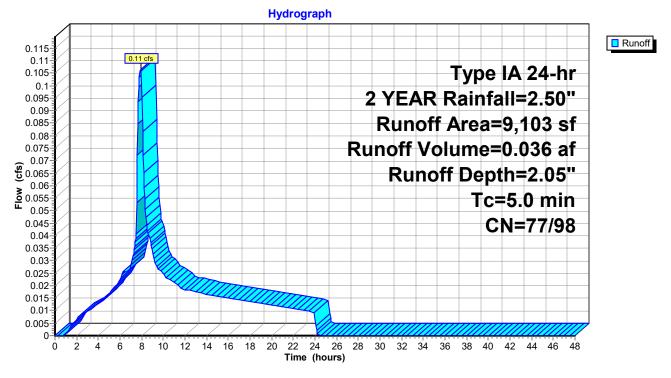
## Summary for Subcatchment 5P: ALDERSGATE (N)

Runoff = 0.11 cfs @ 7.91 hrs, Volume= 0.036 af, Depth= 2.05"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description		
	7,797	98	Paved road	s w/curbs &	& sewers, HSG C
	653	80	>75% Gras	s cover, Go	bod, HSG D
	653	74	>75% Gras	s cover, Go	bod, HSG C
	9,103	95	Weighted A	verage	
	1,306	77	14.35% Per	vious Area	l
	7,797	98	85.65% Imp	ervious Ar	ea
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description
5.0					Direct Entry,

# Subcatchment 5P: ALDERSGATE (N)



# Summary for Subcatchment 6P: ALDERSGATE (S)

Runoff = 0.04 cfs @ 7.90 hrs, Volume= 0.012 af, Depth= 2.27"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

Area 0.	<u>(ac)</u> .062	<u>CN</u> 98			ripti d ro		w/c	urb	s &	sev	/ers	. H	SG	С										
	.062	98	-		00%							,		-										
Tc (min)	Leng (fee		Slo (ft	pe /ft)		ocity /sec		Cap	acit (cfs	•	Des	crip	otio	า										
5.0										I	Dire	ect	Ent	ry,										
					ļ	Sub	ca	tch	me	ent	6P:	A	LD	ER	SG	АТ	Έ(	S)						
									Нус	drogi	aph													_
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0.024	- /												۱u	-					-	-				
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0.012	= /			X																				
0.0	= /																							
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4487-01 Springbrook - Pre	Type IA 24-hr	10 YEAR Rair	nfall=3.50"
Prepared by AKS ENGINEERING & FORESTRY, LLC		Printed	10/7/2022
HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solution	ons LLC		Page 20

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SBUH method, Split Pervious/Imperv. Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1P: W	Runoff Area=1,284,759 sf 2.03% Impervious Runoff Depth=1.96" Flow Length=1,389' Tc=29.8 min CN=84/98 Runoff=9.75 cfs 4.826 af
Subcatchment 2P: SE	Runoff Area=3,125,647 sf 1.30% Impervious Runoff Depth=1.66" low Length=2,065' Tc=27.1 min CN=80/98 Runoff=19.16 cfs 9.912 af
Subcatchment 3P: MOUNTAINVIEW	Runoff Area=25,396 sf  60.03% Impervious  Runoff Depth=2.73" Tc=5.0 min  CN=84/98  Runoff=0.39 cfs  0.133 af
Subcatchment 4P: CENTER (N)	Runoff Area=5,401 sf 80.65% Impervious Runoff Depth=2.95" Tc=5.0 min CN=80/98 Runoff=0.09 cfs 0.030 af
Subcatchment 5P: ALDERSGATE (N)	Runoff Area=9,103 sf 85.65% Impervious Runoff Depth=3.00" Tc=5.0 min CN=77/98 Runoff=0.16 cfs 0.052 af
Subcatchment 6P: ALDERSGATE (S)	Runoff Area=0.062 ac 100.00% Impervious Runoff Depth=3.27" Tc=5.0 min CN=0/98 Runoff=0.05 cfs 0.017 af
Total Runoff Area = 102.2	227 ac Runoff Volume = 14.970 af Average Runoff Depth = 1.76"

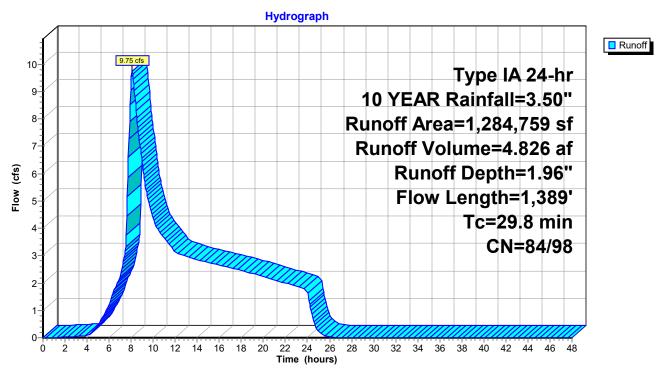
97.83% Pervious = 100.004 ac 2.17% Impervious = 2.223 ac

# Summary for Subcatchment 1P: W

Runoff = 9.75 cfs @ 8.06 hrs, Volume= 4.826 af, Depth= 1.96"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

Ar	ea (sf)	CN E	Description		
6	96,873	82 V	Voods/gras	ss comb., F	Poor, HSG C
5	36,093	86 V	Voods/gras	ss comb., F	Poor, HSG D
:	26,049	98 F	aved road	s w/curbs &	& sewers, HSG C
	9,932			ace, HSG C	
	11,238			,	ood, HSG C
	4,574	80 >	75% Gras	s cover, Go	ood, HSG D
,	84,759		Veighted A	•	
,	58,710		-	vious Area	
	26,049	98 2	03% Impe	ervious Area	а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description
9.8	50	0.0480	0.08	(013)	Shoot Flow, Shoot Orchard
9.0	50	0.0400	0.00		Sheet Flow, Sheet-Orchard Woods: Light underbrush n= 0.400 P2= 2.50"
7.9	658	0.0392	1.39		Shallow Concentrated Flow, Shallow
1.5	000	0.0032	1.00		Short Grass Pasture Kv= 7.0 fps
12.1	681	0.0181	0.94		Shallow Concentrated Flow, Shallow
12.1	001	0.0101	0.01		Short Grass Pasture Kv= 7.0 fps
29.8					



#### Subcatchment 1P: W

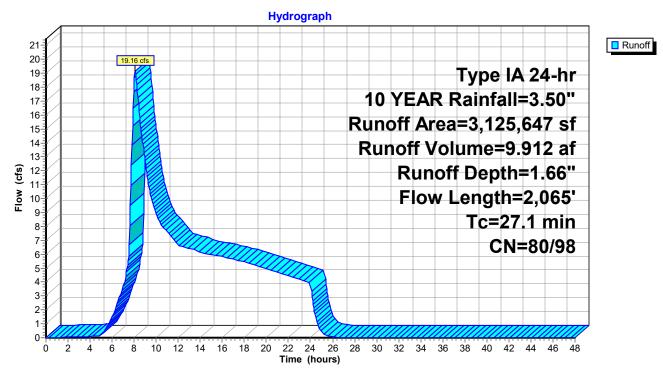
## Summary for Subcatchment 2P: SE

Runoff = 19.16 cfs @ 8.07 hrs, Volume= 9.912 af, Depth= 1.66"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

A	rea (sf)	CN	Description		
	38,333	82	Farmsteads	s, HSG C	
2,0	)22,883	79	Pasture/gra	ssland/ran	ge, Fair, HSG C
	195,843				ge, Fair, HSG D
3	315,113	82	Woods/gras	ss comb., F	Poor, HSG C
	39,683	98	Paved road	s w/curbs &	& sewers, HSG C
	15,812	96	Gravel surfa	ace, HSG (	C
	3,615	79	50-75% Gra	ass cover, l	Fair, HSG C
1	171,757		Brush, Goo	,	
	21,606			s cover, Go	ood, HSG C
*	1,002	100	Stream		
,	25,647		Weighted A	verage	
3,0	)84,962		98.70% Pei		
	40,685	98	1.30% Impe	ervious Are	a
т.	المربية مرالي	01.000	Valasita.	0	Description
Tc	Length	Slope		Capacity	Description
(min)	(feet)	(ft/ft)		(cfs)	
1.3	16	0.1182	2 0.21		Sheet Flow,
0.4	000	0.0000			Grass: Short n= 0.150 P2= 2.50"
3.1	268	0.0208	3 1.44		Shallow Concentrated Flow,
7 4	400	0.0404			Nearly Bare & Untilled Kv= 10.0 fps
7.4	493	0.0124	1.11		Shallow Concentrated Flow,
1 5	170	0 0000	0 0 0 1		Nearly Bare & Untilled Kv= 10.0 fps
1.5	178	0.0822	2 2.01		Shallow Concentrated Flow,
8.2	486	0.0200	0.00		Short Grass Pasture Kv= 7.0 fps
0.2	400	0.0200	0.99		Shallow Concentrated Flow,
5.6	624	0.0709	1.86		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,
5.0	024	0.0708	0.00		Short Grass Pasture Kv= 7.0 fps
	2 065	Total			

27.1 2,065 Total



## Subcatchment 2P: SE

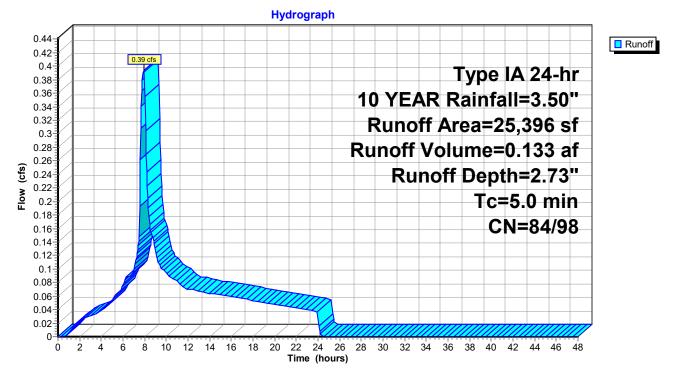
# Summary for Subcatchment 3P: MOUNTAINVIEW

Runoff = 0.39 cfs @ 7.92 hrs, Volume= 0.133 af, Depth= 2.73"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

Area (s	f) CN	Description
6,44	7 86	Woods/grass comb., Poor, HSG D
15,24	6 98	Paved roads w/curbs & sewers, HSG C
3,70	3 80	>75% Grass cover, Good, HSG D
25,39	6 92	Weighted Average
10,15	0 84	39.97% Pervious Area
15,24	6 98	60.03% Impervious Area
Tc Leng (min) (fe	, i	
5.0		Direct Entry,

#### Subcatchment 3P: MOUNTAINVIEW



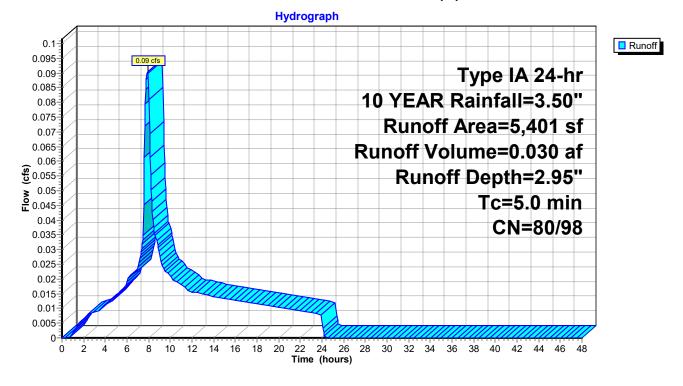
## Summary for Subcatchment 4P: CENTER (N)

Runoff = 0.09 cfs @ 7.91 hrs, Volume= 0.030 af, Depth= 2.95"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

Α	rea (sf)	CN	Description		
	4,356	98	Paved road	s w/curbs &	& sewers, HSG C
	1,045	80	>75% Gras	s cover, Go	bod, HSG D
	5,401	95	Weighted A	verage	
	1,045	80	19.35% Per	vious Area	
	4,356	98	80.65% Imp	pervious Are	ea
т.	1	01	• \/_l;	0	Description
Tc	Length	Slop		Capacity	Description
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	
5.0					Direct Entry,

## Subcatchment 4P: CENTER (N)



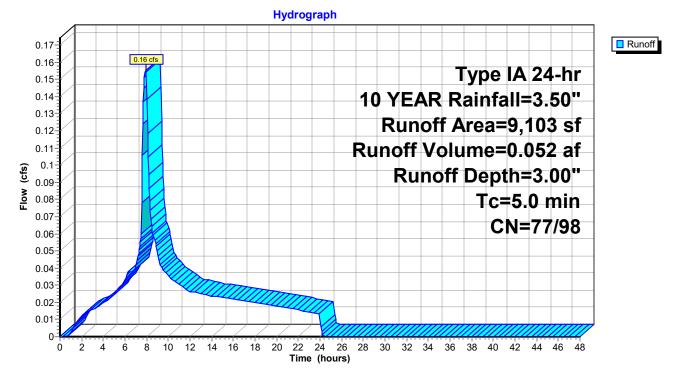
# Summary for Subcatchment 5P: ALDERSGATE (N)

Runoff = 0.16 cfs @ 7.91 hrs, Volume= 0.052 af, Depth= 3.00"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

A	rea (sf)	CN	Description
	7,797	98	Paved roads w/curbs & sewers, HSG C
	653	80	>75% Grass cover, Good, HSG D
	653	74	>75% Grass cover, Good, HSG C
	9,103	95	Weighted Average
	1,306	77	14.35% Pervious Area
	7,797	98	85.65% Impervious Area
Tc (min)	Length (feet)	Slop (ft/f	
5.0			Direct Entry,

# Subcatchment 5P: ALDERSGATE (N)



# Summary for Subcatchment 6P: ALDERSGATE (S)

Runoff = 0.05 cfs @ 7.90 hrs, Volume= 0.017 af, Depth= 3.27"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

Are	ea (	ac)	CN	l De	scrip	otior	1																		
	0.0	)62	98		ved r							/ers	, Н	SG	С										
	0.0	062	98	3 10	0.009	% Ir	mpe	rvio	ous	Are	ea														
T (mir		Len (fe	igth eet)	Slope (ft/ft		eloo ft/s	city ec)	С	apa (	acit cfs		Des	crip	otio	า										
5.	0			•	<u> </u>							Dire	ect	Ent	ry,										
						S	ubo	cat	chı	me	nt	6P:	: <b>A</b>	LD	ER	SG	AT	Έ(	(S)						
		_								Hyd	lrogr	aph													_
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0	.045			ł										10	Y	E/	١R	R	air	nfa	all=	=3.	50	)"	
	0.04-														Ru	no	off	A	rea	a=(	0.0	62	2 a	C	
-	.035												F	Ru	no	ff	Vc	lu	m	e=	0.0	01'	7 a	af	
Flow (cfs) 0	0.03-														F	Ru	no	ff	De	pt	th=	=3.	27	<b>,</b> ,,	
<b>Flov</b>	.025																		T	C=	=5.	0 I	mi	n	*****
	0.02																				CN	=(	)/9	8	
0	.015-																								****
	0.01-						$\overline{\mathcal{D}}$	$\overline{m}$																	
0	.005																								
	0-		2 4	6 8	10	 12	14	16	 18	20	22	24	26	28	30	32	34	36	38	40	42	44	46	48	-

4487-01 Springbrook - Pre	Type IA 24-hr	25 YEAR Rain	nfall=4.00"
Prepared by AKS ENGINEERING & FORESTRY, LLC		Printed	10/7/2022
HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solution	ons LLC		Page 29

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SBUH method, Split Pervious/Imperv. Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1P: W	Runoff Area=1,284,759 sf 2.03% Impervious Runoff Depth=2.40" Flow Length=1,389' Tc=29.8 min CN=84/98 Runoff=12.24 cfs 5.897 af
Subcatchment 2P: SE	Runoff Area=3,125,647 sf 1.30% Impervious Runoff Depth=2.06" ow Length=2,065' Tc=27.1 min CN=80/98 Runoff=24.92 cfs 12.342 af
Subcatchment 3P: MOUNTAINVIEW	Runoff Area=25,396 sf   60.03% Impervious   Runoff Depth=3.21" Tc=5.0 min   CN=84/98   Runoff=0.46 cfs  0.156 af
Subcatchment 4P: CENTER (N)	Runoff Area=5,401 sf   80.65% Impervious   Runoff Depth=3.43" Tc=5.0 min   CN=80/98   Runoff=0.11 cfs  0.035 af
Subcatchment 5P: ALDERSGATE (N	I) Runoff Area=9,103 sf 85.65% Impervious Runoff Depth=3.48" Tc=5.0 min CN=77/98 Runoff=0.18 cfs 0.061 af
Subcatchment 6P: ALDERSGATE (S	Runoff Area=0.062 ac 100.00% Impervious Runoff Depth=3.77" Tc=5.0 min CN=0/98 Runoff=0.06 cfs 0.019 af
Total Runoff Area = 102	.227 ac Runoff Volume = 18.511 af Average Runoff Depth = 2.17"

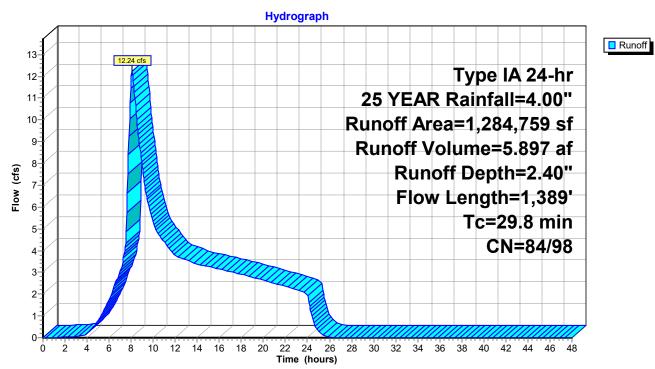
97.83% Pervious = 100.004 ac 2.17% Impervious = 2.223 ac

# Summary for Subcatchment 1P: W

Runoff = 12.24 cfs @ 8.06 hrs, Volume= 5.897 af, Depth= 2.40"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

696,873 82 Woods/grass comb., Poor, HSG C 536,093 86 Woods/grass comb., Poor, HSG D 26,049 98 Paved roads w/curbs & sewers, HSG C 9,932 96 Gravel surface, HSG C	
26,049 98 Paved roads w/curbs & sewers, HSG C	
9 932 96 Gravel surface HSG C	
11,238 74 >75% Grass cover, Good, HSG C	
4,574 80 >75% Grass cover, Good, HSG D	
1,284,759 84 Weighted Average	
1,258,710 84 97.97% Pervious Area	
26,049 98 2.03% Impervious Area	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
9.8 50 0.0480 0.08 <b>Sheet Flow, Sheet-Orchard</b> Woods: Light underbrush n= 0.400 P2= 2.50"	
7.9 658 0.0392 1.39 <b>Shallow Concentrated Flow, Shallow</b>	
Short Grass Pasture Kv= 7.0 fps	
12.1 681 0.0181 0.94 Shallow Concentrated Flow, Shallow	
Short Grass Pasture Kv= 7.0 fps	
29.8 1,389 Total	



### Subcatchment 1P: W

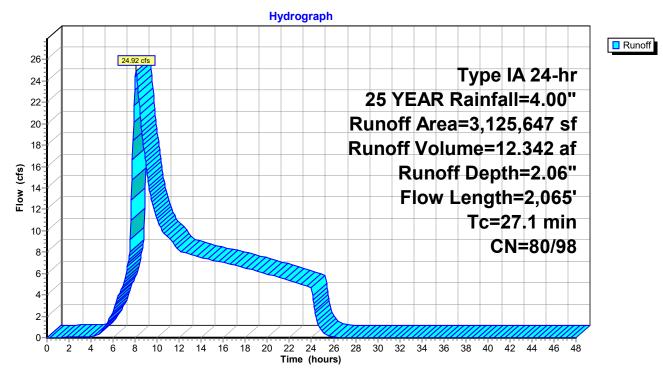
## Summary for Subcatchment 2P: SE

Runoff = 24.92 cfs @ 8.06 hrs, Volume= 12.342 af, Depth= 2.06"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

A	rea (sf)	CN	Description								
	38,333	82	Farmsteads	s, HSG C							
2,0	)22,883	79	Pasture/gra	sture/grassland/range, Fair, HSG C							
	195,843				ge, Fair, HSG D						
3	315,113	82	Woods/gras	ss comb., F	Poor, HSG C						
	39,683	98	Paved road	s w/curbs &	& sewers, HSG C						
	15,812	96	Gravel surfa	ace, HSG (	C						
	3,615	79	50-75% Gra	ass cover, l	Fair, HSG C						
1	171,757		Brush, Goo	,							
	21,606			s cover, Go	bod, HSG C						
*	1,002	100	Stream								
,	25,647		Weighted A	verage							
3,0	)84,962		98.70% Pei								
	40,685 98 1.30% Impervious Area										
т.	المربية مرالي	01.000	Valasita.	0	Description						
Tc	Length	Slope		Capacity	Description						
(min)	(feet)	(ft/ft)		(cfs)							
1.3	16	0.1182	2 0.21		Sheet Flow,						
0.4	000	0.0000			Grass: Short n= 0.150 P2= 2.50"						
3.1	268	0.0208	3 1.44		Shallow Concentrated Flow,						
7 4	400	0.0404			Nearly Bare & Untilled Kv= 10.0 fps						
7.4	493	0.0124	1.11		Shallow Concentrated Flow,						
1 5	170	0 0000	0 0 0 1		Nearly Bare & Untilled Kv= 10.0 fps						
1.5	178	0.0822	2 2.01		Shallow Concentrated Flow,						
8.2	486	0.0200	0.00		Short Grass Pasture Kv= 7.0 fps						
0.2	400	0.0200	0.99		Shallow Concentrated Flow,						
5.6	624	0.0709	1.86		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,						
5.0	024	0.0708	0.00		Short Grass Pasture Kv= 7.0 fps						
	2 065	Total									

27.1 2,065 Total



## Subcatchment 2P: SE

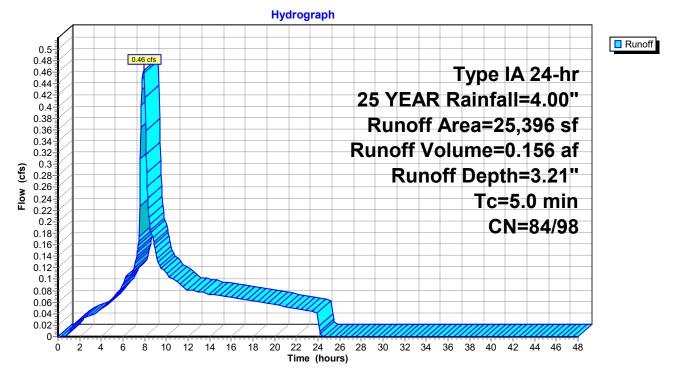
# Summary for Subcatchment 3P: MOUNTAINVIEW

Runoff = 0.46 cfs @ 7.92 hrs, Volume= 0.156 af, Depth= 3.21"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

Are	a (sf)	CN	Description							
6	6,447	86	Woods/gras	s comb., P	Poor, HSG D					
15	5,246	98	Paved roads	s w/curbs &	& sewers, HSG C					
3	3,703	80	>75% Grass	s cover, Go	bod, HSG D					
25	5,396	92	Weighted Av	verage						
10	0,150	84	39.97% Per	vious Area						
15	5,246	98	60.03% Imp	60.03% Impervious Area						
Tc L (min)	_ength (feet)	Slop (ft/ft		Capacity (cfs)	Description					
5.0					Direct Entry,					

### Subcatchment 3P: MOUNTAINVIEW

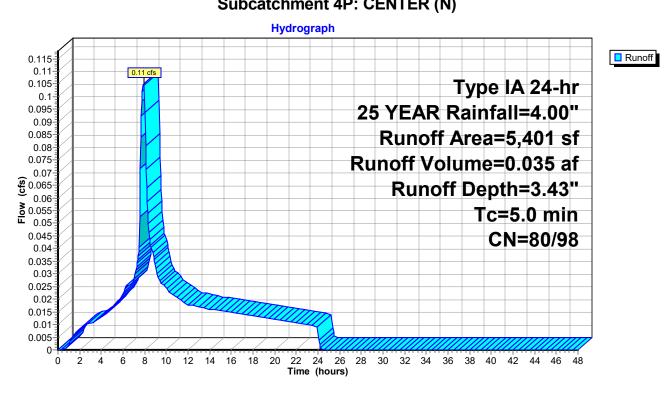


## Summary for Subcatchment 4P: CENTER (N)

Runoff = 0.11 cfs @ 7.91 hrs, Volume= 0.035 af, Depth= 3.43"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

Α	rea (sf)	CN	Description							
	4,356	98	Paved road	s w/curbs &	& sewers, HSG C					
	1,045	80	>75% Gras	s cover, Go	bod, HSG D					
	5,401	95	Weighted A	verage						
	1,045	80	19.35% Per	vious Area						
	4,356	6 98 80.65% Impervious Area								
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description					
5.0					Direct Entry,					
	Subcatchment 4P: CENTER (N)									



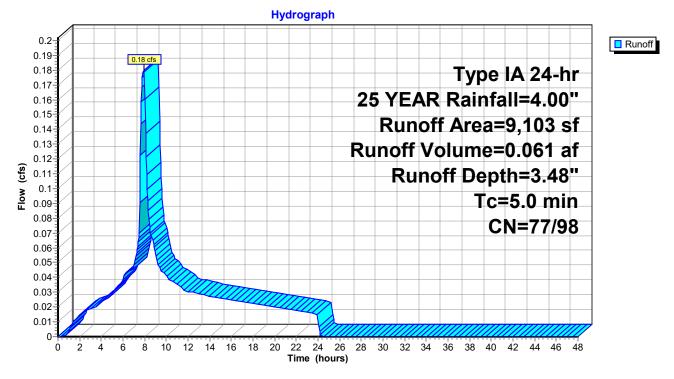
# Summary for Subcatchment 5P: ALDERSGATE (N)

Runoff = 0.18 cfs @ 7.90 hrs, Volume= 0.061 af, Depth= 3.48"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

A	rea (sf)	CN	Description						
	7,797	98	Paved roads w/curbs & sewers, HSG C						
	653	80	>75% Grass cover, Good, HSG D						
	653	74	>75% Grass cover, Good, HSG C						
	9,103	95	Weighted Average						
	1,306	77	14.35% Pervious Area						
	7,797	98	35.65% Impervious Area						
Tc (min)	Length (feet)	Slop (ft/f							
5.0			Direct Entry,						

# Subcatchment 5P: ALDERSGATE (N)



# Summary for Subcatchment 6P: ALDERSGATE (S)

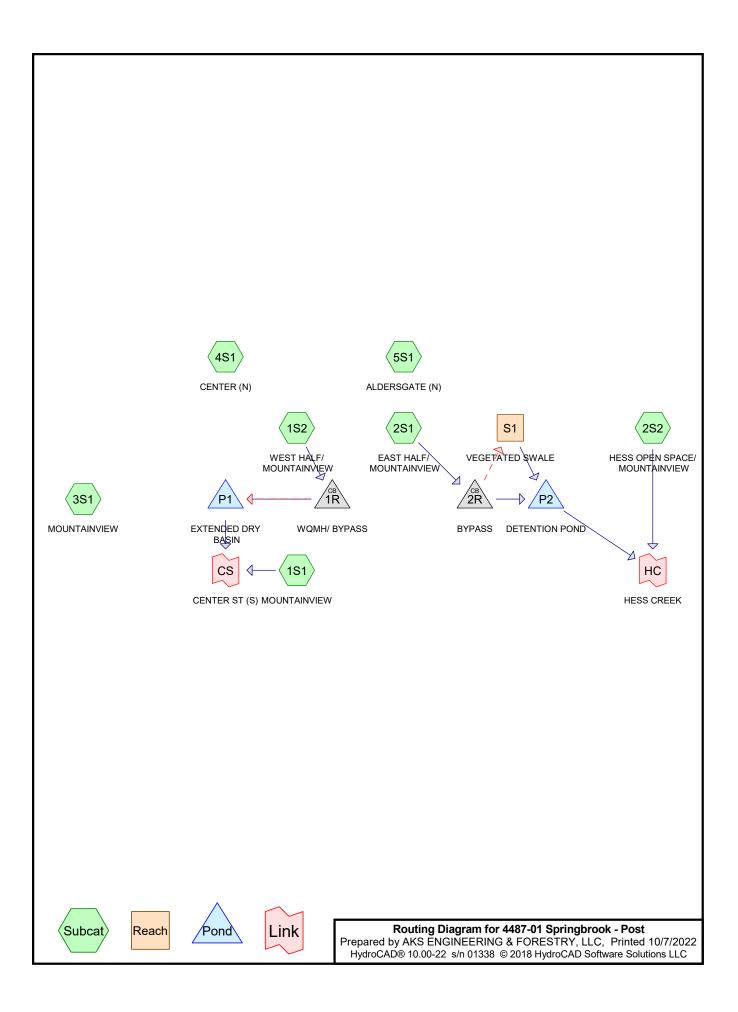
Runoff = 0.06 cfs @ 7.90 hrs, Volume= 0.019 af, Depth= 3.77"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

Area	(ac)	CN	Desci																	
0	.062	98	Pave	d road	s w/c	urbs 8	k se\	vers, l	ISG	С										
0	.062	98	100.0	0% Im	ipervi	ious A	rea													
Tc (min)	Lenç (fe		lope ft/ft)	Veloci (ft/se		Capac (cf		Descr	iptio	n										
5.0								Direc	t Enf	ry,										
				Su	bcat	tchm	ent	6P: /	<b>ט</b> וז	FR	SG	ΔΤ	F (	S)						
				00	bou		/drog						- (	-,						
0.06	5-																			Runoff
0.0	6		0.06 cfs										T	\/ <b>n</b>	~	^	21	h	~	
0.05	5								~									l-hi		
0.0	5		T												-			00'		
0.04	5									Ru	no	off	Ar	ea	=0	.0	62	ac	;	
0.0	4								Ru	no	ff	Vo	lu	me	<b>)=(</b>	).(	)19	) a	f	
<b>(22</b> ) 0.03	5									F	Rui	10	ff	De	ptl	h=	3.	77'	•	
( <b>ct</b> ) 0.03	3																	nir		
0.02	5																	/98		
0.0	2																	/00		
0.01	5																			
0.0	1		~	VII.		m	The													
0.00	5																			
	0			<u> </u>		· · · · · · · · · · · · · · · · · · ·	<del></del>												Ų	
	0 2	4 6	8 10	12 14	4 16	18 20		24 26 (hours		30	32	34	36	38 4	40 4	42	44	46 4	48	



# **Appendix B:** HYDROCAD REPORTS FOR POST-DEVELOPED CONDITION STORM EVENTS



Type IA 24-hr 1/2 2 YEAR Rainfall=1.25" 4487-01 Springbrook - Post Prepared by AKS ENGINEERING & FORESTRY, LLC Printed 10/7/2022 HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solutions LLC Page 2 Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SBUH method, Split Pervious/Imperv. Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method Subcatchment 1S1: MOUNTAINVIEW Runoff Area=3,037 sf 88.90% Impervious Runoff Depth=0.94" Tc=5.0 min CN=80/98 Runoff=0.02 cfs 238 cf Subcatchment 1S2: WEST HALF/ Runoff Area=1,100,942 sf 68.23% Impervious Runoff Depth=0.74" Tc=8.0 min CN=77/98 Runoff=4.48 cfs 68.170 cf Subcatchment 2S1: EAST HALF/ Runoff Area=2,261,386 sf 64.41% Impervious Runoff Depth=0.70" Tc=10.0 min CN=75/98 Runoff=8.52 cfs 131,412 cf Subcatchment 2S2: HESS OPEN SPACE/ Runoff Area=1,063,815 sf 4.10% Impervious Runoff Depth=0.09" Flow Length=455' Tc=8.9 min CN=72/98 Runoff=0.26 cfs 8,108 cf Subcatchment 3S1: MOUNTAINVIEW Runoff Area=15,878 sf 81.07% Impervious Runoff Depth=0.87" Tc=5.0 min CN=80/98 Runoff=0.08 cfs 1,153 cf Runoff Area=5,141 sf 75.43% Impervious Runoff Depth=0.82" Subcatchment 4S1: CENTER (N) Tc=5.0 min CN=80/98 Runoff=0.02 cfs 353 cf Subcatchment 5S1: ALDERSGATE (N) Runoff Area=2,874 sf 58.91% Impervious Runoff Depth=0.64" Tc=5.0 min CN=74/98 Runoff=0.01 cfs 153 cf Avg. Flow Depth=0.53' Max Vel=0.26 fps Inflow=1.68 cfs 83,214 cf Reach S1: VEGETATED SWALE n=0.240 L=132.0' S=0.0050 '/' Capacity=10.83 cfs Outflow=1.66 cfs 83,214 cf Pond 1R: WQMH/ BYPASS Peak Elev=227.44' Inflow=4.48 cfs 68,170 cf Primary=0.55 cfs 926 cf Secondary=3.93 cfs 67,244 cf Outflow=4.48 cfs 68,170 cf Pond 2R: BYPASS Peak Elev=223.05' Inflow=8.52 cfs 131,412 cf Primary=6.84 cfs 48,198 cf Secondary=1.68 cfs 83,214 cf Outflow=8.52 cfs 131,412 cf Pond P1: EXTENDED DRY BASIN Peak Elev=227.32' Storage=43,885 cf Inflow=4.48 cfs 68,170 cf Outflow=0.37 cfs 68.185 cf Pond P2: DETENTION POND Peak Elev=216.87' Storage=106,032 cf Inflow=8.49 cfs 131,412 cf Outflow=0.39 cfs 84,811 cf Inflow=0.37 cfs 68.423 cf Link CS: CENTER ST (S) Primary=0.37 cfs 68,423 cf Link HC: HESS CREEK Inflow=0.54 cfs 92,918 cf Primary=0.54 cfs 92,918 cf Total Runoff Area = 4,453,073 sf Runoff Volume = 209,586 cf Average Runoff Depth = 0.56"

48.97% Pervious = 2,180,498 sf 51.03% Impervious = 2,272,575 sf

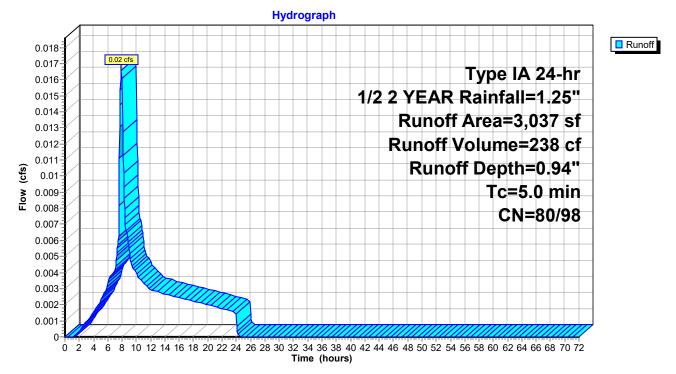
## Summary for Subcatchment 1S1: MOUNTAINVIEW

Runoff = 0.02 cfs @ 7.91 hrs, Volume= 238 cf, Depth= 0.94"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

A	rea (sf)	CN	Description				
	2,700	98	Paved road	s w/curbs &	& sewers		
	337	80	>75% Gras	s cover, Go	ood, HSG D		
	3,037	96	Weighted Average				
	337	80	11.10% Pervious Area				
	2,700	98	88.90% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description		
5.0					Direct Entry,		

#### Subcatchment 1S1: MOUNTAINVIEW



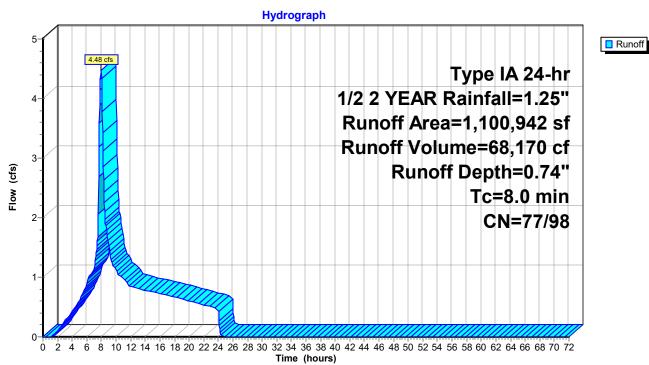
### Summary for Subcatchment 1S2: WEST HALF/ MOUNTAINVIEW

Runoff = 4.48 cfs @ 7.97 hrs, Volume= 68,170 cf, Depth= 0.74"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

	Area (sf)	CN	Description				
*	336,878	98	Right-of-Way Impervious area				
*	345,240	98	120 Lots >2877 sf (2877sf/Lot)				
*	57,717	98	Cluster Lots (90% of total area)				
*	11,309	98	Open Space Impervious				
	170,116	74					
	179,682	80	80 >75% Grass cover, Good, HSG D				
	1,100,942	91	91 Weighted Average				
	349,798	77	31.77% Pervious Area				
	751,144	98	68.23% Impervious Area				
	Tc Length (min) (feet)	Sloj (ft/					
	8.0		Direct Entry,				





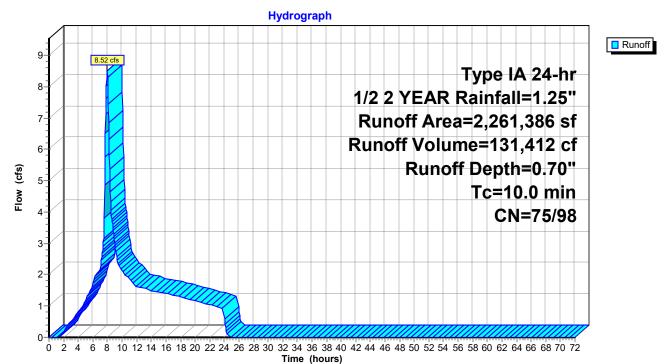
#### Summary for Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

Runoff = 8.52 cfs @ 7.98 hrs, Volume= 131,412 cf, Depth= 0.70"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

	Area (sf)	CN	Description				
*	650,436	98	Right-of-Way Impervious area				
*	716,373	98	249 Lots >2877 sf (2877sf/Lot)				
*	47,186	98	Cluster Lots (90% of total area)				
*	42,676	98	Open Space Impervious				
	547,140	74	>75% Grass cover, Good, HSG C				
	195,956	80	>75% Grass cover, Good, HSG D				
*	14,344	86	Playground surfacing				
	47,275	71	71 Meadow, non-grazed, HSG C				
	2,261,386	90	Weighted Average				
	804,715	75	35.59% Pervious Area				
	1,456,671	98	64.41% Impervious Area				
		~					
	Tc Length	Slo					
	(min) (feet)	(ft/	ft) (ft/sec) (cfs)				
	10.0		Direct Entry,				

#### Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

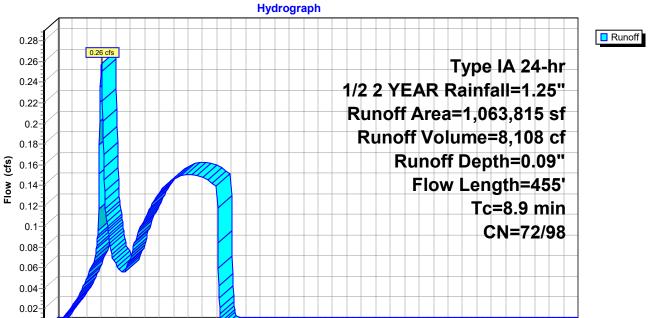


## Summary for Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

Runoff = 0.26 cfs @ 7.98 hrs, Volume= 8,108 cf, Depth= 0.09"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

	A	rea (sf)	CN	I Description					
		11,944	98	98 Paved roads w/curbs & sewers					
	8	15,065	71	Meadow, no	Meadow, non-grazed, HSG C				
		80,425	78	Meadow, no	on-grazed,	HSG D			
*		30,659	98	Paved Path	IS				
		19,458	96	Gravel surfa	ace, HSG (				
		92,519				bod, HSG C			
		7,358			,	bod, HSG D			
		5,373		•	ss comb., F	Poor, HSG C			
*		1,014	100	Stream					
		63,815		Weighted A					
		20,198		95.90% Pei					
		43,617	98	4.10% Impe	ervious Are	а			
	_		~						
,	Tc	Length	Slope		Capacity	Description			
(	min)	(feet)	(ft/ft		(cfs)				
	5.9	100	0.0740	0.28		Sheet Flow,			
						Range n= 0.130 P2= 2.50"			
	2.5	260	0.0600	) 1.71		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.5	95	0.2200	) 3.28		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	8.9	455	Total						



## Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

0-

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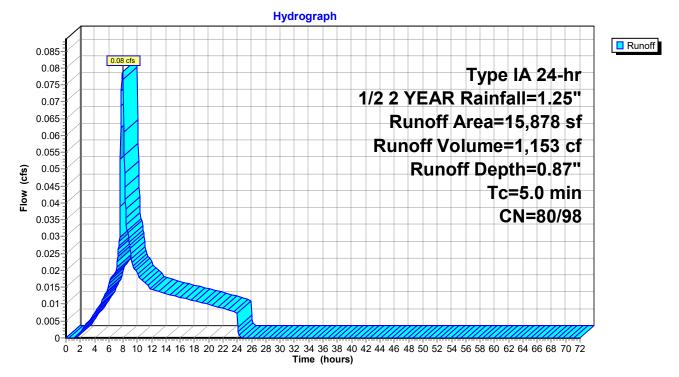
## Summary for Subcatchment 3S1: MOUNTAINVIEW

Runoff = 0.08 cfs @ 7.91 hrs, Volume= 1,153 cf, Depth= 0.87"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

A	rea (sf)	CN	Description			
	12,872	98	Paved road	s w/curbs &	& sewers	
	3,006	80	>75% Gras	s cover, Go	ood, HSG D	
	15,878	95	5 Weighted Average			
	3,006	80	80 18.93% Pervious Area			
	12,872	98	81.07% Imp	pervious Are	rea	
_						
Тс	Length	Slop	,	Capacity	•	
(min)	(feet)	(ft/ft	:) (ft/sec)	(cfs)		
5.0					Direct Entry,	

#### Subcatchment 3S1: MOUNTAINVIEW



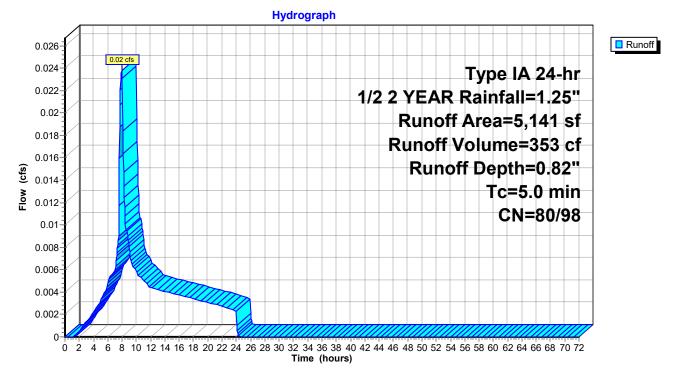
## Summary for Subcatchment 4S1: CENTER (N)

Runoff = 0.02 cfs @ 7.92 hrs, Volume= 353 cf, Depth= 0.82"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

A	rea (sf)	CN	Description			
	3,878	98	Paved road	s w/curbs &	& sewers	
	1,263	80	>75% Gras	s cover, Go	bod, HSG D	
	5,141	94	4 Weighted Average			
	1,263	80	24.57% Pei	rvious Area	1	
	3,878	98	75.43% lmp	pervious Are	ea	
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description	
5.0					Direct Entry,	

## Subcatchment 4S1: CENTER (N)



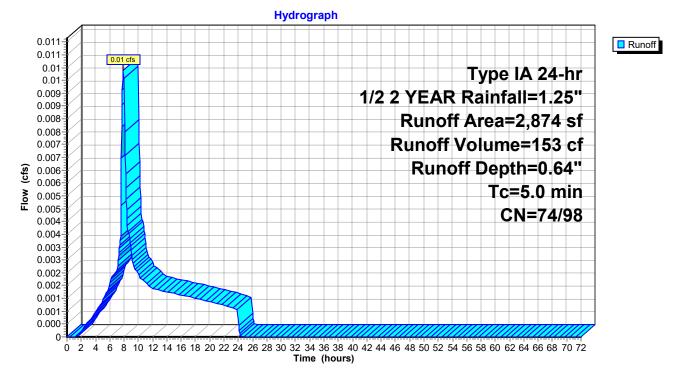
### Summary for Subcatchment 5S1: ALDERSGATE (N)

Runoff = 0.01 cfs @ 7.91 hrs, Volume= 153 cf, Depth= 0.64"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 1/2 2 YEAR Rainfall=1.25"

A	rea (sf)	CN	Description		
	1,693	98	Paved road	s w/curbs &	& sewers
	1,181	74	>75% Gras	s cover, Go	bod, HSG C
	2,874	88	Weighted A	verage	
	1,181	74	41.09% Pervious Area		
	1,693	98	58.91% Imp	pervious Are	ea
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description
5.0					Direct Entry,

## Subcatchment 5S1: ALDERSGATE (N)



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Type IA 24-hr 1/2 2 YEAR Rainfall=1.25" Printed 10/7/2022 Page 11

#### Summary for Reach S1: VEGETATED SWALE

10.83

2.970

Inflow 1.68 cfs @ 7.98 hrs, Volume= 83.214 cf = 8.04 hrs, Volume= 83,214 cf, Atten= 1%, Lag= 3.7 min Outflow = 1.66 cfs @

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 0.26 fps, Min. Travel Time= 8.5 min Avg. Velocity = 0.15 fps, Avg. Travel Time= 14.7 min

Peak Storage= 852 cf @ 8.04 hrs Average Depth at Peak Storage= 0.53' Bank-Full Depth= 1.50' Flow Area= 22.5 sf, Capacity= 10.83 cfs

Custom cross-section, Length= 132.0' Slope= 0.0050 '/' Constant n = 0.240Inlet Invert= 221.00', Outlet Invert= 220.34'

‡

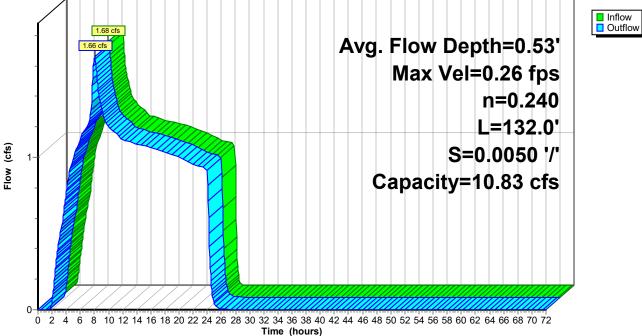
1.50

22.5

	Offse	et Eleva	tion	Cha	n.Depth		
	(fee		eet)	ona	(feet)		
_	-9.5	1 1	.50		0.00		
	-7.0		.50		1.00		
	-5.0		0.00		1.50		
	5.0		0.00		1.50		
	7.0		.50		1.00		
	9.5		.50		0.00		
	Depth E	nd Area	Pe	rim.	S	Storage	Discharge
	(feet)	(sq-ft)	(f	eet)	(cub	ic-feet)	(cfs)
	0.00	0.0	1	10.0		0	0.00
	0.50	6.0	1	14.1		792	1.49

19.5

Hydrograph 1.68 cfs



## **Reach S1: VEGETATED SWALE**

## Summary for Pond 1R: WQMH/ BYPASS

Inflow Area =	1,100,942 sf,	68.23% Impervious,	Inflow Depth = $0.74$ " f	or 1/2 2 YEAR event
Inflow =	4.48 cfs @	7.97 hrs, Volume=	68,170 cf	
Outflow =	4.48 cfs @	7.97 hrs, Volume=	68,170 cf, Atten=	0%, Lag= 0.0 min
Primary =	0.55 cfs @	7.97 hrs, Volume=	926 cf	-
Secondary =	3.93 cfs @	7.97 hrs, Volume=	67,244 cf	

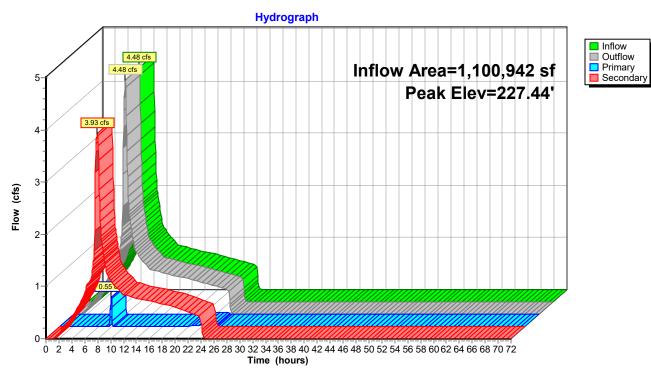
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 227.44' @ 7.97 hrs

Routing	Invert	Outlet Devices
Primary	227.15'	36.0" Round High Flow Bypass
		L= 50.0' CPP, mitered to conform to fill, Ke= 0.700
		Inlet / Outlet Invert= 227.15' / 224.50' S= 0.0530 '/' Cc= 0.900
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
Secondary	225.55'	12.0" Round WQ Flow
		L= 10.0' CPP, mitered to conform to fill, Ke= 0.700
		Inlet / Outlet Invert= 225.55' / 225.35' S= 0.0200 '/' Cc= 0.900
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
Device 2	225.15'	12.0" Round WQ Flow
		L= 32.5' CPP, mitered to conform to fill, Ke= 0.700
		Inlet / Outlet Invert= 225.15' / 224.50' S= 0.0200 '/' Cc= 0.900
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
	Primary	Primary 227.15' Secondary 225.55'

Primary OutFlow Max=0.55 cfs @ 7.97 hrs HW=227.43' TW=225.51' (Dynamic Tailwater) ☐ 1=High Flow Bypass (Inlet Controls 0.55 cfs @ 1.60 fps)

Secondary OutFlow Max=3.93 cfs @ 7.97 hrs HW=227.43' TW=225.51' (Dynamic Tailwater) -2=WQ Flow (Inlet Controls 3.93 cfs @ 5.00 fps)

**Galaxy Control of the second second** 



Pond 1R: WQMH/ BYPASS

## Summary for Pond 2R: BYPASS

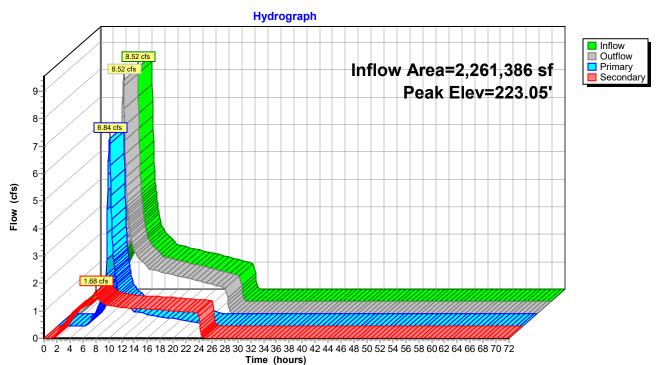
Inflow Area =	2,261,386 sf,	64.41% Impervious,	Inflow Depth = 0.70" for 1/2 2 YEAR event
Inflow =	8.52 cfs @	7.98 hrs, Volume=	131,412 cf
Outflow =	8.52 cfs @	7.98 hrs, Volume=	131,412 cf, Atten= 0%, Lag= 0.0 min
Primary =	6.84 cfs @	7.98 hrs, Volume=	48,198 cf
Secondary =	1.68 cfs @	7.98 hrs, Volume=	83,214 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 223.05' @ 7.98 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	221.20'	<b>8.0" Round WQ Flow</b> L= 40.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 221.20' / 221.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Primary	222.00'	<b>36.0" Round High Flow Bypass</b> L= 50.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 222.00' / 221.20' S= 0.0160 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf

**Primary OutFlow** Max=6.79 cfs @ 7.98 hrs HW=223.05' TW=213.66' (Dynamic Tailwater) **2=High Flow Bypass** (Inlet Controls 6.79 cfs @ 3.08 fps)

Secondary OutFlow Max=1.68 cfs @ 7.98 hrs HW=223.05' TW=221.53' (Dynamic Tailwater) T=WQ Flow (Barrel Controls 1.68 cfs @ 4.81 fps)



## Pond 2R: BYPASS

# Summary for Pond P1: EXTENDED DRY BASIN

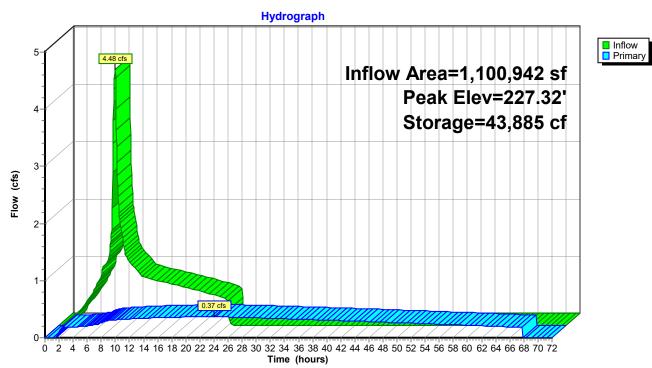
Inflow Area = Inflow = Outflow = Primary =	flow = 4.48 cfs @ 7.97 hrs, Volume= 68,170 cf utflow = 0.37 cfs @ 24.06 hrs, Volume= 68,185 cf, Atten= 92%, Lag= 965.4 min						
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 227.32' @ 24.06 hrs Surf.Area= 17,869 sf Storage= 43,885 cf							
Plug-Flow detention time= 1,269.3 min calculated for 68,138 cf (100% of inflow) Center-of-Mass det. time= 1,270.4 min(1,991.7 - 721.3)							
Volume	Invert Avail.Sto	orage Storage Descr	ption				
#1 22			Data (Irregular) Lis	ted below (Recalc)			
Elevation (feet)	Surf.Area F (sq-ft)	Perim. Inc.Stor (feet) (cubic-fee	-	Wet.Area (sq-ft)			
224.50			0 0	13,412			
230.50		635.5 109,82		25,710			
Device Routi	ng Invert	Outlet Devices					
#1 Prima		12.0" Round Outfa	12.0" Round Outfall Pipe				
		L= 50.0' CPP, square edge headwall, Ke= 0.500					
				= 0.0050 '/' Cc= 0.900			
#2 Devid	ce 1 223.50'			or, Flow Area= 0.79 sf			
#2 Devic #3 Devic		-		C= 0.600			
		Limited to weir flow at low heads					
#4 Prima	ary 225.50'	12.0" Round Outfall Pipe					
		L= 50.0' CPP, squa Inlet / Outlet Invert=		Ke= 0.500 = 0.0250 '/'     Cc= 0.900			
				or, Flow Area= 0.79 sf			
#5 Devic	ce 4 229.00'	<b>30.0" x 16.0" Horiz.</b> Limited to weir flow		C= 0.600			
Primary OutFI	ow Max=0.37 cfs	@ 24.06 hrs HW=227	.32' TW=0.00' (Dy	namic Tailwater)			

Primary OutFlow Max=0.37 cfs @ 24.06 hrs HW=227.32' TW=0.00' (Dynamic Tailwater) -1=Outfall Pipe (Passes 0.37 cfs of 6.40 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.37 cfs @ 9.27 fps) 3=Upper Ditch Inlet (Controls 0.00 cfs)

**4=Outfall Pipe** (Passes 0.00 cfs of 4.34 cfs potential flow)

**5=Upper Ditch Inlet** (Controls 0.00 cfs)



### Pond P1: EXTENDED DRY BASIN

## Summary for Pond P2: DETENTION POND

Inflow Area	a =	2,261,386 sf, 64.41% Impervious, Inflow Depth = 0.70" for 1/2 2 YEAR event
Inflow	=	8.49 cfs @ 7.98 hrs, Volume= 131,412 cf
Outflow	=	0.39 cfs @ 24.31 hrs, Volume= 84,811 cf, Atten= 95%, Lag= 979.9 min
Primary	=	0.39 cfs @ 24.31 hrs, Volume= 84,811 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 216.87' @ 24.31 hrs Surf.Area= 26,985 sf Storage= 106,032 cf Flood Elev= 219.00' Surf.Area= 32,084 sf Storage= 168,924 cf

Plug-Flow detention time= 1,736.9 min calculated for 84,811 cf (65% of inflow) Center-of-Mass det. time= 1,524.1 min (2,253.6 - 729.5)

Volume	Inver	t Avail.Sto	orage	Storage Descript	ion					
#1	#1 212.00' 202,272		272 cf	cf Custom Stage Data (Irregular) Listed below (Recalc)						
Elevatio (fee			Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)				
212.0	00	16,833	666.0	0	0	16,833				
213.0	00	18,860	684.8	17,837	17,837	18,964				
220.0	00	34,628	816.8	184,435	202,272	35,589				
Device	Routing	Invert	Outl	et Devices						
#1	Primary	210.50'	18.0	" Round Outfall						
	-				edge headwall, k					
						= 0.3900 '/'     Cc= 0.900				
				n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf						
#2	Device 1	211.00'		12.0" Round Detention Pipe 1						
			L= 10.0' CMP, square edge headwall, Ke= 0.500							
			Inlet / Outlet Invert= 211.00' / 210.95' S= 0.0050 '/' Cc= 0.900							
#2	Davias 2	211.00'		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf <b>2.5" Vert. Detention Orifice 1</b> C= 0.600						
#3 #4	Device 2		-							
#4	Device 2	216.90'		<b>30.0" x 16.0" Horiz. Upper Ditch Inlet 1</b> C= 0.600						
#5	Device 1	213.00'		Limited to weir flow at low heads						
#5	Device 1	213.00		<b>12.0" Round Detention Pipe 2</b> L= 30.0' CMP, square edge headwall, Ke= 0.500						
						= 0.0250 '/' Cc= 0.900				
						r, Flow Area= 0.79 sf				
#6	Device 5	218.20'			oper Ditch Inlet 2					
110	Device o	210.20		ted to weir flow at		0 0.000				
			<b>_</b>	ica to non now at						
Primary	OutFlow	/lax=0.39 cfs	@ 24.3	31 hrs HW=216.8	7' TW=0.00' (Dyr	namic Tailwater)				
				cfs notential flow)		,				

**1=Outfall** (Passes 0.39 cfs of 20.17 cfs potential flow)

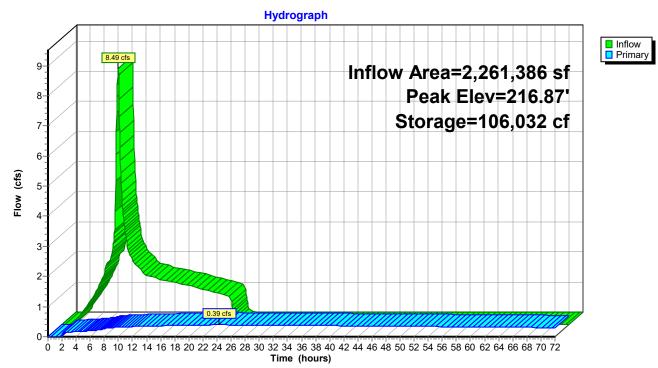
-2=Detention Pipe 1 (Passes 0.39 cfs of 8.76 cfs potential flow)

**3=Detention Orifice 1** (Orifice Controls 0.39 cfs @ 11.56 fps)

4=Upper Ditch Inlet 1 (Controls 0.00 cfs)

-5=Detention Pipe 2 (Passes 0.00 cfs of 6.94 cfs potential flow)

**6=Upper Ditch Inlet 2** (Controls 0.00 cfs)



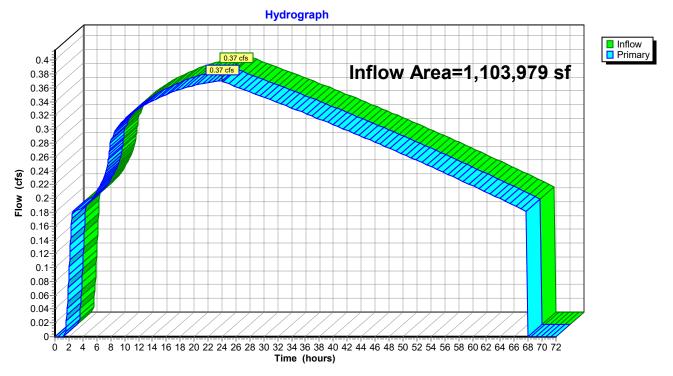
# Pond P2: DETENTION POND

4487-01 Springbrook - PostType IA 24-hr1/2 2 YEAR Rainfall=1.25"Prepared by AKS ENGINEERING & FORESTRY, LLCPrinted10/7/2022HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solutions LLCPage 20

## Summary for Link CS: CENTER ST (S)

Inflow Area	a =	1,103,979 sf, 68.28% Impervious,	Inflow Depth = 0.74" for 1/2 2 YEAR event
Inflow	=	0.37 cfs @ 23.98 hrs, Volume=	68,423 cf
Primary	=	0.37 cfs @ 23.98 hrs, Volume=	68,423 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



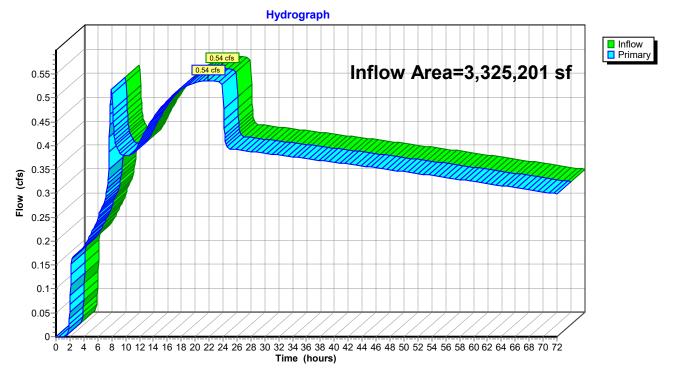
## Link CS: CENTER ST (S)

4487-01 Springbrook - PostType IA 24-hr1/2 2 YEAR Rainfall=1.25"Prepared by AKS ENGINEERING & FORESTRY, LLCPrinted10/7/2022HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solutions LLCPage 21

## Summary for Link HC: HESS CREEK

Inflow Area	a =	3,325,201 sf, 45.12% Impervious, Inflow Depth > 0.34" for 1/2 2 YEAR event
Inflow	=	0.54 cfs @ 21.90 hrs, Volume= 92,918 cf
Primary	=	0.54 cfs $\overline{@}$ 21.90 hrs, Volume= 92,918 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



## Link HC: HESS CREEK

<b>4487-01 Springbrook - Post</b> Prepared by AKS ENGINEERING & HydroCAD® 10.00-22 s/n 01338 © 2018 F	
Runoff by S	0.00-72.00 hrs, dt=0.05 hrs, 1441 points SBUH method, Split Pervious/Imperv. -Ind method - Pond routing by Dyn-Stor-Ind method
Subcatchment 1S1: MOUNTAINVIEW	Runoff Area=3,037 sf 88.90% Impervious Runoff Depth=2.12" Tc=5.0 min CN=80/98 Runoff=0.04 cfs 536 cf
Subcatchment 1S2: WEST HALF/	Runoff Area=1,100,942 sf 68.23% Impervious Runoff Depth=1.78" Tc=8.0 min CN=77/98 Runoff=10.58 cfs 163,718 cf
Subcatchment 2S1: EAST HALF/	Runoff Area=2,261,386 sf 64.41% Impervious Runoff Depth=1.69" Tc=10.0 min CN=75/98 Runoff=19.93 cfs 319,268 cf
	<b>E/</b> Runoff Area=1,063,815 sf 4.10% Impervious Runoff Depth=0.60" Flow Length=455' Tc=8.9 min CN=72/98 Runoff=1.80 cfs 53,194 cf
Subcatchment 3S1: MOUNTAINVIEW	Runoff Area=15,878 sf 81.07% Impervious Runoff Depth=2.01" Tc=5.0 min CN=80/98 Runoff=0.18 cfs 2,658 cf
Subcatchment 4S1: CENTER (N)	Runoff Area=5,141 sf 75.43% Impervious Runoff Depth=1.93" Tc=5.0 min CN=80/98 Runoff=0.06 cfs 827 cf
Subcatchment 5S1: ALDERSGATE (N)	Runoff Area=2,874 sf 58.91% Impervious Runoff Depth=1.59" Tc=5.0 min CN=74/98 Runoff=0.02 cfs 380 cf
Reach S1: VEGETATED SWALE n=0.240 L=	Avg. Flow Depth=0.60' Max Vel=0.28 fps Inflow=2.08 cfs 111,260 cf 132.0' S=0.0050 '/' Capacity=10.83 cfs Outflow=2.06 cfs 111,260 cf
Pond 1R: WQMH/ BYPASS Primary=7.14 cfs 94	Peak Elev=228.53' Inflow=10.58 cfs 163,718 cf ,305 cf Secondary=3.94 cfs 69,413 cf Outflow=10.58 cfs 163,718 cf
Pond 2R: BYPASS Primary=17.84 cfs 208,	Peak Elev=223.80' Inflow=19.93 cfs 319,268 cf 008 cf Secondary=2.08 cfs 111,260 cf Outflow=19.93 cfs 319,268 cf
Pond P1: EXTENDED DRY BASIN	Peak Elev=228.50' Storage=66,227 cf Inflow=10.58 cfs 163,718 cf Outflow=2.63 cfs 156,600 cf
Pond P2: DETENTION POND	Peak Elev=217.30' Storage=117,812 cf Inflow=19.90 cfs 319,268 cf Outflow=6.67 cfs 270,611 cf
Link CS: CENTER ST (S)	Inflow=2.64 cfs 157,136 cf Primary=2.64 cfs 157,136 cf
Link HC: HESS CREEK	Inflow=7.74 cfs 323,805 cf Primary=7.74 cfs 323,805 cf
	3 sf Runoff Volume = 540,582 cf Average Runoff Depth = 1.46" 97% Pervious = 2,180,498 sf 51.03% Impervious = 2,272,575 sf

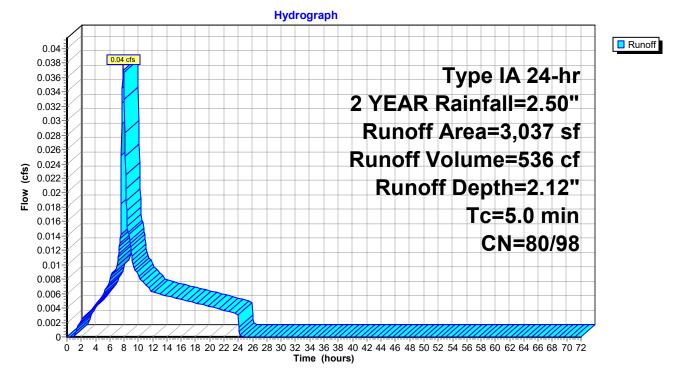
## Summary for Subcatchment 1S1: MOUNTAINVIEW

Runoff = 0.04 cfs @ 7.91 hrs, Volume= 536 cf, Depth= 2.12"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description					
	2,700	98	Paved road	s w/curbs &	& sewers			
	337	80	>75% Gras	s cover, Go	ood, HSG D			
	3,037	96	Weighted A	Weighted Average				
	337	80	11.10% Pervious Area					
	2,700	98	88.90% Impervious Area					
Тс	Length	Slop		Capacity	Description			
(min)	(feet)	(ft/ft	:) (ft/sec)	(cfs)				
5.0					Direct Entry,			

#### Subcatchment 1S1: MOUNTAINVIEW



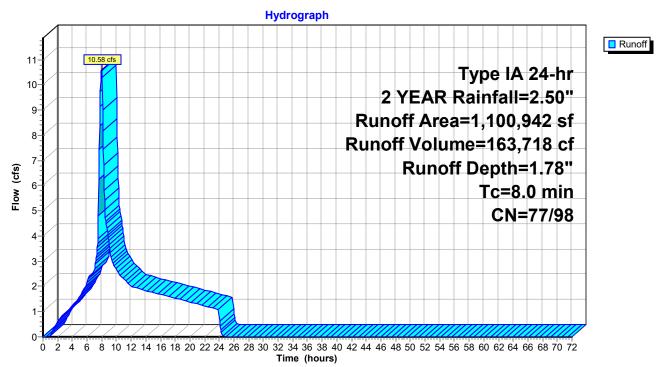
## Summary for Subcatchment 1S2: WEST HALF/ MOUNTAINVIEW

Runoff = 10.58 cfs @ 7.97 hrs, Volume= 163,718 cf, Depth= 1.78"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

	Area (sf)	CN	Description					
*	336,878	98	Right-of-Way Impervious area					
*	345,240	98	120 Lots >2877 sf (2877sf/Lot)					
*	57,717	98	Cluster Lots (90% of total area)					
*	11,309	98	Open Space Impervious					
	170,116	74	>75% Grass cover, Good, HSG C					
	179,682	80	80 >75% Grass cover, Good, HSG D					
	1,100,942	91	91 Weighted Average					
	349,798	77	31.77% Pervious Area					
	751,144	98	68.23% Impervious Area					
	Tc Length	Slop	pe Velocity Capacity Description					
	(min) (feet)	(ft/						
_	8.0		Direct Entry,					





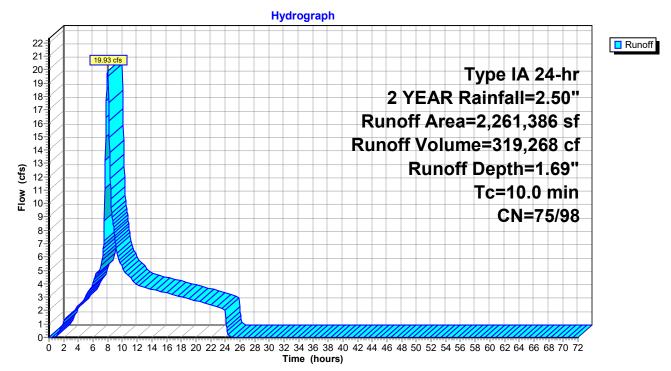
#### Summary for Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

Runoff = 19.93 cfs @ 7.98 hrs, Volume= 319,268 cf, Depth= 1.69"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

_	Area (sf)	CN	Description					
*	650,436	98	Right-of-Way Impervious area					
*	716,373	98	249 Lots >2877 sf (2877sf/Lot)					
*	47,186	98	Cluster Lots (90% of total area)					
*	42,676	98	Open Space Impervious					
	547,140	74	>75% Grass cover, Good, HSG C					
	195,956	80	>75% Grass cover, Good, HSG D					
*	14,344	86	Playground surfacing					
	47,275	71	71 Meadow, non-grazed, HSG C					
	2,261,386	90 Weighted Average						
	804,715	75	35.59% Pervious Area					
	1,456,671	98	64.41% Impervious Area					
	Tc Length	Slo	pe Velocity Capacity Description					
	(min) (feet)	(ft/	ft) (ft/sec) (cfs)					
	10.0		Direct Entry,					
			••					

#### Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

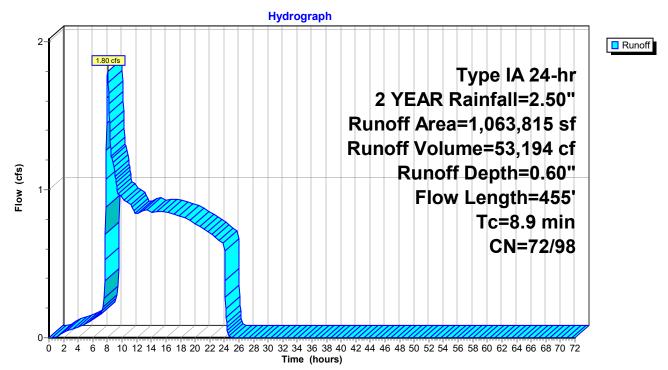


## Summary for Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

Runoff = 1.80 cfs @ 8.04 hrs, Volume= 53,194 cf, Depth= 0.60"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

	Ai	rea (sf)	CN	Description						
		11,944	98	98 Paved roads w/curbs & sewers						
	8	15,065	71	Meadow, no	on-grazed,	HSG C				
		80,425	78	Meadow, no	on-grazed,	HSG D				
*		30,659	98	Paved Path	IS					
		19,458	96	Gravel surf	ace, HSG (					
		92,519	74	>75% Gras	s cover, Go	bod, HSG C				
		7,358	80		,	bod, HSG D				
		5,373	82	Woods/gras	ss comb., F	Poor, HSG C				
*		1,014	100 Stream							
	1,0	63,815	73	Weighted A	verage					
		20,198		95.90% Pe						
		43,617	98	4.10% Impe	ervious Are	а				
	_									
,	Τc	Length	Slope		Capacity	Description				
(	min)	(feet)	(ft/ft	/ /	(cfs)					
	5.9	100	0.0740	0.28		Sheet Flow,				
						Range n= 0.130 P2= 2.50"				
	2.5	260	0.0600	) 1.71		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	0.5	95	0.2200	) 3.28		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	8.9	455	Total							



#### Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

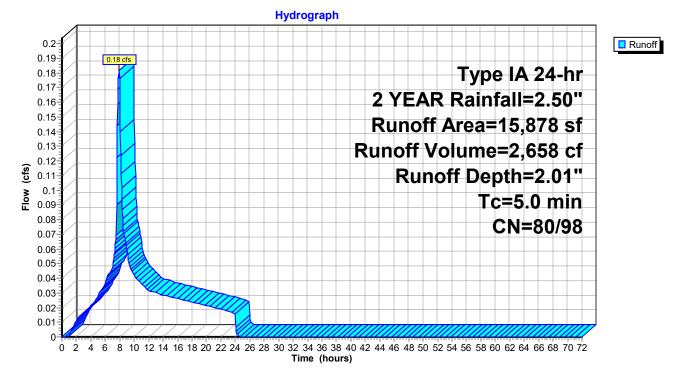
## Summary for Subcatchment 3S1: MOUNTAINVIEW

Runoff = 0.18 cfs @ 7.91 hrs, Volume= 2,658 cf, Depth= 2.01"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description			
	12,872	98	Paved road	s w/curbs &	& sewers	
	3,006	80	>75% Gras	s cover, Go	Good, HSG D	
	15,878	95	Weighted A	verage		
	3,006	80	18.93% Per	vious Area	а	
	12,872	98	81.07% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)		
5.0					Direct Entry,	

#### Subcatchment 3S1: MOUNTAINVIEW



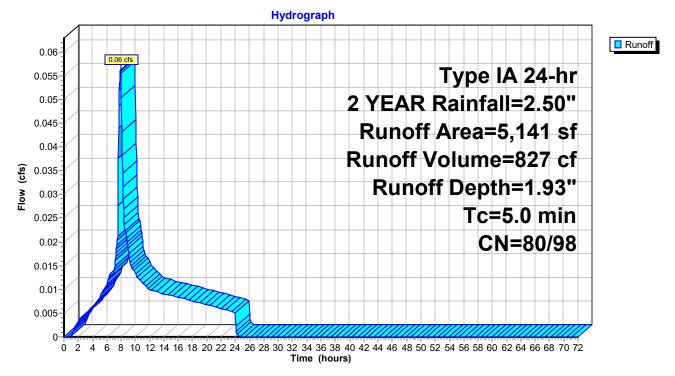
## Summary for Subcatchment 4S1: CENTER (N)

Runoff = 0.06 cfs @ 7.92 hrs, Volume= 827 cf, Depth= 1.93"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description					
	3,878	98	Paved road	s w/curbs &	& sewers			
	1,263	80	>75% Gras	s cover, Go	lood, HSG D			
	5,141	94	Weighted A	Weighted Average				
	1,263	80	24.57% Pervious Area					
	3,878	98	75.43% Impervious Area					
т	1 11.	01		0				
Tc	Length	Slop	,	Capacity	1			
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
5.0					Direct Entry,			

## Subcatchment 4S1: CENTER (N)



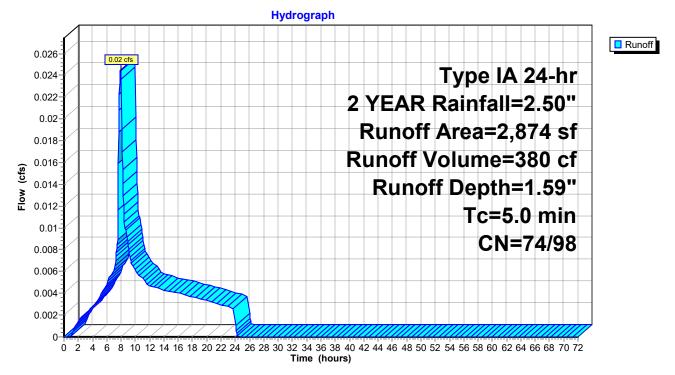
## Summary for Subcatchment 5S1: ALDERSGATE (N)

Runoff = 0.02 cfs @ 7.93 hrs, Volume= 380 cf, Depth= 1.59"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 2 YEAR Rainfall=2.50"

A	rea (sf)	CN	Description		
	1,693	98	Paved road	s w/curbs &	& sewers
	1,181	74	>75% Gras	s cover, Go	ood, HSG C
	2,874	88	Weighted A	verage	
	1,181	74	41.09% Per	vious Area	a
	1,693	98	58.91% Imp	pervious Ar	ea
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description
5.0					Direct Entry,

## Subcatchment 5S1: ALDERSGATE (N)



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#### Summary for Reach S1: VEGETATED SWALE

Inflow = 2.08 cfs @ 7.98 hrs, Volume= 111,260 cf Outflow = 2.06 cfs @ 8.04 hrs, Volume= 111,260 cf, Atten= 1%, Lag= 3.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 0.28 fps, Min. Travel Time= 7.9 min Avg. Velocity = 0.17 fps, Avg. Travel Time= 13.3 min

Peak Storage= 979 cf @ 8.04 hrs Average Depth at Peak Storage= 0.60' Bank-Full Depth= 1.50' Flow Area= 22.5 sf, Capacity= 10.83 cfs

Custom cross-section, Length= 132.0' Slope= 0.0050 '/' Constant n= 0.240 Inlet Invert= 221.00', Outlet Invert= 220.34'

‡

Offse (fee		tion eet)	Chan.Depth (feet <u>)</u>
-9.5	0 ^	1.50	0.00
-7.0	0 0	0.50	1.00
-5.0	0 0	0.00	1.50
5.0	0 0	00.0	1.50
7.0	0 0	0.50	1.00
9.5	0 1	1.50	0.00
Denth F	nd Area	P۵	rim Stor

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	10.0	0	0.00
0.50	6.0	14.1	792	1.49
1.50	22.5	19.5	2,970	10.83

**Reach S1: VEGETATED SWALE** 

## Summary for Pond 1R: WQMH/ BYPASS

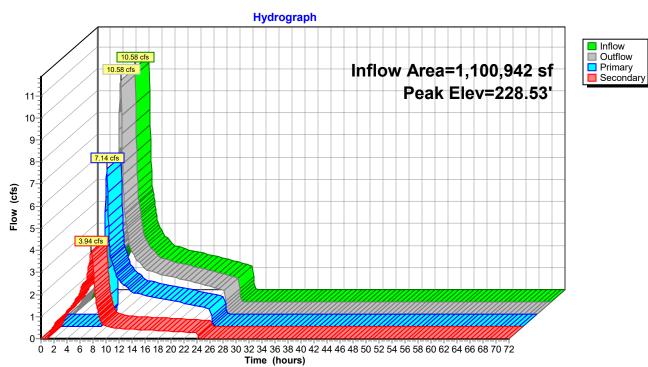
Inflow Area = 1,100,942 sf,		68.23% Impervious,	Inflow Depth = 1.78"	for 2 YEAR event
Inflow =	10.58 cfs @	7.97 hrs, Volume=	163,718 cf	
Outflow =	10.58 cfs @	7.97 hrs, Volume=	163,718 cf, Atter	n= 0%, Lag= 0.0 min
Primary =	7.14 cfs @	7.99 hrs, Volume=	94,305 cf	-
Secondary =	3.94 cfs @	7.71 hrs, Volume=	69,413 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 228.53' @ 10.00 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	227.15'	<b>36.0" Round High Flow Bypass</b> L= 50.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 227.15' / 224.50' S= 0.0530 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#2	Secondary	225.55'	12.0" Round WQ Flow
	-		L= 10.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= 225.55' / 225.35' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	225.15'	12.0" Round WQ Flow
			L= 32.5' CPP, mitered to conform to fill, Ke= $0.700$
			Inlet / Outlet Invert= 225.15' / 224.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Secondary OutFlow Max=3.81 cfs @ 7.71 hrs HW=228.01' TW=226.70' (Dynamic Tailwater) 2=WQ Flow (Controls 3.81 cfs)

**1**-3=WQ Flow (Inlet Controls 3.81 cfs @ 4.86 fps)



Pond 1R: WQMH/ BYPASS

## Summary for Pond 2R: BYPASS

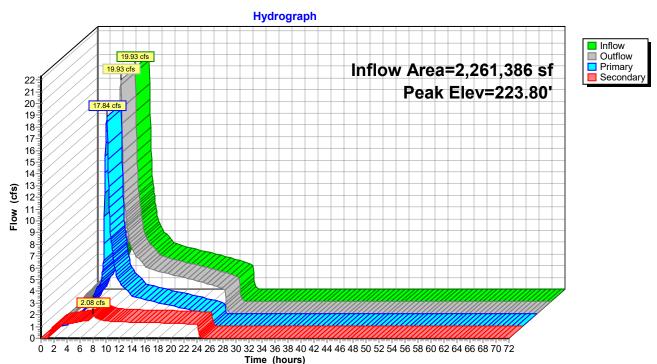
Inflow Area = 2,261,386 sf,		64.41% Impervious,	Inflow Depth = 1.69"	for 2 YEAR event
Inflow =	19.93 cfs @	7.98 hrs, Volume=	319,268 cf	
Outflow =	19.93 cfs @	7.98 hrs, Volume=	319,268 cf, Atter	n= 0%, Lag= 0.0 min
Primary =	17.84 cfs @	7.98 hrs, Volume=	208,008 cf	-
Secondary =	2.08 cfs @	7.98 hrs, Volume=	111,260 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 223.80' @ 7.98 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	221.20'	<b>8.0" Round WQ Flow</b> L= 40.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 221.20' / 221.00' S= 0.0050 '/' Cc= 0.900
#2	Primary	222.00'	n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf <b>36.0" Round High Flow Bypass</b> L= 50.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 222.00' / 221.20' S= 0.0160 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf

**Primary OutFlow** Max=17.74 cfs @ 7.98 hrs HW=223.79' TW=216.05' (Dynamic Tailwater) **2=High Flow Bypass** (Inlet Controls 17.74 cfs @ 4.02 fps)

Secondary OutFlow Max=2.08 cfs @ 7.98 hrs HW=223.79' TW=221.60' (Dynamic Tailwater) T=WQ Flow (Barrel Controls 2.08 cfs @ 5.96 fps)



## Pond 2R: BYPASS

# Summary for Pond P1: EXTENDED DRY BASIN

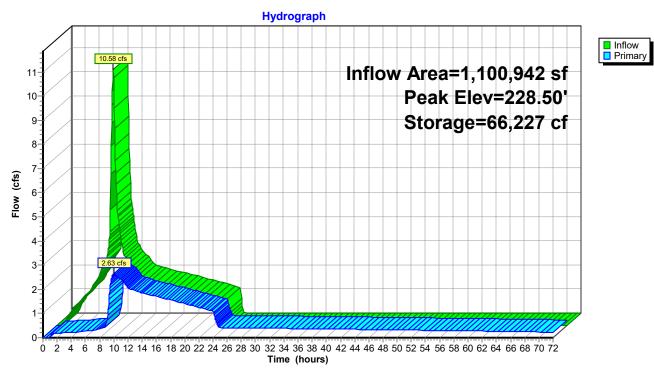
Inflow Area =1,100,942 sf, 68.23% Impervious, Inflow Depth =1.78" for 2 YEAR eventInflow =10.58 cfs @7.97 hrs, Volume=163,718 cfOutflow =2.63 cfs @10.06 hrs, Volume=156,600 cf, Atten= 75%, Lag= 125.1 minPrimary =2.63 cfs @10.06 hrs, Volume=156,600 cf							
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 228.50' @ 10.06 hrs Surf.Area= 19,932 sf Storage= 66,227 cf							
Plug-Flow detention time= 892.2 min calculated for 156,600 cf (96% of inflow) Center-of-Mass det. time= 860.7 min(1,564.2 - 703.5)							
Volume Invert Avail.Storage Storage Description							
#1 224.50' 109,826 cf Custom Stage Data (Irregular) List	ted below (Recalc)						
Elevation Surf.Area Perim. Inc.Store Cum.Store (feet) (sq-ft) (feet) (cubic-feet) (cubic-feet)	Wet.Area (sq-ft)						
224.50 13,412 505.4 0 0	13,412						
230.50 23,680 635.5 109,826 109,826	25,710						
Device Routing Invert Outlet Devices							
#1 Primary 223.40' <b>12.0" Round Outfall Pipe</b>							
	L= 50.0' CPP, square edge headwall, Ke= 0.500						
Inlet / Outlet Invert= 223.40' / 223.15' S							
n= 0.013 Corrugated PE, smooth interio	r, Flow Area= 0.79 sf						
#2 Device 1 223.50' <b>2.7" Vert. WQ Orifice</b> C= 0.600	0 0 000						
#3 Device 1 228.30' <b>30.0" x 16.0" Horiz. Upper Ditch Inlet</b> Limited to weir flow at low heads	C= 0.600						
#4 Primary 225.50' <b>12.0" Round Outfall Pipe</b>							
L= 50.0' CPP, square edge headwall, k	Ke= 0.500						
Inlet / Outlet Invert= 225.50' / 224.25' S	= 0.0250 '/' Cc= 0.900						
n= 0.013 Corrugated PE, smooth interio							
#5 Device 4 229.00' <b>30.0" x 16.0" Horiz. Upper Ditch Inlet</b>	C= 0.600						
Limited to weir flow at low heads							
<b>Primary OutFlow</b> Max=2.63 cfs @ 10.06 hrs HW=228.50' TW=0.00' (Dynamic Tailwater)							

**1=Outfall Pipe** (Passes 2.63 cfs of 7.50 cfs potential flow)

2=WQ Orifice (Orifice Controls 0.42 cfs @ 10.64 fps) 3=Upper Ditch Inlet (Weir Controls 2.21 cfs @ 1.46 fps)

**4=Outfall Pipe** (Passes 0.00 cfs of 5.98 cfs potential flow)

**5=Upper Ditch Inlet** (Controls 0.00 cfs)



# Pond P1: EXTENDED DRY BASIN

### Summary for Pond P2: DETENTION POND

Inflow Are	a =	2,261,386 sf,	64.41% Impervious,	Inflow Depth = 1.69"	for 2 YEAR event
Inflow	=	19.90 cfs @	7.98 hrs, Volume=	319,268 cf	
Outflow	=	6.67 cfs @	9.18 hrs, Volume=	270,611 cf, Atte	en= 66%, Lag= 71.5 min
Primary	=	6.67 cfs @	9.18 hrs, Volume=	270,611 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 217.30' @ 9.18 hrs Surf.Area= 27,975 sf Storage= 117,812 cf Flood Elev= 219.00' Surf.Area= 32,084 sf Storage= 168,924 cf

Plug-Flow detention time= 718.0 min calculated for 270,423 cf (85% of inflow) Center-of-Mass det. time= 616.4 min (1,328.5 - 712.1)

Volume	Inver	t Avail.St	orage	ge Storage Description						
#1	#1 212.00' 202,272 cf		272 cf	Custom Stage D	ata (Irregular) Liste	ed below (Recalc)				
Elevatio (fee		urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)				
212.0	00	16,833	666.0	0	0	16,833				
213.0	00	18,860	684.8	17,837	17,837	18,964				
220.0	00	34,628	816.8	184,435	202,272	35,589				
Device	Routing	Invert	Outl	et Devices						
#1	Primary	210.50	18.0	" Round Outfall						
	-				e edge headwall, k					
						= 0.3900 '/'     Cc= 0.900				
						r, Flow Area= 1.77 sf				
#2	Device 1	211.00		12.0" Round Detention Pipe 1						
				L= 10.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 211.00' / 210.95' S= 0.0050 '/' Cc= 0.900						
#3	Device 2	211.00			<b>Fifice 1</b> C= 0.600	; Flow Area= 0.79 sf				
#3 #4	Device 2 Device 2	211.00	-		pper Ditch Inlet 1					
<del>774</del>	Device 2	210.90		ted to weir flow at		0.000				
#5	Device 1	213.00		" Round Detentio						
110	Device 1	210.00			e edge headwall, k	(e= 0.500				
						= 0.0250 '/' Cc= 0.900				
						, Flow Area= 0.79 sf				
#6	Device 5	218.20			pper Ditch Inlet 2					
				ted to weir flow at						
				8 hrs HW=217.30	' TW=0.00' (Dyna	amic Tailwater)				

**1=Outfall** (Passes 6.67 cfs of 20.92 cfs potential flow)

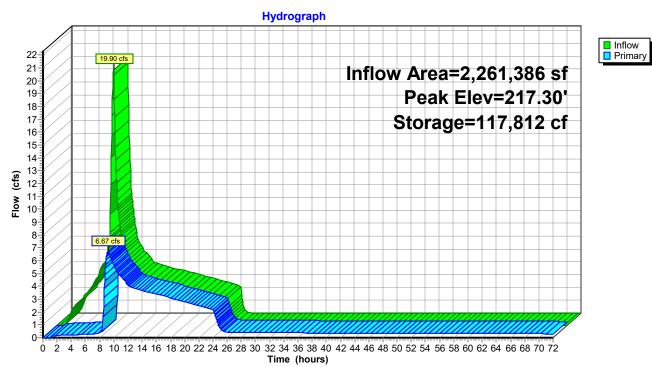
-2=Detention Pipe 1 (Passes 6.67 cfs of 9.10 cfs potential flow)

**1**-3=Detention Orifice 1 (Orifice Controls 0.41 cfs @ 11.98 fps)

4=Upper Ditch Inlet 1 (Weir Controls 6.26 cfs @ 2.06 fps)

-5=Detention Pipe 2 (Passes 0.00 cfs of 7.37 cfs potential flow)

6=Upper Ditch Inlet 2 (Controls 0.00 cfs)



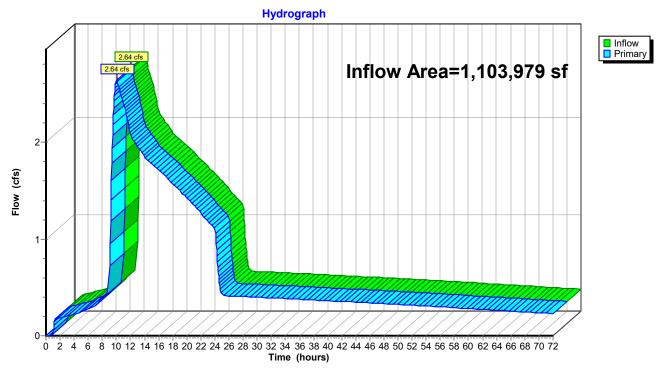
# Pond P2: DETENTION POND

**4487-01 Springbrook - Post** *Type* Prepared by AKS ENGINEERING & FORESTRY, LLC HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solutions LLC

# Summary for Link CS: CENTER ST (S)

Inflow Are	a =	1,103,979 sf,	68.28% Impervious,	Inflow Depth > 1.71"	for 2 YEAR event
Inflow	=	2.64 cfs @	10.06 hrs, Volume=	157,136 cf	
Primary	=	2.64 cfs @	10.06 hrs, Volume=	157,136 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

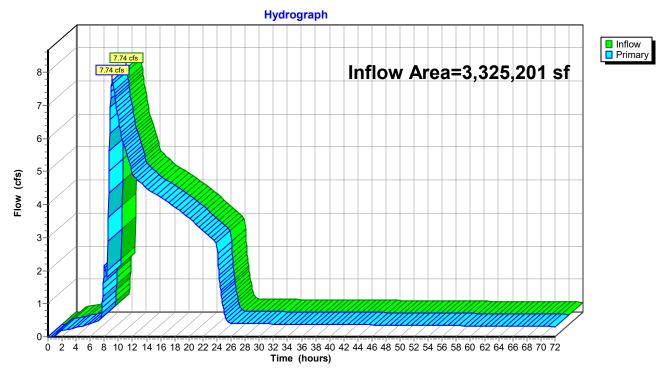


# Link CS: CENTER ST (S)

# Summary for Link HC: HESS CREEK

Inflow Area	a =	3,325,201 sf,	45.12% Impervious,	Inflow Depth > 1.1	7" for 2 YEAR event
Inflow	=	7.74 cfs @	9.15 hrs, Volume=	323,805 cf	
Primary	=	7.74 cfs @	9.15 hrs, Volume=	323,805 cf, A	tten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



# Link HC: HESS CREEK

<b>4487-01 Springbrook - Post</b> Prepared by AKS ENGINEERING & FOR HydroCAD® 10.00-22 s/n 01338 © 2018 Hydro	
Runoff by SBUI	2.00 hrs, dt=0.05 hrs, 1441 points I method, Split Pervious/Imperv. method - Pond routing by Dyn-Stor-Ind method
Subcatchment 1S1: MOUNTAINVIEW	Runoff Area=3,037 sf 88.90% Impervious Runoff Depth=3.09" Tc=5.0 min CN=80/98 Runoff=0.05 cfs 781 cf
Subcatchment 1S2: WEST HALF/ Ru	noff Area=1,100,942 sf 68.23% Impervious Runoff Depth=2.68" Tc=8.0 min CN=77/98 Runoff=16.03 cfs 246,165 cf
Subcatchment 2S1: EAST HALF/ Ru	noff Area=2,261,386 sf 64.41% Impervious Runoff Depth=2.57" Tc=10.0 min CN=75/98 Runoff=30.55 cfs 483,813 cf
	unoff Area=1,063,815 sf 4.10% Impervious Runoff Depth=1.21" ength=455' Tc=8.9 min CN=72/98 Runoff=5.31 cfs 107,169 cf
Subcatchment 3S1: MOUNTAINVIEW	Runoff Area=15,878 sf 81.07% Impervious Runoff Depth=2.96" Tc=5.0 min CN=80/98 Runoff=0.27 cfs 3,914 cf
Subcatchment 4S1: CENTER (N)	Runoff Area=5,141 sf 75.43% Impervious Runoff Depth=2.87" Tc=5.0 min CN=80/98 Runoff=0.08 cfs 1,228 cf
Subcatchment 5S1: ALDERSGATE (N)	Runoff Area=2,874 sf 58.91% Impervious Runoff Depth=2.43" Tc=5.0 min CN=74/98 Runoff=0.04 cfs 583 cf
	Flow Depth=0.64' Max Vel=0.29 fps Inflow=2.36 cfs 124,582 cf ' S=0.0050 '/' Capacity=10.83 cfs Outflow=2.33 cfs 124,582 cf
Pond 1R: WQMH/ BYPASS Primary=13.58 cfs 159,311	Peak Elev=229.07' Inflow=16.03 cfs 246,165 cf cf Secondary=3.25 cfs 86,853 cf Outflow=16.03 cfs 246,165 cf
Pond 2R: BYPASS Primary=28.19 cfs 359,231 c	Peak Elev=224.40' Inflow=30.55 cfs 483,813 cf 5 Secondary=2.36 cfs 124,582 cf Outflow=30.55 cfs 483,813 cf
Pond P1: EXTENDED DRY BASIN Pe	ak Elev=228.90' Storage=74,398 cf Inflow=16.03 cfs 246,165 cf Outflow=7.84 cfs 238,907 cf
Pond P2: DETENTION POND Pea	k Elev=218.44' Storage=151,284 cf Inflow=30.51 cfs 483,813 cf Outflow=12.87 cfs 434,979 cf
Link CS: CENTER ST (S)	Inflow=7.86 cfs 239,688 cf Primary=7.86 cfs 239,688 cf
Link HC: HESS CREEK	Inflow=15.72 cfs 542,148 cf Primary=15.72 cfs 542,148 cf
	Runoff Volume = 843,652 cf Average Runoff Depth = 2.27" Pervious = 2.180,498 sf 51,03% Impervious = 2.272,575 sf

48.97% Pervious = 2,180,498 sf 51.03% Impervious = 2,272,575 sf

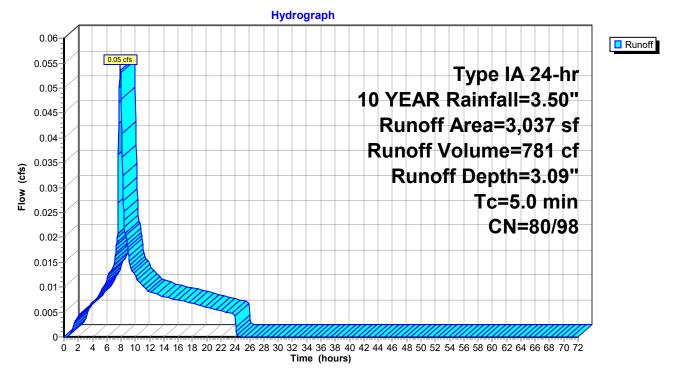
# Summary for Subcatchment 1S1: MOUNTAINVIEW

Runoff = 0.05 cfs @ 7.90 hrs, Volume= 781 cf, Depth= 3.09"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

A	rea (sf)	CN	Description				
	2,700	98	Paved road	s w/curbs &	& sewers		
	337	80	>75% Gras	s cover, Go	lood, HSG D		
	3,037	96	Weighted A	Weighted Average			
	337	80	11.10% Pervious Area				
	2,700	98	88.90% Impervious Area				
Тс	Length	Slop		Capacity			
(min)	(feet)	(ft/ft	:) (ft/sec)	(cfs)			
5.0					Direct Entry,		

### Subcatchment 1S1: MOUNTAINVIEW



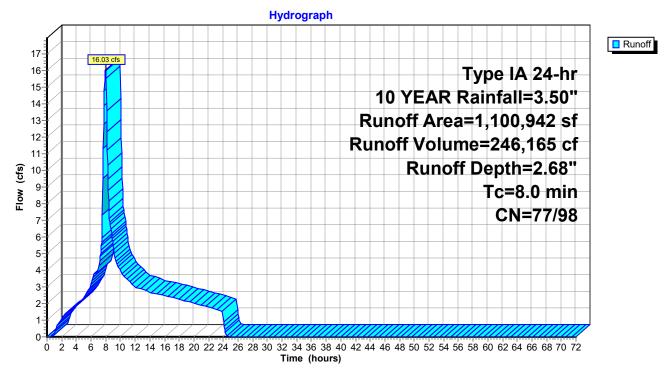
### Summary for Subcatchment 1S2: WEST HALF/ MOUNTAINVIEW

Runoff = 16.03 cfs @ 7.97 hrs, Volume= 246,165 cf, Depth= 2.68"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

_	Area (sf)	CN	Description					
*	336,878	98	Right-of-Way Impervious area					
*	345,240	98	120 Lots >2877 sf (2877sf/Lot)					
*	57,717	98	Cluster Lots (90% of total area)					
*	11,309	98	Open Space Impervious					
	170,116	74	>75% Grass cover, Good, HSG C					
_	179,682	80	>75% Grass cover, Good, HSG D					
	1,100,942	91	Weighted Average					
	349,798	77	31.77% Pervious Area					
	751,144	98	68.23% Impervious Area					
	Tc Length (min) (feet)	Sloj (ft/						
	8.0		Direct Entry,					





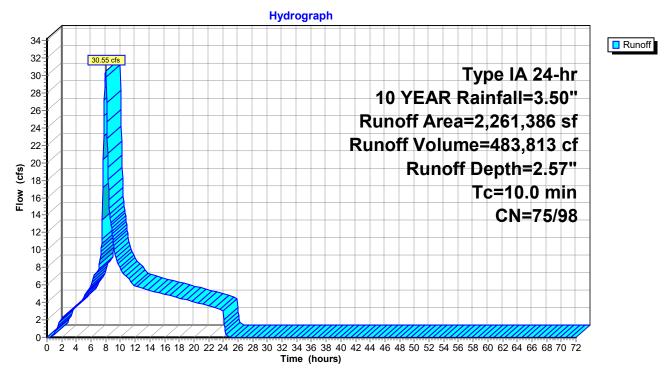
### Summary for Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

Runoff = 30.55 cfs @ 7.98 hrs, Volume= 483,813 cf, Depth= 2.57"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

	Area (sf)	CN	Description						
*	650,436	98	Right-of-Way Impervious area						
*	716,373	98	249 Lots >2877 sf (2877sf/Lot)						
*	47,186	98	Cluster Lots (90% of total area)						
*	42,676	98	Open Space Impervious						
	547,140	74	>75% Grass cover, Good, HSG C						
	195,956	80	>75% Grass cover, Good, HSG D						
*	14,344	86	Playground surfacing						
	47,275	71	Meadow, non-grazed, HSG C						
	2,261,386	90	Weighted Average						
	804,715	75	35.59% Pervious Area						
	1,456,671	98	64.41% Impervious Area						
	Tc Length	Slo	e Velocity Capacity Description						
	(min) (feet)	(ft/	/ft) (ft/sec) (cfs)						
	10.0		Direct Entry,						

### Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

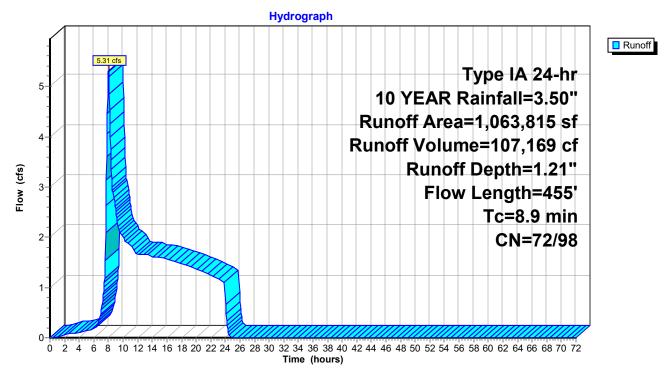


### Summary for Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

Runoff = 5.31 cfs @ 8.01 hrs, Volume= 107,169 cf, Depth= 1.21"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

	Ai	rea (sf)	CN	Description							
		11,944	98	Paved roads w/curbs & sewers							
	8	15,065	71	Meadow, no	<i>l</i> eadow, non-grazed, HSG C						
		80,425	78	Meadow, no	on-grazed,	HSG D					
*		30,659	98	Paved Path	IS						
		19,458	96	Gravel surfa	ace, HSG (						
		92,519	74	>75% Gras	s cover, Go	bod, HSG C					
		7,358	80	>75% Gras	s cover, Go	bod, HSG D					
		5,373	82	Woods/gras	ss comb., F	Poor, HSG C					
*		1,014	100	100 Stream							
	1,0	63,815	73	Weighted A	verage						
	1,0	20,198	72	95.90% Pei	vious Area						
		43,617	98	4.10% Impe	ervious Area	а					
	Тс	Length	Slope		Capacity	Description					
(	(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)						
	5.9	100	0.0740	0.28		Sheet Flow,					
						Range n= 0.130 P2= 2.50"					
	2.5	260	0.0600	) 1.71		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	0.5	95	0.2200	) 3.28		Shallow Concentrated Flow,					
						Short Grass Pasture Kv= 7.0 fps					
	8.9	455	Total								



### Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

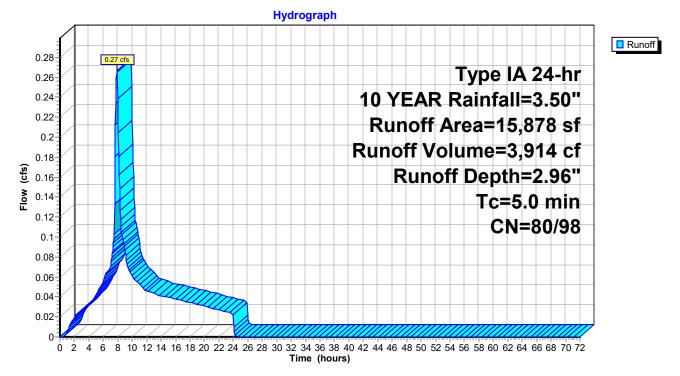
### Summary for Subcatchment 3S1: MOUNTAINVIEW

Runoff = 0.27 cfs @ 7.91 hrs, Volume= 3,914 cf, Depth= 2.96"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

Α	rea (sf)	CN	Description			
	12,872	98	Paved road	s w/curbs &	& sewers	
	3,006	80	>75% Grass	s cover, Go	ood, HSG D	
	15,878	95	Weighted Average			
	3,006	80	18.93% Pervious Area			
	12,872	98	81.07% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description	
5.0					Direct Entry,	

### Subcatchment 3S1: MOUNTAINVIEW



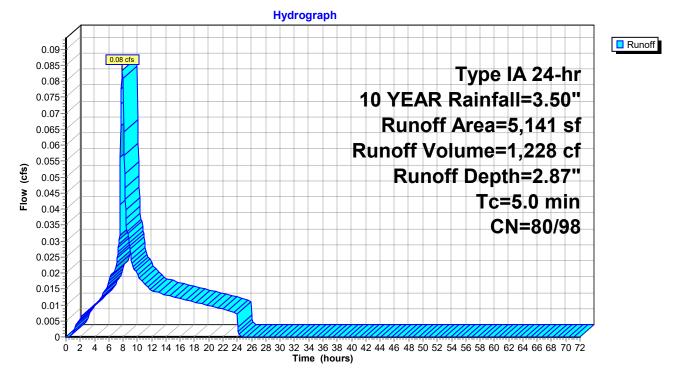
### Summary for Subcatchment 4S1: CENTER (N)

Runoff = 0.08 cfs @ 7.91 hrs, Volume= 1,228 cf, Depth= 2.87"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

A	rea (sf)	CN	Description						
	3,878	98	Paved road	s w/curbs &	& sewers				
	1,263	80	>75% Gras	s cover, Go	bod, HSG D				
	5,141	94	Weighted A	Weighted Average					
	1,263	80	24.57% Per	24.57% Pervious Area					
	3,878	98	75.43% Imp	pervious Are	ea				
Тс	Length	Slop	e Velocity	Capacity	Description				
(min)	(feet)	(ft/f	,	(cfs)	Description				
5.0	(1001)	(101	(10300)	(013)	Direct Entry				
5.0					Direct Entry,				

### Subcatchment 4S1: CENTER (N)



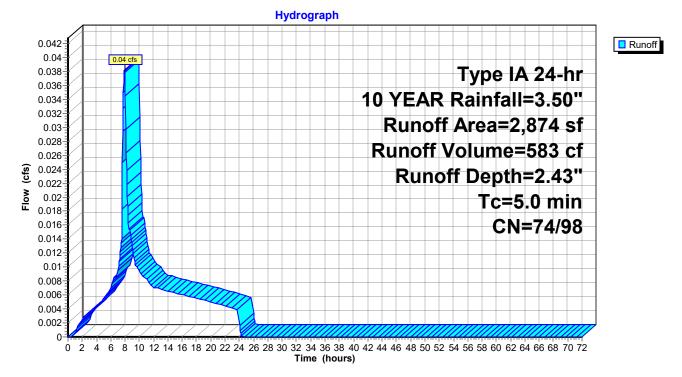
### Summary for Subcatchment 5S1: ALDERSGATE (N)

Runoff = 0.04 cfs @ 7.93 hrs, Volume= 583 cf, Depth= 2.43"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 10 YEAR Rainfall=3.50"

A	rea (sf)	CN	Description		
	1,693	98	Paved road	s w/curbs &	& sewers
	1,181	74	>75% Gras	s cover, Go	bod, HSG C
	2,874	88	Weighted A	verage	
	1,181	74	41.09% Per	vious Area	l de la constante de
	1,693	98	58.91% Imp	pervious Are	ea
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment 5S1: ALDERSGATE (N)



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#### Summary for Reach S1: VEGETATED SWALE

Inflow = 2.36 cfs @ 7.98 hrs, Volume= 124,582 cf Outflow = 2.33 cfs @ 8.04 hrs, Volume= 124,582 cf, Atten= 1%, Lag= 3.3 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 0.29 fps, Min. Travel Time= 7.6 min Avg. Velocity = 0.17 fps, Avg. Travel Time= 12.8 min

Peak Storage= 1,061 cf @ 8.04 hrs Average Depth at Peak Storage= 0.64' Bank-Full Depth= 1.50' Flow Area= 22.5 sf, Capacity= 10.83 cfs

Custom cross-section, Length= 132.0' Slope= 0.0050 '/' Constant n= 0.240 Inlet Invert= 221.00', Outlet Invert= 220.34'

‡

	044		Chan Danth	
	Offset	Elevation	Chan.Depth	
	(feet)	(feet)	(feet)	
	-9.50	1.50	0.00	
	-7.00	0.50	1.00	
	-5.00	0.00	1.50	
	5.00	0.00	1.50	
	7.00	0.50	1.00	
	9.50	1.50	0.00	
	epth End			Storag
(f	eet)	(sa-ft) (f	feet) (cub	hic-feet

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	10.0	0	0.00
0.50	6.0	14.1	792	1.49
1.50	22.5	19.5	2,970	10.83

### **Reach S1: VEGETATED SWALE**

# Summary for Pond 1R: WQMH/ BYPASS

Inflow Area =	1,100,942 sf,	68.23% Impervious,	Inflow Depth = 2.68" for 10 YEAR event	
Inflow =	16.03 cfs @	7.97 hrs, Volume=	246,165 cf	
Outflow =	16.03 cfs @	7.97 hrs, Volume=	246,165 cf, Atten= 0%, Lag= 0.0 min	I
Primary =	13.58 cfs @	7.98 hrs, Volume=	159,311 cf	
Secondary =	3.25 cfs @	6.17 hrs, Volume=	86,853 cf	

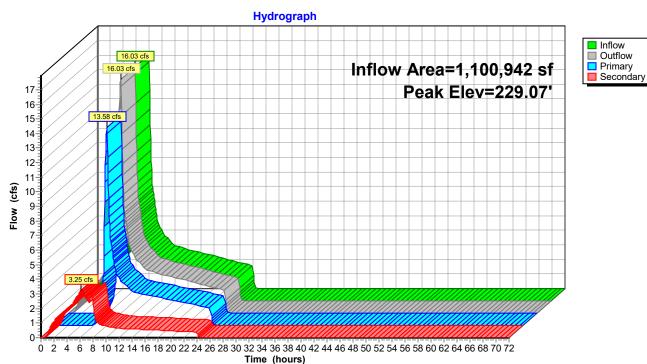
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 229.07' @ 8.05 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	227.15'	<b>36.0" Round High Flow Bypass</b> L= 50.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 227.15' / 224.50' S= 0.0530 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#2	Secondary	225.55'	<b>12.0" Round WQ Flow</b> L= 10.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 225.55' / 225.35' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	225.15'	<b>12.0" Round WQ Flow</b> L= 32.5' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 225.15' / 224.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=12.02 cfs @ 7.98 hrs HW=229.01' TW=228.57' (Dynamic Tailwater) ☐ 1=High Flow Bypass (Outlet Controls 12.02 cfs @ 3.72 fps)

Secondary OutFlow Max=3.19 cfs @ 6.17 hrs HW=227.33' TW=226.42' (Dynamic Tailwater) 2=WQ Flow (Controls 3.19 cfs)

**1**-3=WQ Flow (Inlet Controls 3.19 cfs @ 4.06 fps)



### Pond 1R: WQMH/ BYPASS

### Summary for Pond 2R: BYPASS

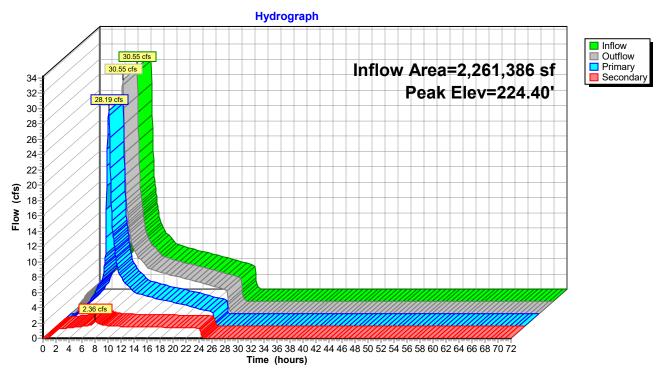
Inflow Area =	2,261,386 sf,	64.41% Impervious,	Inflow Depth = 2.57" for 10 YEAR event
Inflow =	30.55 cfs @	7.98 hrs, Volume=	483,813 cf
Outflow =	30.55 cfs @	7.98 hrs, Volume=	483,813 cf, Atten= 0%, Lag= 0.0 min
Primary =	28.19 cfs @	7.98 hrs, Volume=	359,231 cf
Secondary =	2.36 cfs @	7.98 hrs, Volume=	124,582 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 224.40' @ 7.98 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	221.20'	<b>8.0" Round WQ Flow</b> L= 40.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 221.20' / 221.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Primary	222.00'	<b>36.0" Round High Flow Bypass</b> L= 50.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 222.00' / 221.20' S= 0.0160 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf

**Primary OutFlow** Max=28.03 cfs @ 7.98 hrs HW=224.39' TW=217.67' (Dynamic Tailwater) **2=High Flow Bypass** (Inlet Controls 28.03 cfs @ 4.64 fps)

Secondary OutFlow Max=2.35 cfs @ 7.98 hrs HW=224.39' TW=221.64' (Dynamic Tailwater) T=WQ Flow (Barrel Controls 2.35 cfs @ 6.74 fps)



# Pond 2R: BYPASS

# Summary for Pond P1: EXTENDED DRY BASIN

Inflow Outflow	Inflow Area =       1,100,942 sf, 68.23% Impervious, Inflow Depth = 2.68" for 10 YEAR event         Inflow =       16.03 cfs @       7.97 hrs, Volume=       246,165 cf         Outflow =       7.84 cfs @       8.43 hrs, Volume=       238,907 cf, Atten= 51%, Lag= 27.4 min         Primary =       7.84 cfs @       8.43 hrs, Volume=       238,907 cf						
				Span= 0.00-72.00 h rea= 20,660 sf Sto			
Center-o	of-Mass det	. time= 587.5	min ( 1	,283.0 - 695.5 )	1 cf (97% of inflow)		
Volume				Storage Description			
#1	224.50	' 109,8	826 cf	Custom Stage Da	<b>ata (Irregular)</b> Listed	below (Recalc)	
Elevatio (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
224.5	50	13,412	505.4	0	0	13,412	
230.5	50	23,680	635.5	109,826	109,826	25,710	
Device	Routing	Inver		et Devices			
#1	Primary	223.40		" Round Outfall P	•		
					edge headwall, Ke=		
					223.15 S= 0 E, smooth interior, 1	0.0050 '/' Cc= 0.900	
#2	Device 1	223.50		Vert. WQ Orifice		10W Alea - 0.79 31	
#3	Device 1	228.30		-	per Ditch Inlet C=	0.600	
				ted to weir flow at I			
#4	Primary	225.50		" Round Outfall P			
					edge headwall, Ke=		
					224.25 S= 0 E, smooth interior, 1	0.0250 '/' Cc= 0.900 Flow Area= 0.79 sf	
#5	Device 4	229.00			per Ditch Inlet C=		
		•		ted to weir flow at l	•		

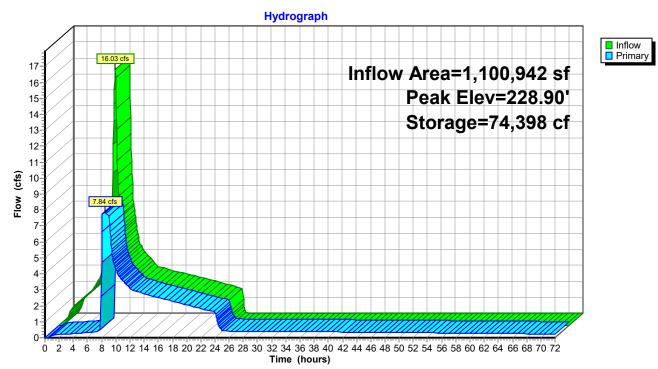
Primary OutFlow Max=7.84 cfs @ 8.43 hrs HW=228.90' TW=0.00' (Dynamic Tailwater)

-1=Outfall Pipe (Barrel Controls 7.84 cfs @ 9.98 fps)

**2=WQ Orifice** (Passes < 0.44 cfs potential flow) **3=Upper Ditch Inlet** (Passes < 11.66 cfs potential flow)

**4=Outfall Pipe** (Passes 0.00 cfs of 6.44 cfs potential flow)

**5=Upper Ditch Inlet** (Controls 0.00 cfs)



### Pond P1: EXTENDED DRY BASIN

### Summary for Pond P2: DETENTION POND

Inflow Area	a =	2,261,386 sf,	64.41% Impervious,	Inflow Depth = 2.57"	for 10 YEAR event
Inflow	=	30.51 cfs @	7.98 hrs, Volume=	483,813 cf	
Outflow	=	12.87 cfs @	8.75 hrs, Volume=	434,979 cf, Atte	n= 58%, Lag= 46.0 min
Primary	=	12.87 cfs @	8.75 hrs, Volume=	434,979 cf	

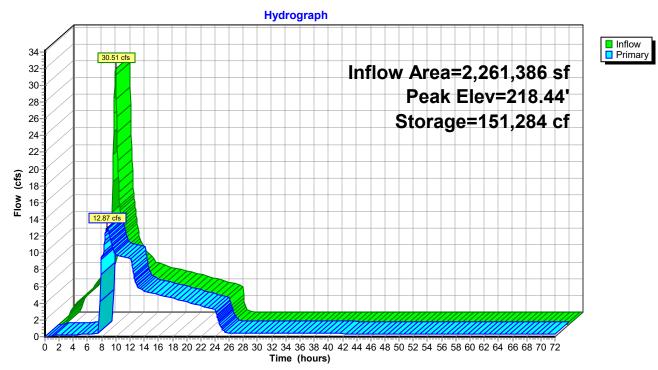
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 218.44' @ 8.75 hrs Surf.Area= 30,698 sf Storage= 151,284 cf Flood Elev= 219.00' Surf.Area= 32,084 sf Storage= 168,924 cf

Plug-Flow detention time= 485.3 min calculated for 434,677 cf (90% of inflow) Center-of-Mass det. time= 416.2 min (1,120.2 - 704.1)

Volume	Invert	Avail.St	orage	Storage Description	n			
#1	212.00	202,	272 cf	Custom Stage Da	<b>ta (Irregular)</b> Listed	below (Recalc)		
Elevatio (fee		urf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
212.0	)0	16,833	666.0	0	0	16,833		
213.0		18,860	684.8	17,837	17,837	18,964		
220.0	00	34,628	816.8	184,435	202,272	35,589		
Device	Routing	Inver	t Outle	et Devices				
#1	Primary	210.50	' 18.0	" Round Outfall				
#2	Device 1	211.00	Inlet n= 0 ' <b>12.0</b> L= 1 Inlet	/ Outlet Invert= 210 .013 Corrugated P <b>" Round Detentior</b> 0.0' CMP, square / Outlet Invert= 211	E, smooth interior, 1 1 <b>Pipe 1</b> edge headwall, Ke=	.3900 '/' Cc= 0.900 Flow Area= 1.77 sf = 0.500 .0050 '/' Cc= 0.900		
#3	Device 2	211.00		Vert. Detention Or				
#4	Device 2	216.90			per Ditch Inlet 1 C	C= 0.600		
#5 #6	Device 1 Device 5	213.00 218.20	' <b>12.0</b> L= 3 Inlet n= 0 ' <b>30.0</b>	Limited to weir flow at low heads <b>12.0" Round Detention Pipe 2</b> L= 30.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 213.00' / 212.25' S= 0.0250 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf <b>30.0" x 16.0" Horiz. Upper Ditch Inlet 2</b> C= 0.600 Limited to weir flow at low heads				
Primary OutFlow Max=12.87 cfs @ 8.75 hrs HW=218.44' TW=0.00' (Dynamic Tailwater) 1=Outfall (Passes 12.87 cfs of 22.81 cfs potential flow) 2=Detention Pipe 1 (Inlet Controls 9.96 cfs @ 12.68 fps) 3=Detention Orifice 1 (Passes < 0.44 cfs potential flow) 4=Upper Ditch Inlet 1 (Passes < 19.90 cfs potential flow) 5=Detention Pipe 2 (Passes 2.91 cfs of 8.40 cfs potential flow)								

**5=Detention Pipe 2** (Passes 2.91 cfs of 8.40 cfs potential flow)

-6=Upper Ditch Inlet 2 (Weir Controls 2.91 cfs @ 1.60 fps)

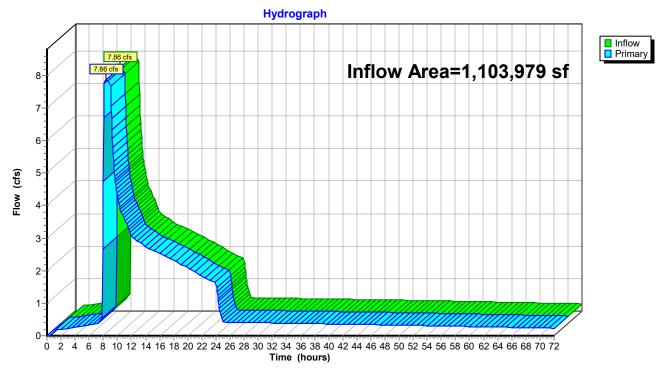


# Pond P2: DETENTION POND

# Summary for Link CS: CENTER ST (S)

Inflow Area =	1,103,979 sf,	68.28% Impervious,	Inflow Depth > 2.61"	for 10 YEAR event
Inflow =	7.86 cfs @	8.41 hrs, Volume=	239,688 cf	
Primary =	7.86 cfs @	8.41 hrs, Volume=	239,688 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

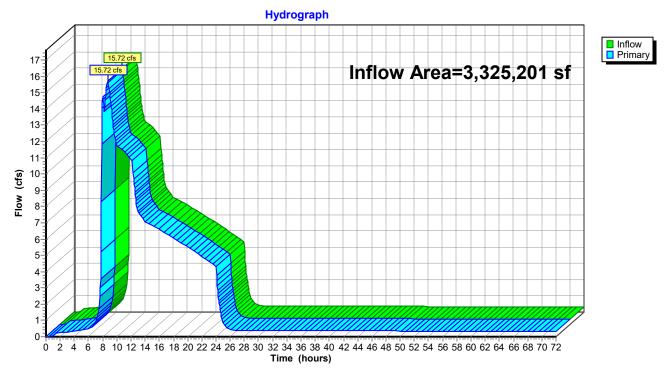


# Link CS: CENTER ST (S)

# Summary for Link HC: HESS CREEK

Inflow Area	a =	3,325,201 sf,	45.12% Impervious,	Inflow Depth > 1.96"	for 10 YEAR event
Inflow	=	15.72 cfs @	8.70 hrs, Volume=	542,148 cf	
Primary	=	15.72 cfs @	8.70 hrs, Volume=	542,148 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



# Link HC: HESS CREEK

4487-01 Springbrook - PostType IA 24-hr25 YEAR Rainfall=4.00"Prepared by AKS ENGINEERING & FORESTRY, LLCPrinted10/7/2022HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solutions LLCPage 62									
Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SBUH method, Split Pervious/Imperv. Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method									
Subcatchment 1S1: MOUNTAINVIEWRunoff Area=3,037 sf88.90% ImperviousRunoff Depth=3.57"Tc=5.0 minCN=80/98Runoff=0.06 cfs904 cf									
Subcatchment 1S2: WEST HALF/Runoff Area=1,100,942 sf68.23% ImperviousRunoff Depth=3.14"Tc=8.0 minCN=77/98Runoff=18.83 cfs288,496 cf									
Subcatchment 2S1: EAST HALF/Runoff Area=2,261,386 sf64.41% ImperviousRunoff Depth=3.02"Tc=10.0 minCN=75/98Runoff=36.07 cfs568,809 cf									
Subcatchment 2S2: HESS OPEN SPACE/ Runoff Area=1,063,815 sf 4.10% Impervious Runoff Depth=1.55" Flow Length=455' Tc=8.9 min CN=72/98 Runoff=7.39 cfs 137,815 cf									
Subcatchment 3S1: MOUNTAINVIEWRunoff Area=15,878 sf81.07% ImperviousRunoff Depth=3.44"Tc=5.0 minCN=80/98Runoff=0.31 cfs4,550 cf									
Subcatchment 4S1: CENTER (N)Runoff Area=5,141 sf75.43% ImperviousRunoff Depth=3.34"Tc=5.0 minCN=80/98Runoff=0.10 cfs1,432 cf									
Subcatchment 5S1: ALDERSGATE (N)Runoff Area=2,874 sf58.91% ImperviousRunoff Depth=2.87"Tc=5.0 minCN=74/98Runoff=0.05 cfs688 cf									
Reach S1: VEGETATED SWALE         Avg. Flow Depth=0.66'         Max Vel=0.30 fps         Inflow=2.50 cfs         130,073 cf           n=0.240         L=132.0'         S=0.0050 '/'         Capacity=10.83 cfs         Outflow=2.47 cfs         130,073 cf									
Pond 1R: WQMH/ BYPASS         Peak Elev=229.45'         Inflow=18.83 cfs         288,496 cf           Primary=16.53 cfs         195,335 cf         Secondary=3.25 cfs         93,159 cf         Outflow=18.83 cfs         288,494 cf									
Pond 2R: BYPASS         Peak Elev=224.73'         Inflow=36.07 cfs         568,809 cf           Primary=33.57 cfs         438,736 cf         Secondary=2.50 cfs         130,073 cf         Outflow=36.07 cfs         568,809 cf									
Pond P1: EXTENDED DRY BASINPeak Elev=229.25' Storage=81,794 cfInflow=18.83 cfs288,494 cfOutflow=11.32 cfs281,188 cf									
Pond P2: DETENTION PONDPeak Elev=218.81' Storage=162,868 cfInflow=36.02 cfs568,809 cfOutflow=18.94 cfs519,912 cf									
Link CS: CENTER ST (S) Inflow=11.35 cfs 282,093 cf Primary=11.35 cfs 282,093 cf									
Link HC: HESS CREEK Inflow=24.60 cfs 657,727 cf Primary=24.60 cfs 657,727 cf									
Total Runoff Area = 4,453,073 sf Runoff Volume = 1,002,695 cf Average Runoff Depth = 2.70"									

48.97% Pervious = 2,180,498 sf 51.03% Impervious = 2,272,575 sf

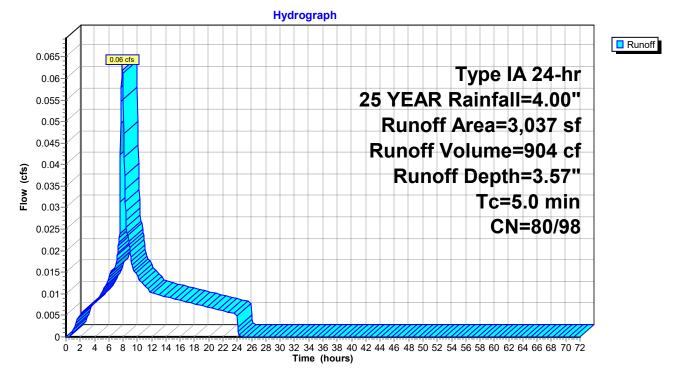
# Summary for Subcatchment 1S1: MOUNTAINVIEW

Runoff = 0.06 cfs @ 7.90 hrs, Volume= 904 cf, Depth= 3.57"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

rea (sf)	CN	Description			
2,700	98	Paved road	s w/curbs &	& sewers	
337	80	>75% Gras	s cover, Go	ood, HSG D	
3,037	96	Weighted A	verage		
337	80	11.10% Pervious Area			
2,700	98	88.90% Impervious Area			
	<u>.</u> .		<b>•</b> •		
0		,		Description	
(feet)	(ft/f	t) (ft/sec)	(cfs)		
				Direct Entry,	
	2,700 337 3,037 337	2,700 98 337 80 3,037 96 337 80 2,700 98 Length Slop	2,700         98         Paved road           337         80         >75% Grass           3,037         96         Weighted A           337         80         11.10% Per           2,700         98         88.90% Imp           Length         Slope         Velocity	2,70098Paved roads w/curbs33780>75% Grass cover, G3,03796Weighted Average3378011.10% Pervious Area2,7009888.90% Impervious ALengthSlopeVelocityCapacity	

### Subcatchment 1S1: MOUNTAINVIEW



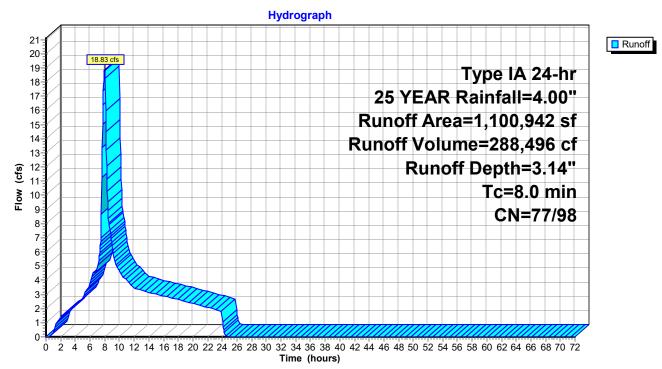
### Summary for Subcatchment 1S2: WEST HALF/ MOUNTAINVIEW

Runoff = 18.83 cfs @ 7.97 hrs, Volume= 288,496 cf, Depth= 3.14"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

_	Area (sf)	CN	Description
*	336,878	98	Right-of-Way Impervious area
*	345,240	98	120 Lots >2877 sf (2877sf/Lot)
*	57,717	98	Cluster Lots (90% of total area)
*	11,309	98	Open Space Impervious
	170,116	74	>75% Grass cover, Good, HSG C
	179,682	80	>75% Grass cover, Good, HSG D
	1,100,942	91	Weighted Average
	349,798	77	31.77% Pervious Area
	751,144	98	68.23% Impervious Area
	Tc Length (min) (feet)	Slop (ft/	
	8.0		Direct Entry,





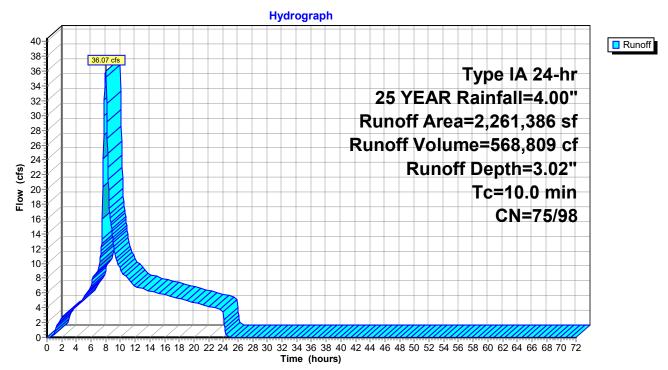
### Summary for Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

Runoff = 36.07 cfs @ 7.98 hrs, Volume= 568,809 cf, Depth= 3.02"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

	A	rea (sf)	CN	Description					
*	6	50,436	98	Right-of-Wa	ay Impervio	ous area			
*	7	16,373	98	249 Lots >2	877 sf (287	377sf/Lot)			
*		47,186	98	Cluster Lots	s (90% of to	otal area)			
*		42,676	98	Open Space	e Împerviou	us			
	5	47,140	74	>75% Gras	s cover, Go	ood, HSG C			
	1	95,956	80	>75% Gras	s cover, Go	ood, HSG D			
*		14,344	86	Playground	surfacing				
		47,275	71	71 Meadow, non-grazed, HSG C					
	2,2	61,386	90	Weighted A	verage				
	8	04,715	75	35.59% Per	vious Area	a			
	1,4	56,671	98	64.41% Imp	ervious Ar	rea			
				•					
	Tc	Length	Slop	e Velocity	Capacity	Description			
_	(min)	(feet)	(ft/f	ft) (ft/sec)	(cfs)	· · · · · · · · · · · · · · · · · · ·			
	10.0					Direct Entry,			
						**			

### Subcatchment 2S1: EAST HALF/ MOUNTAINVIEW

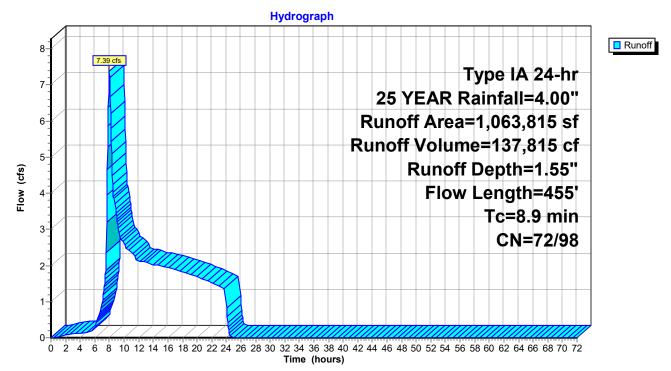


### Summary for Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

Runoff = 7.39 cfs @ 8.00 hrs, Volume= 137,815 cf, Depth= 1.55"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

	Ai	rea (sf)	CN	Description						
		11,944	98	98 Paved roads w/curbs & sewers						
	8	15,065	71	Meadow, no	on-grazed,	HSG C				
		80,425	78	Meadow, no	on-grazed,	HSG D				
*		30,659	98	Paved Path	S					
		19,458	96	Gravel surfa	ace, HSG (					
		92,519	74	>75% Gras	s cover, Go	bod, HSG C				
		7,358	80	>75% Gras	s cover, Go	bod, HSG D				
		5,373	82	Woods/gras	ss comb., F	Poor, HSG C				
*		1,014	100	Stream						
	1,063,815 73 Weighted Average				verage					
	1,020,198 72 95.90% Pervious Are			95.90% Per	vious Area					
		43,617	,617 98 4.10% Impervious Area			а				
	Тс	Length	Slope		Capacity	Description				
(	(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
	5.9	100	0.0740	0.28		Sheet Flow,				
						Range n= 0.130 P2= 2.50"				
	2.5 260 0.0600 1.71			Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps				
	0.5	95	0.2200	) 3.28		Shallow Concentrated Flow,				
						Short Grass Pasture Kv= 7.0 fps				
	8.9	455	Total							



### Subcatchment 2S2: HESS OPEN SPACE/ MOUNTAINVIEW

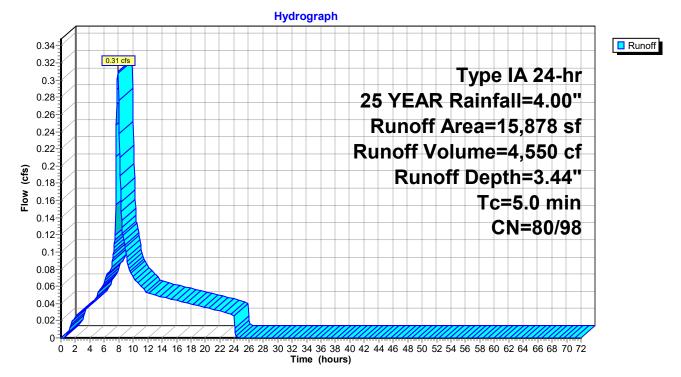
### Summary for Subcatchment 3S1: MOUNTAINVIEW

Runoff = 0.31 cfs @ 7.91 hrs, Volume= 4,550 cf, Depth= 3.44"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

ea (sf)	CN	Description		
2,872	98	Paved road	s w/curbs &	& sewers
3,006	80	>75% Grass	s cover, Go	Good, HSG D
5,878	95	Weighted A	verage	
3,006	80	18.93% Per	vious Area	а
2,872	98	81.07% Imp	ervious Are	rea
Length (feet)			Capacity (cfs)	
				Direct Entry,
	2,872 3,006 5,878 3,006 2,872 _ength	2,872 98 3,006 80 5,878 95 3,006 80 2,872 98 _ength Slope	2,872 98 Paved road 3,006 80 >75% Grass 5,878 95 Weighted A 3,006 80 18.93% Per 2,872 98 81.07% Imp _ength Slope Velocity	2,872 98 Paved roads w/curbs 3,006 80 >75% Grass cover, G 5,878 95 Weighted Average 3,006 80 18.93% Pervious Are 2,872 98 81.07% Impervious A _ength Slope Velocity Capacity

### Subcatchment 3S1: MOUNTAINVIEW



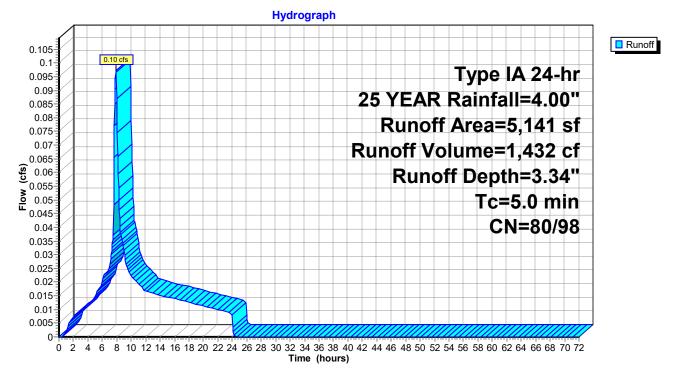
### Summary for Subcatchment 4S1: CENTER (N)

Runoff = 0.10 cfs @ 7.91 hrs, Volume= 1,432 cf, Depth= 3.34"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

Α	rea (sf)	CN	Description							
	3,878	98	Paved road	Paved roads w/curbs & sewers						
	1,263	80	>75% Gras	>75% Grass cover, Good, HSG D						
	5,141	94	Weighted Average							
	1,263	80	24.57% Pervious Area							
	3,878	98	75.43% Impervious Area							
-		~		<b>o</b>						
Tc	Length	Slop	,	Capacity	Description					
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)						
5.0					Direct Entry,					
					-					

# Subcatchment 4S1: CENTER (N)



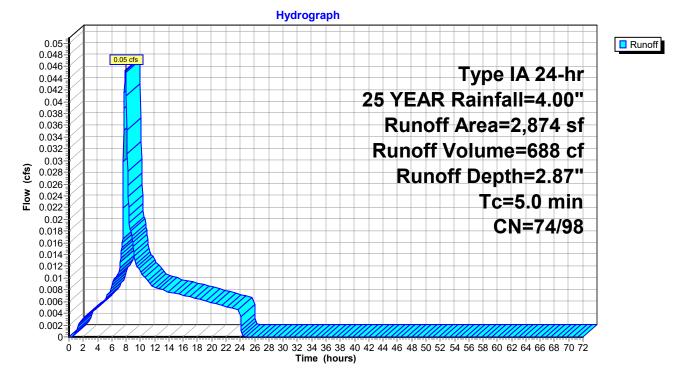
### Summary for Subcatchment 5S1: ALDERSGATE (N)

Runoff = 0.05 cfs @ 7.93 hrs, Volume= 688 cf, Depth= 2.87"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type IA 24-hr 25 YEAR Rainfall=4.00"

A	rea (sf)	CN	Description		
	1,693	98	Paved road	s w/curbs &	& sewers
	1,181	74	>75% Gras	s cover, Go	ood, HSG C
	2,874	88	Weighted A	verage	
	1,181	74	41.09% Per	vious Area	a
	1,693	98	58.91% Imp	pervious Are	ea
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment 5S1: ALDERSGATE (N)



**4487-01 Springbrook - Post** Prepared by AKS ENGINEERING & FORESTRY, LLC HydroCAD® 10.00-22 s/n 01338 © 2018 HydroCAD Software Solutions LLC

#### Summary for Reach S1: VEGETATED SWALE

Inflow = 2.50 cfs @ 7.98 hrs, Volume= 130,073 cf Outflow = 2.47 cfs @ 8.04 hrs, Volume= 130,073 cf, Atten= 1%, Lag= 3.2 min

\_ .

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 0.30 fps, Min. Travel Time= 7.4 min Avg. Velocity = 0.17 fps, Avg. Travel Time= 12.6 min

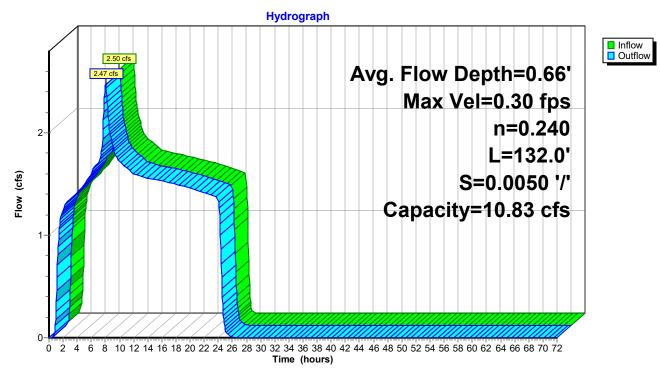
Peak Storage= 1,101 cf @ 8.04 hrs Average Depth at Peak Storage= 0.66' Bank-Full Depth= 1.50' Flow Area= 22.5 sf, Capacity= 10.83 cfs

Custom cross-section, Length= 132.0' Slope= 0.0050 '/' Constant n= 0.240 Inlet Invert= 221.00', Outlet Invert= 220.34'

‡

	Offset	Elevation	Chan.Depth	
	(feet)	(feet)	(feet)	
	-9.50	1.50	0.00	
	-7.00	0.50	1.00	
	-5.00	0.00	1.50	
	5.00	0.00	1.50	
	7.00	0.50	1.00	
	9.50	1.50	0.00	
	epth End			Storag
//	( 1 )	(fi) /1	() (	:_ £

Depth	End Area	Perim.	Storage	Discharge
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cfs)
0.00	0.0	10.0	0	0.00
0.50	6.0	14.1	792	1.49
1.50	22.5	19.5	2,970	10.83



### **Reach S1: VEGETATED SWALE**

## Summary for Pond 1R: WQMH/ BYPASS

Inflow Area =	1,100,942 sf,	68.23% Impervious,	Inflow Depth = 3.14" for 25 YEAR event
Inflow =	18.83 cfs @	7.97 hrs, Volume=	288,496 cf
Outflow =	18.83 cfs @	7.97 hrs, Volume=	288,494 cf, Atten= 0%, Lag= 0.0 min
Primary =	16.53 cfs @	7.98 hrs, Volume=	195,335 cf
Secondary =	3.25 cfs @	5.50 hrs, Volume=	93,159 cf

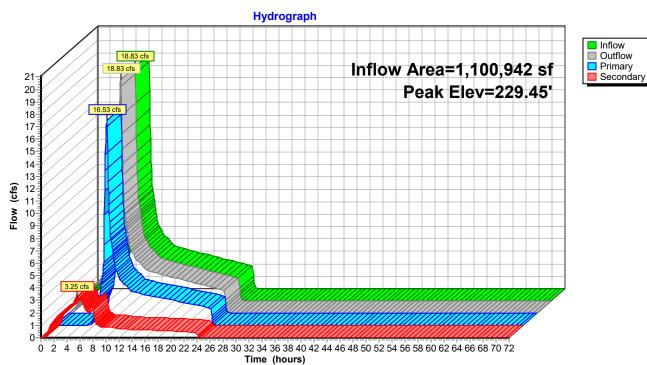
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 229.45' @ 8.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	227.15'	<b>36.0" Round High Flow Bypass</b> L= 50.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 227.15' / 224.50' S= 0.0530 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#2	Secondary	225.55'	<b>12.0" Round WQ Flow</b> L= 10.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 225.55' / 225.35' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	225.15'	<b>12.0" Round WQ Flow</b> L= 32.5' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 225.15' / 224.50' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=14.83 cfs @ 7.98 hrs HW=229.39' TW=228.98' (Dynamic Tailwater) ☐ 1=High Flow Bypass (Outlet Controls 14.83 cfs @ 3.63 fps)

Secondary OutFlow Max=3.19 cfs @ 5.50 hrs HW=227.24' TW=226.32' (Dynamic Tailwater)

**1**-3=WQ Flow (Inlet Controls 3.19 cfs @ 4.07 fps)



### Pond 1R: WQMH/ BYPASS

### Summary for Pond 2R: BYPASS

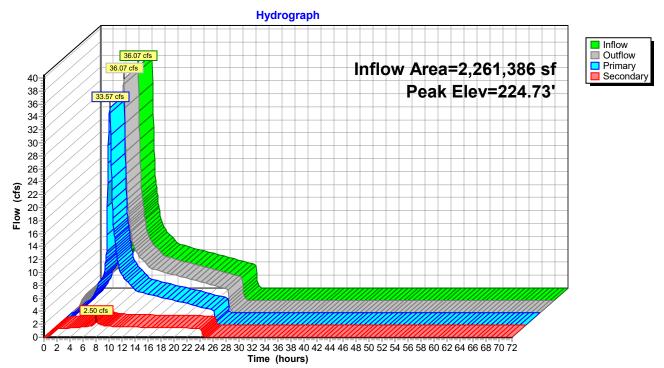
Inflow Area =	2,261,386 sf,	64.41% Impervious,	Inflow Depth = 3.02" for 25 YEAR event
Inflow =	36.07 cfs @	7.98 hrs, Volume=	568,809 cf
Outflow =	36.07 cfs @	7.98 hrs, Volume=	568,809 cf, Atten= 0%, Lag= 0.0 min
Primary =	33.57 cfs @	7.98 hrs, Volume=	438,736 cf
Secondary =	2.50 cfs @	7.98 hrs, Volume=	130,073 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 224.73' @ 7.98 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	221.20'	<b>8.0" Round WQ Flow</b> L= 40.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet Invert= $221.20'$ / $221.00'$ S= $0.0050'$ /' Cc= $0.900$ n= $0.013$ Corrugated PE, smooth interior, Flow Area= $0.35$ sf
#2	Primary	222.00'	<b>36.0" Round High Flow Bypass</b> L= 50.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 222.00' / 221.20' S= 0.0160 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf

**Primary OutFlow** Max=33.38 cfs @ 7.98 hrs HW=224.72' TW=218.28' (Dynamic Tailwater) **2=High Flow Bypass** (Inlet Controls 33.38 cfs @ 4.95 fps)

Secondary OutFlow Max=2.49 cfs @ 7.98 hrs HW=224.72' TW=221.66' (Dynamic Tailwater) T=WQ Flow (Barrel Controls 2.49 cfs @ 7.14 fps)



# Pond 2R: BYPASS

### Summary for Pond P1: EXTENDED DRY BASIN

Inflow A Inflow Outflow Primary	= =	1,100,942 s 18.83 cfs @ 11.32 cfs @ 11.32 cfs @	7.97 h 8.29 h	% Impervious, Inflo rs, Volume= rs, Volume= rs, Volume=	ow Depth = 3.14" 288,494 cf 281,188 cf, Atten= 281,188 cf	for  25 YEAR event = 40%,  Lag= 19.4 min
				Span= 0.00-72.00 l rea= 21,308 sf St		
				lculated for 281,18 ,201.5 - 692.3)	8 cf (97% of inflow)	
Volume	Inv	ert Avail.	Storage	Storage Descripti	on	
#1	224.	50' 109	9,826 cf	Custom Stage Da	<b>ata (Irregular)</b> Listed	l below (Recalc)
<b>F</b> laundi			Derive		Ourse Otherse	
Elevatio (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
224.5	/	13,412	505.4	0	0	13,412
230.		23,680	635.5	109,826	109,826	25,710
200.		20,000	000.0	100,020	100,020	20,110
Device	Routing	Inve	ert Outle	et Devices		
#1	Primary	223.4		" Round Outfall F	•	
					edge headwall, Ke	
						).0050 '/' Cc= 0.900
#2	Device <sup>-</sup>	1 223.5		Vert. WQ Orifice	PE, smooth interior,	Flow Area= 0.79 st
#2 #3	Device				oper Ditch Inlet C=	- 0.600
<i>#</i> <b>0</b>	Device	1 220.0		ted to weir flow at l		0.000
#4	Primary	225.5		" Round Outfall F		
					edge headwall, Ke	
						).0250 '/' Cc= 0.900
					PE, smooth interior,	
#5	Device 4	4 229.0			per Ditch Inlet C=	= 0.600
			LIM	ted to weir flow at I	ow neads	

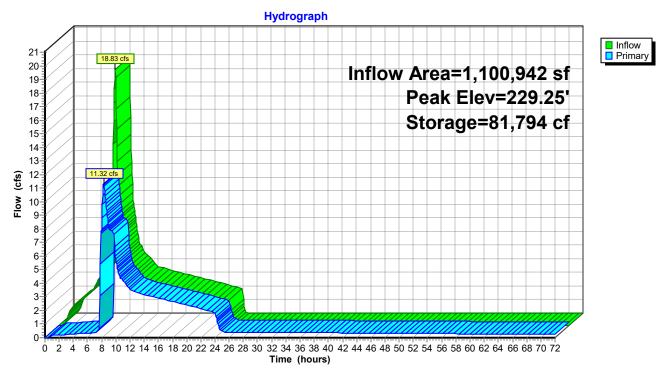
**Primary OutFlow** Max=11.31 cfs @ 8.29 hrs HW=229.25' TW=0.00' (Dynamic Tailwater)

-1=Outfall Pipe (Barrel Controls 8.13 cfs @ 10.35 fps)

**2=WQ Orifice** (Passes < 0.45 cfs potential flow) **3=Upper Ditch Inlet** (Passes < 15.67 cfs potential flow)

**4=Outfall Pipe** (Passes 3.18 cfs of 6.82 cfs potential flow)

**5=Upper Ditch Inlet** (Weir Controls 3.18 cfs @ 1.64 fps)



### Pond P1: EXTENDED DRY BASIN

### Summary for Pond P2: DETENTION POND

Inflow Area	a =	2,261,386 sf,	64.41% Impervious,	Inflow Depth = 3.02"	for 25 YEAR event
Inflow	=	36.02 cfs @	7.98 hrs, Volume=	568,809 cf	
Outflow	=	18.94 cfs @	8.44 hrs, Volume=	519,912 cf, Atte	en= 47%, Lag= 27.3 min
Primary	=	18.94 cfs @	8.44 hrs, Volume=	519,912 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 218.81' @ 8.44 hrs Surf.Area= 31,612 sf Storage= 162,868 cf Flood Elev= 219.00' Surf.Area= 32,084 sf Storage= 168,924 cf

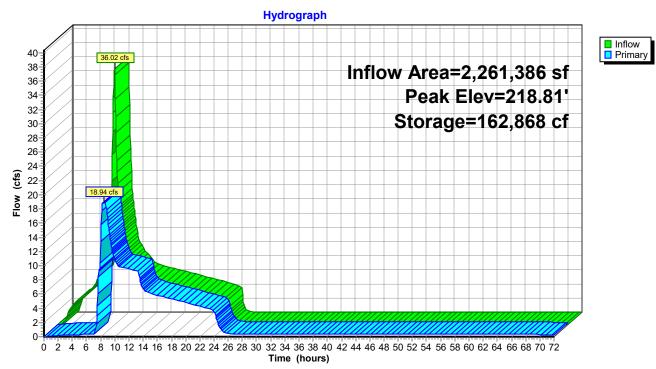
Plug-Flow detention time= 421.5 min calculated for 519,912 cf (91% of inflow) Center-of-Mass det. time= 360.3 min (1,061.2 - 700.9)

Volume	Invert	Avail.Sto	orage	Storage Description	on		
#1	212.00'	202,2	72 cf	Custom Stage Da	<b>ata (Irregular)</b> Liste	ed below (Recalc)	_
Elevatic (fee		urf.Area  F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
212.0	/	16,833	666.0	0	0	16,833	
213.0		,	684.8	17,837	17,837	18,964	
220.0	0	34,628	816.8	184,435	202,272	35,589	
Device	Routing	Invert	Outl	et Devices			
#1	Primary	210.50'		" Round Outfall			
				0.0' CMP, square			
						= 0.3900 '/' Cc= 0.900	
		044.001				,Flow Area= 1.77 sf	
#2	Device 1	211.00'		" Round Detentio			
				0.0' CMP, square		e= 0.500 0.0050 '/' Cc= 0.900	
						, Flow Area= 0.79 sf	
#3	Device 2	211.00'		Vert. Detention O			
#3	Device 2	216.90'		" x 16.0" Horiz. Up			
<i>n</i> -	Device 2	210.00		ted to weir flow at l		0-0.000	
#5	Device 1	213.00'		" Round Detentio			
<i>"</i> 0	Denice	210.00		0.0' CMP, square		(e= 0.500	
			Inlet	/ Outlet Invert= 21	3.00' / 212.25' S=	= 0.0250 '/' Cc= 0.900	
			n= 0	.013 Corrugated F	E, smooth interior	,Flow Area= 0.79 sf	
#6	Device 5	218.20'	30.0	" x 16.0" Horiz. Up	per Ditch Inlet 2	C= 0.600	
			Limi	ted to weir flow at le	ow heads		
1=Ou	tfall (Passe	es 18.94 cfs o	of 23.3	44 hrs HW=218.81 9 cfs potential flow) Is 10.22 cfs @ 13.0	)	amic Tailwater)	
	-3=Detentio	on Orifice 1 (	Passe	s < 0.46 cfs potenti			

**4=Upper Ditch Inlet 1** (Passes < 22.18 cfs potential flow)

-5=Detention Pipe 2 (Inlet Controls 8.71 cfs @ 11.09 fps)

**6=Upper Ditch Inlet 2** (Passes 8.71 cfs of 11.92 cfs potential flow)

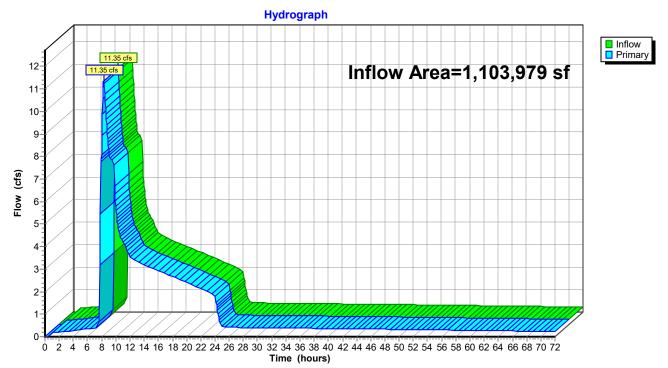


### Pond P2: DETENTION POND

# Summary for Link CS: CENTER ST (S)

Inflow Area	=	1,103,979 sf,	68.28% Impervious,	Inflow Depth >	3.07"	for 25 YEAR event
Inflow =	=	11.35 cfs @	8.29 hrs, Volume=	282,093 cf		
Primary =	=	11.35 cfs @	8.29 hrs, Volume=	282,093 cf	, Atten	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

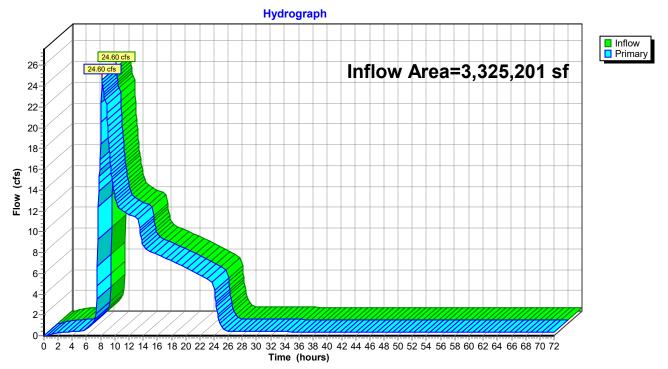


# Link CS: CENTER ST (S)

## Summary for Link HC: HESS CREEK

Inflow Are	a =	3,325,201 sf,	45.12% Impervious,	Inflow Depth > 2.37'	for 25 YEAR event
Inflow	=	24.60 cfs @	8.21 hrs, Volume=	657,727 cf	
Primary	=	24.60 cfs @	8.21 hrs, Volume=	657,727 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



# Link HC: HESS CREEK



# **Appendix C:** TR-55 RUNOFF CURVE NUMBERS

#### **Table 2-2a**Runoff curve numbers for urban areas 1/2

Cover description				umbers for c soil group	
	Average percent		• 0	01	
Cover type and hydrologic condition i	mpervious area <sup>2</sup>		В	С	D
Fully developed urban areas (vegetation established)					
Open space (lawns, parks, golf courses, cemeteries, etc.) 와:					
Poor condition (grass cover < 50%)		68	79	86	89
Fair condition (grass cover 50% to 75%)		49	69	79	84
Good condition (grass cover > 75%)		39	61	74	80
Impervious areas:	•••••	50	01	• •	00
Paved parking lots, roofs, driveways, etc.					
(excluding right-of-way)		98	98	98	98
Streets and roads:	•••••	50	50	50	50
Paved; curbs and storm sewers (excluding					
right-of-way)		98	98	98	98
Paved; open ditches (including right-of-way)		83	38 89	92	93
		85 76	85	92 89	95 91
Gravel (including right-of-way)		76 72	89 82	89 87	91 89
Dirt (including right-of-way)	•••••	12	82	81	89
Western desert urban areas:		60	88	05	00
Natural desert landscaping (pervious areas only) 4/		63	77	85	88
Artificial desert landscaping (impervious weed barrier,					
desert shrub with 1- to 2-inch sand or gravel mulch					
and basin borders)		96	96	96	96
Urban districts:					
Commercial and business		89	92	94	95
Industrial	72	81	88	91	93
Residential districts by average lot size:					
1/8 acre or less (town houses)		77	85	90	92
1/4 acre		61	75	83	87
1/3 acre		57	72	81	86
1/2 acre	25	54	70	80	85
1 acre	20	51	68	79	84
2 acres	12	46	65	77	82
Developing urban areas					
Newly graded areas					
(pervious areas only, no vegetation) <sup>5/</sup>		77	86	91	94
dle lands (CN's are determined using cover types					
similar to those in table 2-2c).					

<sup>1</sup> Average runoff condition, and  $I_a = 0.2S$ .

<sup>2</sup> The average percent impervious area shown was used to develop the composite CN's. Other assumptions are as follows: impervious areas are directly connected to the drainage system, impervious areas have a CN of 98, and pervious areas are considered equivalent to open space in good hydrologic condition. CN's for other combinations of conditions may be computed using figure 2-3 or 2-4.

<sup>3</sup> CN's shown are equivalent to those of pasture. Composite CN's may be computed for other combinations of open space

cover type.

<sup>4</sup> Composite CN's for natural desert landscaping should be computed using figures 2-3 or 2-4 based on the impervious area percentage (CN = 98) and the pervious area CN. The pervious area CN's are assumed equivalent to desert shrub in poor hydrologic condition.

<sup>5</sup> Composite CN's to use for the design of temporary measures during grading and construction should be computed using figure 2-3 or 2-4 based on the degree of development (impervious area percentage) and the CN's for the newly graded pervious areas.

#### **Table 2-2b**Runoff curve numbers for cultivated agricultural lands 1/2

	Cover description		Curve numbers for hydrologic soil group					
	Cover description	Hydrologic	ilyurologic son group					
Cover type	Treatment 2/	condition <sup>3/</sup>	А	В	С	D		
Fallow	Bare soil	_	77	86	91	94		
	Crop residue cover (CR)	Poor Good	76 74	85 83	90 88	93 90		
Row crops	Straight row (SR)	Poor Good	72 67	81 78	88 85	91 89		
	SR + CR	Poor Good	$\begin{array}{c} 71 \\ 64 \end{array}$	80 75	87 82	90 85		
	Contoured (C)	Poor Good	$\begin{array}{c} 70 \\ 65 \end{array}$	79 75	84 82	88 86		
	C + CR	Poor Good	$\begin{array}{c} 69 \\ 64 \end{array}$	78 74	83 81	87 85		
	Contoured & terraced (C&T)	Poor Good	$\begin{array}{c} 66 \\ 62 \end{array}$	74 71	80 78	82 81		
	C&T+ CR	Poor Good	$\begin{array}{c} 65 \\ 61 \end{array}$	73 70	79 77	81 80		
Small grain	SR	Poor Good	$\begin{array}{c} 65\\ 63 \end{array}$	76 75	84 83	88 87		
	SR + CR	Poor Good	$\begin{array}{c} 63\\ 64\\ 60\end{array}$	75 72	83 80	86 84		
	С	Poor Good	$\begin{array}{c} 63\\ 61\end{array}$	74 73	82 81	85 84		
	C + CR	Poor Good	62 60	73 72	81 80	84 83		
	C&T	Poor Good	$\begin{array}{c} 61 \\ 59 \end{array}$	72 70	79 78	82 81		
	C&T+ CR	Poor Good	60 58	$\begin{array}{c} 71 \\ 69 \end{array}$	78 77	81 80		
Close-seeded or broadcast	SR	Poor Good	66 58	77 72	85 81	89 85		
legumes or rotation	С	Poor Good	$\begin{array}{c} 66\\ 55 \end{array}$	75 $69$	83 78	85 83		
meadow	C&T	Poor Good	63 51	73 67	80 76	83 80		

 $^{\rm 1}$  Average runoff condition, and  $\rm I_a{=}0.2S$ 

 $^2$  Crop residue cover applies only if residue is on at least 5% of the surface throughout the year.

<sup>3</sup> Hydraulic condition is based on combination factors that affect infiltration and runoff, including (a) density and canopy of vegetative areas, (b) amount of year-round cover, (c) amount of grass or close-seeded legumes, (d) percent of residue cover on the land surface (good  $\geq$  20%), and (e) degree of surface roughness.

Poor: Factors impair infiltration and tend to increase runoff.

Good: Factors encourage average and better than average infiltration and tend to decrease runoff.

#### Table 2-2c Runoff curve numbers for other agricultural lands $1\!\!/$

Cover description		Curve numbers for hydrologic soil group				
Cover type	Hydrologic condition	А	B	C	D	
Pasture, grassland, or range—continuous	Poor	68	79	86	89	
forage for grazing. $2$	Fair Good	$\frac{49}{39}$	$\begin{array}{c} 69 \\ 61 \end{array}$	79 74	84 80	
Meadow—continuous grass, protected from grazing and generally mowed for hay.	_	30	58	71	78	
Brush—brush-weed-grass mixture with brush the major element. ${}^{3\!/}$	Poor Fair Good	48 35 30 4⁄	$67 \\ 56 \\ 48$	77 70 65	83 77 73	
Woods—grass combination (orchard or tree farm). 5/	Poor Fair Good	57 43 32	73 65 58	82 76 72	86 82 79	
Woods. 6/	Poor Fair Good	45 36 30 4⁄	66 60 55	77 73 70	83 79 77	
Farmsteads—buildings, lanes, driveways, and surrounding lots.	—	59	74	82	86	

1 Average runoff condition, and  $I_a = 0.2S$ .

 $\mathbf{2}$ *Poor:* <50%) ground cover or heavily grazed with no mulch. Fair: 50 to 75% ground cover and not heavily grazed.

Good: > 75% ground cover and lightly or only occasionally grazed. 3

*Poor*: <50% ground cover.

50 to 75% ground cover. Fair:

*Good:* >75% ground cover.

4 Actual curve number is less than 30; use CN = 30 for runoff computations.

5CN's shown were computed for areas with 50% woods and 50% grass (pasture) cover. Other combinations of conditions may be computed from the CN's for woods and pasture.

6 Poor: Forest litter, small trees, and brush are destroyed by heavy grazing or regular burning. Fair: Woods are grazed but not burned, and some forest litter covers the soil. Good: Woods are protected from grazing, and litter and brush adequately cover the soil.



# Appendix D: USDA-NRCS SOIL RESOURCE REPORT



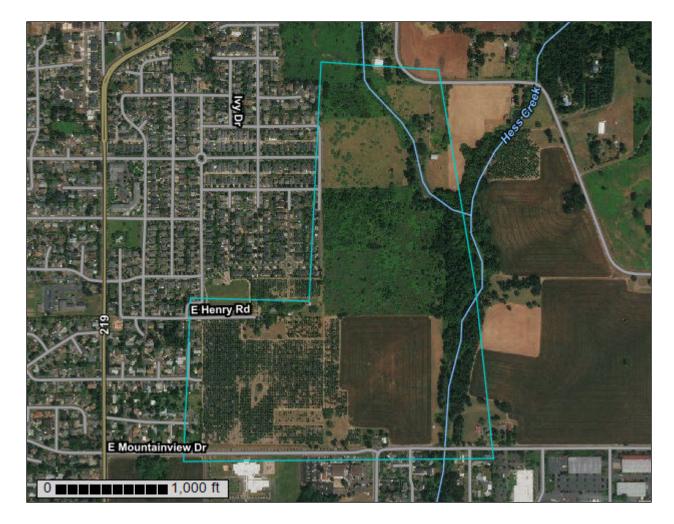
United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

# Custom Soil Resource Report for Yamhill County, Oregon



# Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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# **Soil Information for All Uses**

# **Soil Properties and Qualities**

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

# **Soil Qualities and Features**

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

# Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

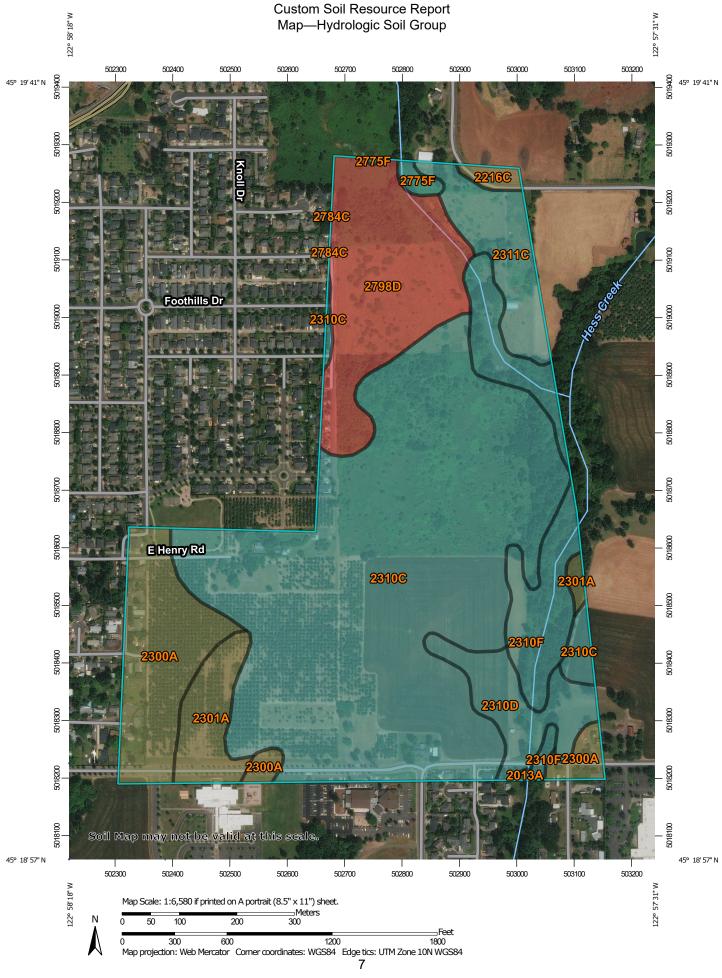
Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

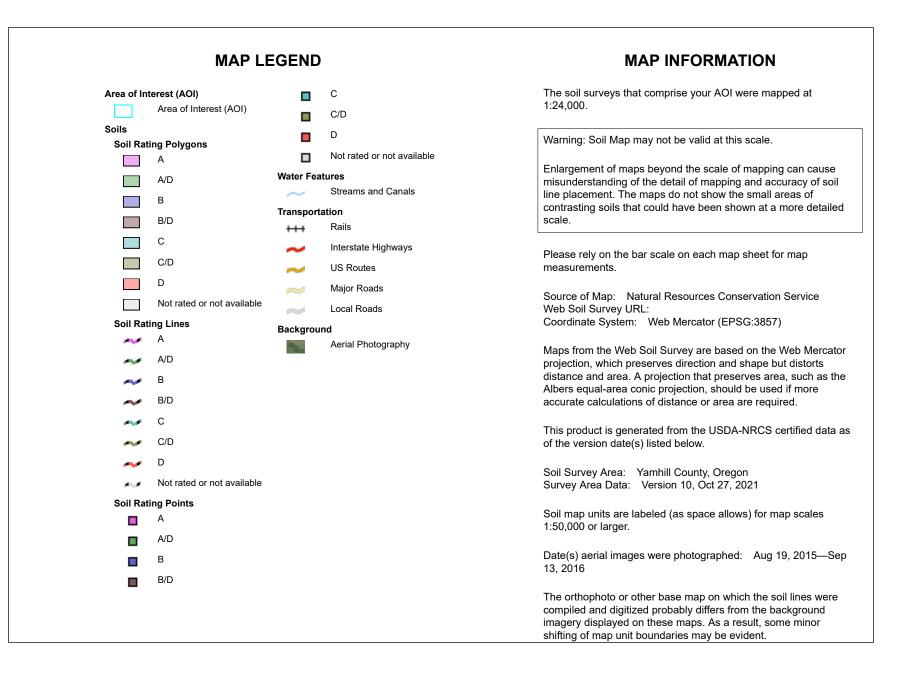
Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.





# Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
2013A	Wapato silty clay loam, 0 to 3 percent slopes	C/D	0.1	0.0%
2216C	Chehalem silty clay loam, volcanic, 3 to 12 percent slopes	C/D	0.9	0.6%
2300A	Aloha silt loam, 0 to 3 percent slopes	C/D	15.1	10.0%
2301A	Amity silt loam, 0 to 3 percent slopes	C/D	6.1	4.1%
2310C	Woodburn silt loam, 3 to 12 percent slopes	С	80.5	53.4%
2310D	Woodburn silt loam, 12 to 20 percent slopes	С	7.2	4.8%
2310F	Woodburn silt loam, 20 to 55 percent slopes	С	12.3	8.1%
2311C	Helvetia silt loam, 2 to 12 percent slopes	С	7.7	5.1%
2775F	Saum-Ritner complex, 30 to 75 percent slopes	С	0.8	0.5%
2784C	Witzel-Ritner complex, 2 to 12 percent slopes, stony	D	0.0	0.0%
2798D	Witham silty clay loam, hummocky, 2 to 25 percent slopes	D	19.9	13.2%
Totals for Area of Interest			150.6	100.0%

# **Rating Options—Hydrologic Soil Group**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

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